

# MOUNTAIN SHADOWS RESORT TRAFFIC IMPACT ANALYSIS 

5445 East Lincoln Drive Town of Paradise Valley, Arizona

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## EXECUTIVE SUMMARY

This report documents a traffic impact analysis (TIA) performed for the Mountain Shadows Resort. The Resort consists of an existing 183-room hotel and 41 condominiums. A new Bianco restaurant is proposed for the northeast corner of the site and will consist of approximately 5,000 square feet.

CivTech, Inc. has been retained by Westroc Hospitality LLC to perform the TIA for the proposed median break at the west driveway. The purpose of this TIA is to determine if a westbound left turn lane is warranted at the existing Mountain Shadows West Driveway. This driveway currently operates as a right in/right out driveway only.

The following conclusions have been documented for this study.

## General

- The proposed restaurant is anticipated to generate approximately 420 additional weekday daily trips, with 4 ( $3 \mathrm{in} / 1$ out) additional trips occurring in the AM peak hour and 39 (26 in/13 out) additional trips occurring in the PM peak hour.


## Existing Conditions

- The results of the existing conditions analysis summarized in Table 2 indicates that all intersections currently operate at an overall acceptable level of service (LOS C or better) under existing lane configurations and signal timing.


## Opening Year 2020

- The results of the 2020 opening year Synchro analysis indicates that all study intersections are anticipated to experience an acceptable level of service LOS C or better.


## Left Turn Lane Analysis

- The Town of Paradise Valley does not provide explicit criteria for the installation of dedicated left turn lanes; they typically defer to City of Scottsdale design standards.
o Per the City of Scottsdale Design Standards and Policy Manual (DS\&PM), driveways located on arterial roadways must be spaced 330 feet apart, with a minimum spacing of 250 feet with no restrictions. The eastern and western driveways for the Mountain Shadows Resort are spaced approximately 530 feet apart, meaning that a left turn lane into the site at the western driveway should be permitted. A dedicated westbound left turn lane is also safer than making a U-turn at an unprotected signalized intersection or at a median break, which was observed to be happening during both the AM and PM peak hours.


## Queue Storage

- The recommended storage lengths are provided for horizon year 2020 using the total traffic projections.
o There is an existing median break along Lincoln Drive directly east of the proposed median break at the west driveway. This median break is utilized as emergency access for the Colonia Miramonte housing development north of the Mountain Shadows Resort. Since this is a gated emergency access only not available to residents for daily usage, it was determined this median break can be utilized in the design of a westbound left turn lane into the Mountain Shadows Resort west driveway. The determination was provided by the Town based on concept designs provided by the Town's consultant as part of earlier review comments for this project. A minimum of 75 feet of storage is recommended with a 90foot taper for a total length of 165 feet.
o It is recommended that the median be constructed with mountable curb in the areas proximate to the Colonia Miramonte emergency access.


## INTRODUCTION

Mountain Shadows Resort currently consists of a 183-room hotel and 41 condominiums. A new Bianco Restaurant is proposed for the northeast corner of the existing site. The Mountain Shadows Resort is located at 5445 E Lincoln Drive in the Town of Paradise Valley, Arizona.

## Study Requirements

The purpose of this study is to determine if a median break and dedicated westbound left turn lane is warranted at the Mountain Shadows West Driveway on Lincoln Drive. CivTech has been informed that vehicles utilize the signalized intersection of Desert Fairways Drive and Lincoln Drive to access the west driveway after driving past 56 ${ }^{\text {th }}$ Street and the Mountain Shadows East Driveway. Vehicles travelling west on Lincoln Drive and miss the entrance to the resort, either from $56{ }^{\text {th }}$ Street or the east driveway, typically attempt to make a U-turn at the first opportunity. Most of the U-turns observed occurred at the median break west of the site and east of the signalized intersection of Desert Fairways Drive and Lincoln Drive; there is a "No U-turn" sign at this location. The U-turns in this area have caused issues with the neighbors in the area and also create an unsafe situation for other vehicles on Lincoln Drive.

The specific objectives of the study are:

- To determine if enough vehicles are making U-turns at Desert Fairways Drive on Lincoln Drive to warrant the addition of a median break and dedicated westbound left turn lane at the Mountain Shadows West Driveway.
- To recommend additional street or traffic control improvements, where necessary, to mitigate existing and future delays at all study intersections.


## Study Area

This study technically does not require a full TIA based alone on the addition of the new Bianco Restaurant. However, since U-turns have been observed at the signalized intersection of Desert Fairways Drive and Lincoln Drive and the need for a westbound left turn lane into the site at the west driveway is being assessed, a full TIA has been performed. The following study area intersections have been evaluated:

- Desert Fairways Drive \& Lincoln Drive
- Mountain Shadows West \& Lincoln Drive
- Mountain Shadows East \& Lincoln Drive
- $56^{\text {th }}$ Street $\&$ Lincoln Drive


## Horizon Years

The proposed opening of the new Bianco Restaurant is spring of 2020, therefore an opening year of 2020 will be analyzed.
A location map of the study area is provided in Figure 1.


Figure I: Vicinity Map

## EXISTING CONDITIONS

## SURROUNDING LAND USE

The surrounding area includes various land uses. Directly north of the site, on the north side of Lincoln Drive, is the site for the Colonia Miramonte housing development and El Chorro Restaurant. South, East, and West of the site are detached single-family homes.

## EXISTING ROADWAY NETWORK

The existing roadway network analyzed in this study includes Desert Fairways Drive, Lincoln Drive, and $56^{\text {th }}$ Street.

Desert Fairways Drive is a north-south road with one lane in each travelling direction. Desert Fairways Drive begins at the intersection with Tatum Boulevard and continues east for approximately 0.25 miles before looping to the south and continuing south until the intersection with Lincoln Drive, at which point it transitions to Camelback Manor Drive. The posted speed limit is 25 miles per hour along Desert Fairways Drive. Once becoming Camelback Manor there is no posted speed limit.

Lincoln Drive is an east-west four-lane major arterial with two lanes in each travelling direction. Within the vicinity of the site, there are raised medians along portions of the road. Lincoln Drive begins just east of the State Route 51 freeway and continues east for approximately 7 miles before terminating at the intersection with Cattletrack Road, just west of the Arizona Canal. The posted speed limit is 40 miles per hour (mph) within the vicinity of the site.
$56^{\text {th }}$ Street is a north-south two-lane collector road with one lane in each travelling direction. 56 ${ }^{\text {th }}$ Street begins just north of Lincoln Drive at the El Chorro Restaurant driveway and continues south for approximately 0.5 miles before terminating at the intersection with McDonald Drive. The posted speed limit is 25 mph within the vicinity of the site.

## EXISTING INTERSECTION CONFIGURATION

The intersection of Desert Fairways Drive and Lincoln Drive is a four-legged signalized intersection with permissive/protected left turns on the eastbound approach and permissive phasing on the northbound, southbound and westbound approaches. The northbound approach consists of one shared left-turn/through/right-turn lane. The southbound approach consists of one dedicated left-turn lane and a shared through/right-turn lane. The eastbound and westbound approaches each have one dedicated left-turn lane, one through lane and a shared through/right-turn lane. There are pedestrian crosswalks across all legs of the intersection.

The intersection of Mountain Shadows West and Lincoln Drive is a three-legged, stop-controlled "T" intersection with free movements on the eastbound and westbound approaches and a stop sign on the northbound approach. The northbound approach consists of one right turn lane. The eastbound approach consists of two through lanes and a dedicated right-turn lane. The westbound approach has two through lanes and a median to restrict left-turns into the site.

The intersection of Mountain Shadows East and Lincoln Drive is a three-legged, stop-controlled "T" intersection with free movements on the eastbound and westbound approaches and a stop sign on the northbound approach. The northbound approach consists of one shared left-turn/right-turn lane. The eastbound approach consists of two through lanes and a dedicated right-turn lane. The westbound approach consists of one dedicated left-turn lane and two through lanes.

The intersection of $56^{\text {th }}$ Street and Lincoln Drive is a four-legged signalized intersection with permissive phasing on all approaches. The northbound approach consists of one shared left-turn/through lane and a dedicated right-turn lane. The southbound approach consists of one shared left-turn/through/right-turn lane. The eastbound and westbound approaches each consist of a dedicated left-turn lane, one through lane and a shared through/right-turn lane.

The existing intersection configurations and traffic control is illustrated in Figure 2.

## EXISTING TRAFFIC VOLUMES

CivTech engaged Field Data Services of Arizona, Inc. to record traffic volumes at the four study intersections within the project vicinity. Peak hour volume turning movement counts were performed from 7:00-9:00 AM and 4:00-6:00 PM on Thursday, March 7, 2019. Peak hour turning movement counts were conducted at the following study intersections:

- Desert Fairways Drive \& Lincoln Drive
- Mountain Shadows West Driveway \& Lincoln Drive
- Mountain Shadows East Driveway \& Lincoln Drive
- $56^{\text {th }}$ Street $\&$ Lincoln Drive

Due to the nature of this study, U-turn data was also collected at the following intersections to determine if an eastbound left turn lane into the Mountain Shadows Resort site would be warranted. Field Data Services of Arizona, Inc. recorded U-turns at the following intersection from 7:00-9:00 AM and 4:00-6:00 PM on Thursday, March 7, 2019.

- Desert Fairways Drive \& Lincoln Drive

CivTech recorded U-turn data at the median break west of the Mountain Shadows West Driveway, but east of the intersection with Desert Fairways Drive from 7:00-9:00 AM and 4:00-6:00 PM on Thursday, March 7, 2019. These counts were conducted because it was noted by the client that U-turns also occur at this location.

The Town of Paradise Valley requires that a seasonal adjustment factor be applied to existing traffic counts taken outside of typical months. These traffic counts were conducted in March, and winter months typically have higher amounts of traffic due to the high number of seasonal visitors. The City of Scottdale 2016 Average Daily Traffic Volumes seasonal adjustment factor for March is 0.93. Existing 2019 traffic volumes with the seasonal adjustment factor applied are presented in Figure 3 for the weekday AM and PM peak hours. Raw traffic volume data obtained for this study has been included in Appendix B.



XX(XX) - AM(PM) Peak Hour Traffic Volumes
$\square$ X(X) - AM(PM) Peak Hour U-Turn Volumes
Note: U-Turns not included in the left turning Volumes at Intersection "1"
Colonia Miramonte Gated Emergency Access


Figure 3: Existing Traffic Volumes

## EXISTING CAPACITY ANALYSIS

Peak hour capacity analyses have been conducted for the study intersections based on existing intersection configurations and traffic volumes. All intersections have been analyzed using the methodologies presented in the Highway Capacity Manual (HCM), Special Report 209, and Updated 2016 and using Synchro software, version 10.0 under the HCM $6^{\text {th }}$ edition methodology.

The concept of level of service (LOS) uses qualitative measures that characterize operational conditions within the traffic stream. The individual levels of service are described by factors that include speed, travel time, freedom to maneuver, traffic interruptions, and comfort and convenience. Six levels of service are defined for each type of facility for which analysis procedures are available. They are given letter designations A through $F$, with LOS A representing the best operating conditions and LOS F the worst. Each level of service represents a range of operating conditions. Levels of service for intersections are defined in terms of delay ranges. Table 1 lists the level of service criteria for signalized and unsignalized intersections, respectively.

Table 1: Level of Service Criteria

| Level of Service | Control Delay (seconds/vehicle) |  |
| :---: | :---: | :---: |
|  | Signalized Intersections | Unsignalized Intersections |
| A | $\leq 10$ | $\leq 10$ |
| B | $>10-20$ | $>10-15$ |
| C | $>20-35$ | $>15-25$ |
| F | $>35-55$ | $>25-35$ |
| F | $>55-80$ | $>35-50$ |

Source: Exhibit 19-8, Exhibit 20-2, Exhibit 21-8 and Exhibit 22-8, Highway Capacity Manual 2017
Synchro 10.0 software calculates the LOS per the HCM $6^{\text {th }}$ edition methodology. The $6{ }^{\text {th }}$ edition HCM documents the signalized LOS calculation methodology which takes into account lane geometry, traffic volumes and cycle length/phasing to compute LOS. Synchro analysis worksheets report individual movement delay/LOS and overall delay/LOS for signalized intersections; unsignalized intersection worksheets report the worst-case delay/LOS and the average overall intersection delay. Signal timing data for the intersections of Lincoln Drive/Desert Fairways Drive and Lincoln Drive/56th Street were provided by the Town of Paradise Valley. Results of the existing level of service analyses are shown in Table 2 for both AM and PM peak hours. The existing conditions analysis worksheets have been included in Appendix C.

Table 2: Existing Peak Hour Levels of Service

| ID | Intersection | Intersection Control | Approach/ Movement | $\begin{gathered} \text { Existing LOS } \\ \text { AM (PM) } \\ \hline \hline \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: |
| 1 | Desert Fairways Dr \& Lincoln Dr | Signal | NB | C(C) |
|  |  |  | SB | $\mathrm{C}(\mathrm{C})$ |
|  |  |  | EB | A(A) |
|  |  |  | WB | $B(B)$ |
|  |  |  | Overall | B(B) |
| 2 | Mountain Shadows West Drwy \& Lincoln Dr | 1-way stop (NB) | NB Right | B(B) |
| 3 | Mountain Shadows East Drwy \& Lincoln Dr | $\begin{aligned} & \text { 1-way stop } \\ & \text { (NB) } \\ & \hline \end{aligned}$ | NB Shared WB Left | $\begin{aligned} & \hline \mathrm{C}(\mathrm{~A}) \\ & \mathrm{A}(\mathrm{~A}) \\ & \hline \end{aligned}$ |
| 4 | $56^{\text {th }}$ St \& Lincoln Dr | Signal | $\begin{aligned} & \hline \text { NB } \\ & \text { SB } \\ & \text { EB } \\ & \text { WB } \\ & \hline \end{aligned}$ | $\begin{aligned} & \mathrm{C}(\mathrm{C}) \\ & \mathrm{C}(\mathrm{C}) \\ & \mathrm{A}(\mathrm{~A}) \\ & \mathrm{A}(\mathrm{~A}) \\ & \hline \end{aligned}$ |
|  |  |  | Overall | A(A) |

The results of the existing conditions analysis summarized in Table 2 indicates that all intersections currently operate at an overall acceptable level of service (LOS C or better) under existing lane configurations and signal timing.

## PROPOSED DEVELOPMENT

## SITE LOCATION

The proposed development will be located at 5445 East Lincoln Drive in the Town of Paradise Valley, Arizona on the same site as the existing Mountain Shadows Resort.

## SITE ACCESS

There are two existing access points for this development along Lincoln Drive, described as follows:

- The West Access is an existing right in/right out only access along Lincoln Drive located approximately 855 feet east of the intersection of Desert Fairways Drive and Lincoln Drive.
- The East Access is an existing full movement access along Lincoln Drive located approximately 500 feet east of the west access.

The two existing Mountain Shadows Driveways will remain in their existing location, as well as a third access on $56^{\text {th }}$ Street on the eastern border of the site. Access to $56^{\text {th }}$ Street from the Mountain Shadows Resort was not evaluated based on a scoping meeting for this study with Town staff. Vehicular movement to/from the $56^{\text {th }}$ Street access are anticipated to remain the same and operate well in the existing conditions. The existing west access is proposed to include a dedicated westbound left turn lane to allow for left turns into the site while still restricting northbound left turns out of the site. All other access points are proposed to remain the same.
The site plan and access points are depicted in Figure 4.


Figure 4: Site Plan and Access

## TRIP GENERATION

The potential trip generation for the proposed development was estimated utilizing the Institute of Transportation Engineers (ITE) Trip Generation Manual, 10 ${ }^{\text {th }}$ Edition and Trip Generation Handbook, $3^{\text {rd }}$ Edition. The ITE Trip Generation Manual contains data collected by various transportation professionals for a wide range of different land uses. The data are summarized in the report and average rates and equations have been established that correlate the relationship between an independent variable that describes the development size and generated trips for each categorized land use. The report provides information for daily and peak hour trips.

Mountain Shadows Resort is an already existing resort with a 183-room hotel and 41 condominiums. A new Bianco restaurant is proposed for the northeast corner of the site. The restaurant will consist of 3,500 square feet (sf) of indoor seating and $1,500 \mathrm{sf}$ of patio seating. The land use code (LUC) used for this restaurant is 931 for a quality restaurant. For this ITE LUC, there is no data provided for the entering/exiting percentage during the AM peak hour. This is the case for the last three editions of the Trip Generation Manual. Since no data is provided, the percentage of vehicles entering during the AM peak hour of generator was used. The peak hour of generator is typically the peak hour calculated based on the specific land use and not the typical peak hour between 7:00 and 9:00 AM. Table 3 depicts the trip generation summary for the proposed development. Trip generation calculations are provided in Appendix D.

Table 3: Trip Generation Summary

| Proposed Use | $\begin{aligned} & \text { ITE } \\ & \text { LUC } \end{aligned}$ | Size | Units | Weekday Trips |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Daily <br> Total | AM |  |  | PM |  |  |
|  |  |  |  |  | In | Out | Total | In | Out | Total |
| Quality Restaurant | 931 | 5,000 | SF | 420 | 3 | 1 | 4 | 26 | 13 | 39 |
| Subtotals |  |  |  | 420 | 3 | 1 | 4 | 26 | 13 | 39 |

As shown in Table 3, the proposed development is anticipated to generate approximately 420 additional weekday daily trips, with 4 (3 in/1 out) additional trips occurring in the AM peak hour and 39 (26 in/13 out) additional trips occurring in the PM peak hour.

## TRIP DISTRIBUTION AND ASSIGNMENT

A single trip distribution pattern was assumed for the proposed development. It is expected that the quality restaurant will generate trips based on future population within a 10 -mile radius of the site. Future total population within a 10 -mile radius of the site, as predicted by the 2030 socio-economic data compiled by the Maricopa Association of Governments (MAG), was used as a basis to estimate trip distribution for the resort development

The resulting trip distribution percentages for the study area are shown in Table 4. The trip distribution calculations are included in Appendix E.

Table 4: Site Trip Distribution

| Direction (To/From) | Trip Distribution |
| :---: | :---: |
| East on Lincoln Drive | $37 \%$ |
| West on Lincoln Drive | $49 \%$ |
| North on Desert Fairways Drive | $2 \%$ |
| South on Camelback Manor Drive | $2 \%$ |
| South on $56^{\text {th }}$ Street | $10 \%$ |
| Total | $\mathbf{1 0 0 \%}$ |

Figure 5 illustrates the trip distribution percentages shown in Table 4 on the existing roadway network with the study area. The percentages presented in Figure 5 were applied to the site trips generated to determine the AM and PM peak hour site traffic at the intersections within the study area. The resulting site generated traffic for the proposed development are presented in Figure 6.


Figure 5: Trip Distribution
Mountain Shadows Resort - Traffic Impact Analysis


Desert Fairways Dr \& Lincoln Dr


Mountain Shadows East \& Lincoln Dr


Mountain Shadows West \& Lincoln Dr


56th St \& Lincoln Dr

XX(XX) - AM(PM) Peak Hour Traffic Volumes

- Colonia Miramonte Gated Emergency Access


Figure 6: Site Generated Traffic Volumes

## FUTURE BACKGROUND TRAFFIC

CivTech applied a growth rate to the seasonally adjusted traffic counts for this study in order to obtain the background traffic volumes along the adjacent roadway network. The Town of Paradise Valley does not publish historic traffic data; for this project, the growth rates were determined using the City of Scottsdale traffic data. In reviewing the City of Scottsdale Traffic Counts Map, a $1.7 \%$ average growth rate was found on Scottsdale Road between Indian Bend Road and Lincoln Drive. Table 5 shows the expansion factors used for the proposed opening year 2020.

Table 5: Growth Rate Expansion Factors

| Horizon Year | Expansion Factor |
| :---: | :---: |
| 2020 | 1.017 |

Applying the growth rate expansion factors to the seasonally adjusted existing traffic volumes predicts the volume of traffic anticipated on the surrounding area roads for opening year 2020. It was assumed that all vehicles making U-turns at the median break west of the site and at the intersection of Desert Fairways Drive and Lincoln Drive were negotiating the movement to return to the Mountain Shadows Resort because they had missed an earlier left-in access. Therefore, the U-turns counted at the median break west of the site and at the intersection of Desert Fairways Drive and Lincoln Drive were re-assigned as westbound left turns into the site at the Mountain Shadows West Driveway in order to determine the feasibility of a median break and the addition of westbound left turn lane.

Calculated background traffic for the opening year 2020 is included in Figure 7. More detailed calculation sheets are included in Appendix F.

## TOTAL TRAFFIC

Total traffic was determined by adding the site generated traffic to the projected background traffic. Total peak hour traffic volumes for the opening year 2020 are shown in Figure 8.


Desert Fairways Dr \& Lincoln Dr


Mountain Shadows East \& Lincoln Dr


Mountain Shadows West \& Lincoln Dr


XX(XX) - AM(PM) Peak Hour Traffic Volumes
$\star$ Colonia Miramonte Gated Emergency Access


Figure 7: 2020 Background Traffic Volumes


Desert Fairways Dr \& Lincoln Dr


Mountain Shadows East \& Lincoln Dr


Mountain Shadows West \& Lincoln Dr


XX(XX) - AM(PM) Peak Hour Traffic Volumes

- Colonia Miramonte Gated Emergency Access


Figure 8: 2020 Total Traffic Volumes

## TRAFFIC AND IMPROVEMENT ANALYSIS

## INTERSECTION CAPACITY ANALYSIS

Peak hour capacity analyses have been conducted for all of the intersections within the study area. All study area intersections were analyzed using Synchro 10.0 analysis software and the methodologies previously presented. The mitigation measures proposed for the existing conditions was applied to the future conditions. The overall intersection and approach levels of service are summarized in Table 6 for the 2020 opening year. Detailed analysis worksheets can be found in Appendix G.

Table 6: 2020 Peak Hour Analysis

|  | Intersection | Intersection Control | Approach/ Movement | 2020 LOS AM(PM) |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| ID |  |  |  | No-Build | Build |
| 1 | Desert Fairways Dr \& Lincoln Dr | Signal | NB | C(C) | C(C) |
|  |  |  | SB | C(C) | C(C) |
|  |  |  | EB | A(A) | A(A) |
|  |  |  | WB | $B(B)$ | $B(B)$ |
|  |  |  | Overall | $B(B)$ | $B(B)$ |
| 2 | Mountain Shadows West | 1-way stop | NB Right | $\mathrm{B}(\mathrm{B})$ | B(B) |
| 2 | Drwy \& Lincoln Dr | (NB) | WB Left | A(A) | A(A) |
| 3 | Mountain Shadows East | 1-way stop | NB Shared | C(A) | C(A) |
| 3 | Drwy \& Lincoln Dr | (NB) | WB Left | A(A) | A(A) |
| 4 | $56^{\text {th }}$ St \& Lincoln Dr | Signal | NB | C(C) | C(C) |
|  |  |  | SB | C(C) | C(C) |
|  |  |  | EB | A(A) | A(A) |
|  |  |  | WB | A(A) | A(A) |
|  |  |  | Overall | A(A) | A(A) |

The results of the 2020 opening year Synchro analysis summarized in Table 6 indicates that all study intersections are anticipated to experience an acceptable level of service LOS C or better.

The proposed lane configurations are presented in Figure 9.


Figure 9: Proposed Lane Configurations and Traffic Controls

## LEFT TURN LANE ANALYSIS

The Town of Paradise Valley does not provide explicit criteria for the installation of dedicated left turn lanes; they typically defer to City of Scottsdale design standards.

According to the City of Scottsdale Design Standards and Policies Manual (DS\&PM), left turn lanes should be determined based on anticipated turning volume and whether there is signalized or unsignalized traffic control. Per the DS\&PM, driveways located on arterial roadways must be spaced 330 feet apart, with a minimum spacing of 250 feet with no restrictions. The eastern and western driveways for the Mountain Shadows Resort are spaced approximately 530 feet apart, meaning that a left turn lane into the site at the western driveway could be permitted. A dedicated westbound left turn lane is also safer than making a U-turn at an unprotected signalized intersection or at a median break, which was observed to be happening during both the AM and PM peak hours.

The U-turns counted at the intersection of Desert Fairways Drive and Lincoln Drive, as well as the U-turns counted at the median break just east of this intersection, were assumed to become westbound left turns at the Mountain Shadows West Driveway. These U-turns and the site trips assigned to this driveway were used to determine the need for a left turn lane. In total, there are assumed to be approximately 16 westbound left turns at the West Driveway during the PM peak hour with 1,175 oncoming eastbound trips.

City of Scottsdale DS\&PM Chapter 5, Section 5-3.201, Driveway Spacing has been included in Appendix H. The proposed westbound left turn lane concept prepared by CivTech has been included in Appendix I.

## QUEUE LENGTH ANALYSIS

Adequate turn storage should be supplied on any approach where turn lanes are permitted and/or warranted. A queuing analysis was performed for all warranted/recommended and existing intersection turn lanes where site traffic is expected. According to the methodology documented in A Policy on Geometric Design of Highways and Streets (the AASHTO "Green Book"), the storage length for a turn lane is typically estimated as the length required to hold the average number of arriving vehicles per two minutes, where unsignalized, or per one-and-a half signal cycles, where signalized. ${ }^{1}$ The formulas used for the calculations are shown below.

For signalized intersections, the storage length is determined by the following formula:

$$
\text { Storage Length }=[1.5 \times(\mathrm{veh} / \mathrm{hr}) /(\mathrm{cycles} / \mathrm{hr})] \times 25 \text { feet }
$$

For unsignalized intersections, the storage length is determined by the following formula:
Storage Length $=[(\mathrm{veh} / \mathrm{hr}) /(30$ periods $/ \mathrm{hr})] \times 25$ feet
Using the traffic volumes and lane configurations projected for the 2020 horizon year, the resulting turn lane storage for turn movements affiliated with the site using AASHTO guidelines were calculated with a 65-second cycle length and are summarized in Table 7. Calculations for the queue storage length recommendations are provided in Appendix J.

Table 7: Queue Storage Lengths

| ID | Intersection | Intersection Control | Movement | Queue Storage |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | $\begin{gathered} \hline \text { Existing } \\ \text { (1) } \\ \hline \hline \end{gathered}$ | AASHTO | $95^{\text {th }}$ Percentile | Recommended |
| 1 | Desert Fairways <br> Dr \& Lincoln Dr | Signalized | SB Left EB Left WB Left | $\begin{array}{r} 75^{\prime} \\ 150^{\prime} \\ 55^{\prime} \\ \hline \end{array}$ | $\begin{array}{r} \hline \hline 125^{\prime} \\ 75^{\prime} \\ 25^{\prime} \\ \hline \end{array}$ | $\begin{array}{r} \hline 80^{\prime} \\ 30^{\prime} \\ <25 \\ \hline \end{array}$ | $\begin{array}{r} \hline(2) 75 \\ 150 \\ 55 \\ \hline \end{array}$ |
| 2 | Mountain Shadows West Drwy \& Lincoln Dr | 1-way stop (NB) | WB Left EB Right | 115' | $\begin{aligned} & 25 \\ & 50 \\ & \text { 50' } \end{aligned}$ | $\begin{aligned} & <25 \\ & <25 \\ & \end{aligned}$ | $\begin{gathered} \text { (3) } 75 \\ 115 \end{gathered}$ |
| 3 | Mountain Shadows East Drwy \& Lincoln Dr | 1-way stop (NB) | WB Left EB Right | $\begin{array}{r} 75 \\ 100 \end{array}$ | $\begin{aligned} & 25 \\ & 25 \end{aligned}$ | $\begin{aligned} & <25 \\ & <25 \end{aligned}$ | $\begin{array}{r} 75 \\ 100 \end{array}$ |
| 4 | $56^{\text {th }}$ St $\&$ Lincoln Dr | Signalized | EB Left WB Left NB Right | $\begin{array}{r} 45 \\ 75 \\ 78 \\ 285 \\ \hline \end{array}$ | $\begin{aligned} & \text { 50' } \\ & 50 \\ & 25^{\prime} \\ & \hline \end{aligned}$ | $\begin{array}{r} <25 \\ 30^{\prime} \\ <25 \\ \hline \end{array}$ | $\begin{array}{r} \hline 45 \\ 75^{\prime} \\ 285 \\ \hline \end{array}$ |

(1) Measured from stop bar to end of storage length
(2) Not the responsibility of the developer
(3) Minimum recommended storage length

The recommended storage lengths in Table 7 are provided for horizon year 2020 using the total traffic projections. The minimum storage length recommendation for the proposed westbound left turn lane at the Mountain Shadows West Driveway is 75 feet, or enough storage for 3 vehicles at a time.

[^0]There is an existing median break along Lincoln Drive directly east of the proposed median break at the west driveway. This median break is utilized as emergency access for the Colonia Miramonte housing development north of the Mountain Shadows Resort. Since this is a gated emergency access only not available to residents for daily usage, it was determined this median break can be utilized in the design of a westbound left turn lane into the Mountain Shadows Resort west driveway. The determination was provided by the Town based on concept designs provided by the Town's consultant as part of earlier review comments for this project. A minimum of 75 feet of storage is recommended with a 90-foot taper for a total length of 165 feet. The median located in the vicinity of the Colonia Miramonte emergency access should be constructed with mountable curb to allow access, if needed, by emergency vehicles.

## CONCLUSIONS

The following conclusions have been documented for this study.

## General

- The proposed restaurant is anticipated to generate approximately 420 additional weekday daily trips, with 4 ( $3 \mathrm{in} / 1$ out) additional trips occurring in the AM peak hour and 39 (26 in/13 out) additional trips occurring in the PM peak hour.


## Existing Conditions

- The results of the existing conditions analysis summarized in Table 2 indicates that all intersections currently operate at an overall acceptable level of service (LOS C or better) under existing lane configurations and signal timing.


## Opening Year 2020

- The results of the 2020 opening year Synchro analysis indicates that all study intersections are anticipated to experience an acceptable level of service LOS C or better.


## Left Turn Lane Analysis

- The Town of Paradise Valley does not provide explicit criteria for the installation of dedicated left turn lanes; they typically defer to City of Scottsdale design standards.
o Per the City of Scottsdale Design Standards and Policy Manual (DS\&PM), driveways located on arterial roadways must be spaced 330 feet apart, with a minimum spacing of 250 feet with no restrictions. The eastern and western driveways for the Mountain Shadows Resort are spaced approximately 530 feet apart, meaning that a left turn lane into the site at the western driveway should be permitted. A dedicated westbound left turn lane is also safer than making a U-turn at an unprotected signalized intersection or at a median break, which was observed to be happening during both the AM and PM peak hours.


## Queue Storage

- The recommended storage lengths are provided for horizon year 2020 using the total traffic projections.
o There is an existing median break along Lincoln Drive directly east of the proposed median break at the west driveway. This median break is utilized as emergency access for the Colonia Miramonte housing development north of the Mountain Shadows Resort. Since this is a gated emergency access only not available to residents for daily usage, it was
determined this median break can be utilized in the design of a westbound left turn lane into the Mountain Shadows Resort west driveway. The determination was provided by the Town based on concept designs provided by the Town's consultant as part of earlier review comments for this project. A minimum of 75 feet of storage is recommended with a 90foot taper for a total length of 165 feet.
o It is recommended that the median be constructed with mountable curb in the areas proximate to the Colonia Miramonte emergency access.


## LIST OF REFERENCES

Highway Capacity Manual. Transportation Research Board, Washington, D.C., 2000.

Manual on Uniform Traffic Control Devices. U.S. Department of Transportation, Federal Highways Administration, Washington, D.C., 2009.

Roadway Design Manual, Maricopa County Department of Transportation, Phoenix, Arizona, Revised April 2004.

Trip Generation Manual, $10^{\text {th }}$ Edition, Institute of Transportation Engineers, Washington, D.C., 2016.

Trip Generation Handbook, $3^{\text {nd }}$ Edition, Institute of Transportation Engineers, Washington, D.C., 2016.

## TECHNICAL APPENDIX

## APPENDIX A: REVIEW COMMENTS AND RESPONSES

## APPENDIX B: EXISTING TRAFFIC COUNTS

APPENDIX C: EXISTING PEAK HOUR ANALYSIS
APPENDIX D: TRIP GENERATION
APPENDIX E: TRIP DISTRIBUTION
APPENDIX F: BACKGROUND TRAFFIC
APPENDIX G: 2020 PEAK HOUR ANALYSIS
APPENDIX H: SCOTTSDALE DS\&PM SECTION 5-3.201
APPENDIX I: LEFT TURN LANE DESIGN CONCEPT
APPENDIX J: QUEUE STORAGE ANALYSIS
APPENDIX K: SUP ACCESS DIAGRAMS

## APPENDIX A

## REVIEW COMMENTS AND RESPONSES

## CivTech

## 2nd Submittal

Disposition Codes:
(1) Will Comply
(2) Will Evaluate
(3) Delete Comment
(4) Defer to Consultant/Owner

Reviewer Name, Agency: Kimley Horn on behalf of Town of Paradise Valley

| Item | Review Comment | (Code) \& Response |
| :---: | :---: | :---: |
| 1. | Pg 1: Queue storage - based on assessor, this access [Marriott Resort emergency access] falls within the Colonia Miamonte Owners Association | (1) Marriott Resort reference was changed to Colonia Miramonte housing development. |
| 2. | Pg 1: Queue Storage - Using ADOT's criterian for warranting left turn median break is really not appropriate. Access will be based on spacing, volumes, safety, proximity to other driveways and proximity to other signalized intersection. Like COS DS\&PM indicated, the Transportation Research Board (TRB), Access Management manual would be a more appropriate document to utilize in the absence of access related criteria. It should also be noted that ADOT has access criteria for median break locations. Main concern is how the proposed left turn will work with the existing emergency access location. | (2) City of Scottsdale Design Standards and Policies Manual (DS\&PM) was used to detrmine driveway spacing requirements instead of determining the need for a left turn lane using ADOT criteria. The recommendation for this turn lane is also based on the safety of vehicles since many vehicles now are making U-turns at Desert Fairways/Lincoln and Lincoln is not neccessarily wide enough to be making this movement. Analysis proposed for the access indicate acceptable operations and sight distance. The emergency access will continue to be operational, but will now be restricted by a median, as shown in the latest median design plans being prepared by Kimley Horn. A mountalbe median is recommended in this location. |
| 3. | Pg 2: Queue storage - This is not an accurate statement as emergency response may come from either direction depending on which fire station responds based on call volume and if existing calls for service are responded to. | (1) Statement was revised to indicate that emergency vehicles could approch the access from either the east or the west |
| 4. | Pg 2: Queue storage - see comments in Figure 3, the volumes are not balancing between access \#2 and \#3. Looking at aerials, I do not see any other driveways or opportunities for u-turn maneuvers. For this reason, I am wondering if there is more traffic utilizing the emergency access? | (1) Volumes were balanced between both access points and 56th Street intersection on Lincoln Drive. The emergency access is gated without an opportunity for interim use by the residents. |
| 5. | Pg 2: Queue storage - How would left turns out of Mountain Shadows be prohibited? | (1) Left turns out of the site will be prohibited by the median design, an Appendix K has been added to show the full design and the site plan figure will be updated with the median design. |
| 6. | Pg 5: change "Marriott Resort" to "CM HOA and El Chorro Restaurant" | (1) Reference in text was changed |
| 7. | Pg 5: Desert Fairways has a posted speed limit of 25 mph , Camelback Manor has no posted speed limit | (1) Speed limit for Desert Fairways Drive was updated |

## CivTech

Appendix A

## 2nd Submittal

Disposition Codes:
(1) Will Comply
(2) Will Evaluate
(3) Delete Comment
(4) Defer to Consultant/Owner

Reviewer Name, Agency: Kimley Horn on behalf of Town of Paradise Valley

| Item | Review Comment | (Code) \& Response |
| :---: | :---: | :---: |
| 8. | Pg 5: change "a private property" to "El Chorro Restaurant" | (1) Reference in text was changed |
| 9. | Pg 6: Please verify seasonal factor with MAG. The most recent MAG volume map (2015) does not provide factor and the moset recent MAG map with adjust is 2011 and says seasonal factor is 1.037 | (3) After discussion with Paul Mood from the Town of Paradise Valley, it was agreed that the City of Scottsdale seasonal adjustment factor could be used in place of the MAG adjustment factor. An adjustment factor of 0.93 was used per City of Scottsdale DS\&PM. |
| "'masme | Figqure $2:$ shmwew |  |
| "'112" | Figure 3: The volumes between the intersections do not add up or balance. I am especially questioning what is occuring between access \#2 and \#3. Is there more traffic using the emergency access? | (1) Volumes were balanced between both access points and 56th Street intersection on Lincoln Drive. The emergency access is gated without an opportunity for interim use by the residents. |
| "'"'s"', | Figure 3: Clarify - is the 6 u-turns in addition to are part of the 5 lefts? Synchro output show 5 lefts. | (1) The 6 U-turns observed at the intersection of Desert Fairways Drive and Lincoln Drive are not included in the existing westbound left turn volume presented in Figure 3. The U-turns were included as westbound left turns at the western Mountain Shadows driveway for the background and total scenarios. |
| 13. | Pg 10: LOS Dorw LOS C? | (1) LOS D was changed to LOS C |
| 14.' | Pg 10: SUP access diagram has primary access off of 56th Street | (1) Text was added to indicate that Paul Mood has agreed to allow CivTech to not analyze the access off of 56th Street since the addition of the left turn will not change the operation of anticipated volumes at the driveway on 56th Street. |
| 15. | Pg 10: indicate both access points are off of Lincoln Drive | (1) Text included to indicate that both access points that were analyzed are on Lincoln Drive |
| 16.'s. |  include in traffic analysis. | (3) Text was added to indicate that Paul Mood has agreed to allow CivTech to not analyze the access off of 56th Street since the addition of the left turn will not change the operation of anticipated volumes at the driveway on 56th Street. |
| "'"'s"', | Pg 10: provide layout of proposed left turn median break | (1) Figure 4 will be updated to show the proposed left turn median break on Licoln Drive. The concept design is also included in Appendix K. |

Appendix A

## 2nd Submittal

Disposition Codes:
(1) Will Comply
(2) Will Evaluate
(3) Delete Comment
(4) Defer to Consultant/Owner

Reviewer Name, Agency: Kimley Horn on behalf of Town of Paradise Valley

| Item | Review Comment | (Code) \& Response |
| :---: | :---: | :---: |
| 18. | Figure 4: show 56th Street access point and add dimensions between the access points | (1) Dimensions have been added to Figure 4 showing the driveway spacing |
| 19. | Figure 4: labeled currently at Figure 5 | (1) Figure label has been updated to show correct figure number |
| 20. | Figure 6: show site generated traffic at 56th Street access point | (3) Text was added to indicate that Paul Mood has agreed to allow CivTech to not analyze the access off of 56th Street since the addition of the left turn will not change the operation of anticipated volumes at the driveway on 56th Street. |
| 21. | Figure 6: with a driveway (\#3) close to the proposed restaurant, why is right turn traffic using \#2 versus \#3 to gain access to the restaurant? They have to drive through parking lot to get to site? | (2) While most vehicles will be making right turns at the second access on Lincoln Drive, some vehicles will see the first access and choose to turn there without knowledge that there is a second access point on Lincoln Drive. This is typical of driver behavior. Moving right turn vehicles from access 2 to access 3 does not impact the operational analysis of either intersection. |
| 22. | Pg 16: Were all the u-turns actually observed using the driveway or is this just an assumption? | (1) Text was added to indicate that all U-turns were assumed to be making right turns into the Mountain Shadows Resort and that this was an assumption. |
| 23. | Pg 21: Town of Paradise Valley "typically" defers to City of Scottsdale | (1) "'Typically" was added to text to be more descriptive. |
| 24. | Pg 21: As previously commented, this is access control. City of Scottsdale and other agency do have criteria on access/driveway spacing and requires for median breaks. In the absence of criteria associated with access use TRB | (1) City of Scottsdale access control standards were utilized instead of ADOT criteria. |
| 25. | Pg 21: U-turn vehicles actually observed going to mountain shadows west driveway? |  been added to clarify the assumption. |
| 26. | Pg 22: 75 feet recommended in table, 50 feet recomen"wnew Update text to recommend 75 feet | (1) Text was updated recommending 75 feet of queue storage. |
| 27. | Pg 22: update Marriott Resort references to CM HOA | (1) Reference changed. |
| 28. | Pg 23: LOS C or LOD D? | (1) LOS D was changed to LOS C. |
| 29. | Pg 23: Was City of Phoenix turn lane criteria discussed in report? | (2) This statement was deleted as it was not relevent to the discussion in the TIA body text. |

## CivTech

Appendix A

## 2nd Submittal

Disposition Codes: (1) Will Comply (2) Will Evaluate (3) Delete Comment (4) Defer to Consultant/Owner
Reviewer Name, Agency: Kimley Horn on behalf of Town of Paradise Valley

| Item Review Comment | (Code) \& Response |
| :---: | :---: |
| 30. Pg 23: Left Turn Lane Analysis - Access will be dependent on what occurs with the Colonia Mariamonte emergency access and treatment for closure and maintaining emergency access. | (2) Discussion was added in the text addressing emergency access. |

## APPENDIX B

## EXISTING TRAFFIC COUNTS

## CivTech

## Project \#: 19-1139-001

## TMC SUMMARY OF Desert Fairways Dr. \& Lincoln Dr.



## Intersection Turning Movement Prepared by:

## veracitytraffic group

N-S STREET: Desert Fairways Dr.
E-W STREET: Lincoln Dr.

DATE: 03/07/19
DAY: THURSDAY

LOCATION: Paradise Valley
PROJECT\# 19-1139-001

| LANES: | NORTHBOUND |  |  | SOUTHBOUND |  |  | EASTBOUND |  |  | WESTBOUND |  |  | TOTAL |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | NL 0 | $\begin{gathered} \text { NT } \\ 1 \end{gathered}$ | NR 0 | SL 1 | $\begin{aligned} & \text { ST } \\ & 0.5 \end{aligned}$ | SR 0.5 | EL 1 | $\begin{gathered} \text { ET } \\ 2 \end{gathered}$ | ER 0 | $\begin{gathered} \text { WL } \\ 1 \end{gathered}$ | $\begin{gathered} \text { WT } \\ 2 \end{gathered}$ | WR |  |
| 6:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:15 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:30 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:45 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7:00 AM | 2 | 0 | 0 | 14 | 0 | 6 | 11 | 307 | 0 | 3 | 239 | 6 | 588 |
| 7:15 AM | 1 | 0 | 3 | 16 | 0 | 24 | 10 | 317 | 0 | 2 | 252 | 13 | 638 |
| 7:30 AM | 1 | 0 | 4 | 15 | 0 | 16 | 21 | 375 | 0 | 3 | 303 | 18 | 756 |
| 7:45 AM | 0 | 0 | 0 | 22 | 0 | 16 | 17 | 326 | 1 | 0 | 268 | 14 | 664 |
| 8:00 AM | 0 | 0 | 0 | 25 | 0 | 7 | 16 | 338 | 1 | 0 | 280 | 10 | 677 |
| 8:15 AM | 1 | 1 | 1 | 18 | 1 | 8 | 18 | 320 | 2 | 2 | 256 | 17 | 645 |
| 8:30 AM | 0 | 0 | 3 | 21 | 0 | 18 | 19 | 338 | 1 | 3 | 239 | 19 | 661 |
| 8:45 AM | 1 | 0 | 3 | 17 | 1 | 3 | 25 | 327 | 1 | 2 | 242 | 21 | 643 |
| 9:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 9:15 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 9:30 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 9:45 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10:15 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10:30 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10:45 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11:15 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11:30 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11:45 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |


| TOTAL | NL | NT | NR | SL | ST | SR | EL | ET | ER | WL | WT | WR | TOTAL |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| VolumesApproach \%App/Depart | 6 | 1 | 14 | 148 | 2 | 98 | 137 | 2648 | 6 | 15 | 2079 | 118 | 5272 |
|  | 28.57 | 4.76 | 66.67 | 59.68 | 0.81 | 39.52 | 4.91 | 94.88 | 0.21 | 0.68 | 93.99 | 5.33 |  |
|  | 21 | / | 256 | 248 | / | 23 | 2791 | / | 2810 | 2212 | / | 2183 |  |

AM Peak Hr Begins at: 730 AM
PEAK

| Volumes | 2 | 1 | 5 | 80 | 1 | 47 | 72 | 1359 | 4 | 5 | 1107 | 59 | 2742 |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Approach \% | 25.00 | 12.50 | 62.50 | 62.50 | 0.78 | 36.72 | 5.02 | 94.70 | 0.28 | 0.43 | 94.53 | 5.04 |  | PEAK HR.

FACTOR: $\left\lvert\, \begin{array}{llllllll}1 & 0.400 & 0.842 & 0.906 & 0.904 & \mid & 0.907\end{array}\right.$
CONTROL:
Signal
COMMENT 1:
GPS:
33.531053, -111.966596

## I ntersection Turning Movement

## Field Data Services of Arizona, Inc. 520.316.6745

veracitytraffic group

| N-S STREET: | Desert Fairways Dr. | DATE: 03/07/19 | LOCATION: Paradise Valley |
| :--- | :--- | :---: | :--- |
| E-W STREET: | Lincoln Dr. |  | DAY: THURSDAY |


| LANES: | NORTHBOUND |  |  | SOUTHBOUND |  |  | EASTBOUND |  |  | WESTBOUND |  |  | TOTAL |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{gathered} \mathrm{NL} \\ 0 \end{gathered}$ | $\begin{gathered} \text { NT } \\ \hline \end{gathered}$ | $\begin{gathered} \text { NR } \\ 0 \end{gathered}$ | $\begin{gathered} \mathrm{SL} \\ 1 \end{gathered}$ | $\begin{aligned} & \text { ST } \\ & 0.5 \end{aligned}$ | $\begin{aligned} & \text { SR } \\ & 0.5 \end{aligned}$ | $\begin{gathered} \mathrm{EL} \\ 1 \end{gathered}$ | $\begin{gathered} \text { ET } \\ 2 \end{gathered}$ | $\begin{gathered} \text { ER } \\ 0 \end{gathered}$ | WL | $\begin{gathered} \text { WT } \\ 2 \end{gathered}$ | $\begin{gathered} \text { WR } \\ 0 \end{gathered}$ |  |
| 1:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1:15 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1:30 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1:45 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2:15 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2:30 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2:45 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3:15 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3:30 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3:45 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4:00 PM | 0 | 1 | 1 | 18 | 1 | 15 | 22 | 245 | 2 | 1 | 313 | 30 | 649 |
| 4:15 PM | 0 | 1 | 1 | 19 | 0 | 22 | 18 | 274 | 0 | 1 | 366 | 20 | 722 |
| 4:30 PM | 0 | 1 | 1 | 20 | 0 | 22 | 10 | 289 | 0 | 2 | 383 | 32 | 760 |
| 4:45 PM | 2 | 0 | 0 | 28 | 1 | 18 | 20 | 267 | 2 | 0 | 345 | 50 | 733 |
| 5:00 PM | 0 | 1 | 2 | 48 | 0 | 24 | 17 | 285 | 1 | 2 | 313 | 40 | 733 |
| 5:15 PM | 2 | 0 | 2 | 24 | 1 | 26 | 20 | 313 | 0 | 1 | 319 | 59 | 767 |
| 5:30 PM | 1 | 0 | 1 | 43 | 1 | 23 | 21 | 305 | 1 | 1 | 294 | 34 | 725 |
| 5:45 PM | 1 | 0 | 1 | 30 | 0 | 22 | 19 | 282 | 0 | 3 | 246 | 31 | 635 |
| 6:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:15 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:30 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:45 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |


| TOTAL | NL | NT | NR | SL | ST | SR | EL | ET | ER | WL | WT | WR | TOTAL |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Volumes | 6 | 4 | 9 | 230 | 4 | 172 | 147 | 2260 | 6 | 11 | 2579 | 296 | 5724 |
| Approach \% | 31.58 | 21.05 | 47.37 | 56.65 | 0.99 | 42.36 | 6.09 | 93.66 | 0.25 | 0.38 | 89.36 | 10.26 |  |
| App/Depart | 19 | 1 | 447 | 406 | 1 | 21 | 2413 | 1 | 2499 | 2886 | 1 | 2757 |  |

PM Peak Hr Begins at: $\quad 430$ PM
PEAK

| Volumes | 4 | 2 | 5 | 120 | 2 | 90 | 67 | 1154 | 3 | 5 | 1360 | 181 | 2993 |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Approach \% | 36.36 | 18.18 | 45.45 | 56.60 | 0.94 | 42.45 | 5.47 | 94.28 | 0.25 | 0.32 | 87.97 | 11.71 |  |

PEAK HR.
FACTOR
0.688
0.736
| 0.919
| 0.927
0.976

CONTROL: Signal
COMMENT 1: 0
GPS:
33.531053, -111.966596

## Project \#: 19-1139-001

## TMC SUMMARY OF Desert Fairways Dr. \& Lincoln Dr.



## Intersection Turning Movement

 Prepared by:
## veracitytraffic group

N-S STREET: Desert Fairways Dr.
DATE: 03/07/19
LOCATION: Paradise Valley
E-W STREET: Lincoln Dr.
DAY: THURSDAY
PROJECT\# 19-1139-001

|  | NORTHBOUND |  |  | SOUTHBOUND |  |  | EASTBOUND |  |  | WESTBOUND |  |  | TOTAL |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| LANES: | $\begin{gathered} \mathrm{NL} \\ 0 \end{gathered}$ | $\begin{gathered} \text { NT } \\ 1 \end{gathered}$ | $\begin{gathered} \text { NR } \\ 0 \end{gathered}$ | SL 1 | $\begin{aligned} & \text { ST } \\ & 0.5 \end{aligned}$ | $\begin{aligned} & \text { SR } \\ & 0.5 \end{aligned}$ | $\begin{gathered} \mathrm{EL} \\ 1 \end{gathered}$ | $\begin{gathered} \text { ET } \\ 2 \end{gathered}$ | $\begin{gathered} \text { ER } \\ 0 \end{gathered}$ | $\begin{gathered} \text { WL } \\ \hline \end{gathered}$ | $\begin{gathered} \text { WT } \\ 2 \end{gathered}$ | $\begin{gathered} \text { WR } \\ 0 \end{gathered}$ |  |
| 6:00 AM | U-TURNS ONLY |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:15 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:30 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:45 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 2 |
| 7:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 3 |
| 7:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| 7:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 |
| 8:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 |
| 8:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 |
| 8:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| 8:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| 9:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 9:15 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
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| 10:45 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
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| 11:15 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11:30 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11:45 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |


| TOTAL | NL | NT | NR | SL | ST | SR | EL | ET | ER | WL | WT | WR | TOTAL |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Volumes | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 8 | 0 | 0 | 8 |
| Approach $\%$ | $\# \# \# \#$ | $\# \# \# \#$ | $\# \# \# \#$ | $\# \# \# \#$ | $\# \# \# \#$ | $\# \# \# \#$ | $\# \# \# \#$ | $\# \# \# \#$ | $\# \# \# \#$ | 100.00 | 0.00 | 0.00 |  |
|  | App/Depart | 0 | 1 | 0 | 0 | 1 | 8 | 0 | 1 | 0 | 8 | 1 | 0 |

AM Peak Hr Begins at: 700 AM
PEAK

| Volumes | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 6 | 0 | 0 | 6 |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Approach \% | \#\#\#\# | $0 \# \# \#$ | $\# \# \# \#$ | $\# \# \# \#$ | $\# \# \# \#$ | $\# \# \# \#$ | $\# \# \# \#$ | $\# \# \# \#$ | $\# \# \# \#$ | 100.00 | 0.00 | 0.00 |  | PEAK HR.

FACTOR: $\left\lvert\, \begin{array}{llllllll} & 0.000 & 0.000 & 0.000 & 0.500 & \mid & 0.500\end{array}\right.$
CONTROL:
Signal
COMMENT 1:
GPS:
33.531053, -111.966596

## I ntersection Turning Movement

## Field Data Services of Arizona, Inc. 520.316.6745

## veracitytraffic group

| N-S STREET: | Desert Fairways Dr. | DATE: 03/07/19 | LOCATION: Paradise Valley |
| :--- | :--- | :---: | :--- |
| E-W STREET: | Lincoln Dr. |  | DAY: THURSDAY |


|  | NORTHBOUND |  |  | SOUTHBOUND |  |  | EASTBOUND |  |  | WESTBOUND |  |  | TOTAL |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| LANES: | $\begin{gathered} \text { NL } \\ 0 \end{gathered}$ | $\begin{gathered} \text { NT } \\ 1 \end{gathered}$ | $\begin{gathered} \text { NR } \\ 0 \end{gathered}$ | SL 1 | $\begin{aligned} & \text { ST } \\ & 0.5 \end{aligned}$ | $\begin{aligned} & \text { SR } \\ & 0.5 \end{aligned}$ | $\begin{gathered} \mathrm{EL} \\ 1 \end{gathered}$ | $\begin{gathered} \text { ET } \\ 2 \end{gathered}$ | $\begin{gathered} \text { ER } \\ 0 \end{gathered}$ | $\begin{gathered} \text { WL } \\ 1 \end{gathered}$ | $\begin{gathered} \text { WT } \\ 2 \end{gathered}$ | $\begin{gathered} \text { WR } \\ 0 \end{gathered}$ |  |
| 1:00 PM | U-TURNS ONLY |  |  |  |  |  |  |  |  |  |  |  |  |
| 1:15 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1:30 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1:45 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2:15 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2:30 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2:45 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3:15 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3:30 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3:45 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| 4:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| 4:30 PM | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 2 |
| 4:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 |
| 5:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 2 |
| 5:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 1 |
| 5:30 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 2 |
| 5:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 2 |
| 6:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:15 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:30 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:45 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |


| TOTAL | NL | NT | NR | SL | ST | SR | EL | ET | ER | WL | WT | WR | TOTAL |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Volumes | $\begin{array}{ccc}0 & 0 & 0 \\ \# \# \# \# \# \# \# \# \# \# ~\end{array}$ |  |  | 0 | 0 | 0 | 2 | 0 | 0 | 8 | 0 | 0 | 10 |
| Approach \% |  |  |  | \#\#\#\# | \#\#\#\# | \#\#\#\# | 100.00 | 0.00 | 0.00 | 100.00 | 0.00 | 0.00 |  |
| App/Depart | 0 | / | 2 | 0 | / | 8 | 2 | / | 0 | 8 | / | 0 |  |

PM Peak Hr Begins at: 500 PM
PEAK
 Approach \% $\mid$ \#\#\#\# \#\#\#\# \#\#\#\#|"\#\#\# \#\#\#\# \#\#\#\#|100.00 00.00

PEAK HR.
FACTOR
CONTROL: Signal
COMMENT 1: 0
GPS:
33.531053, -111.966596

## Project \#: 19-1139-002

## TMC SUMMARY OF Mountain Shadows West \& Lincoln Dr.



## veracitytraffic group

N-S STREET: Mountain Shadows West
DATE: 03/07/19
LOCATION: Paradise Valley
E-W STREET: Lincoln Dr.
DAY: THURSDAY
PROJECT\# 19-1139-002

| LANES: | NORTHBOUND |  |  | SOUTHBOUND |  |  | EASTBOUND |  |  | WESTBOUND |  |  | TOTAL |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{gathered} \mathrm{NL} \\ 0 \end{gathered}$ | $\begin{gathered} \text { NT } \\ 0 \end{gathered}$ | NR | SL 0 | ST 0 | SR 0 | EL 0 | $\begin{gathered} \text { ET } \\ 2 \end{gathered}$ | ER 1 | $\begin{gathered} \text { WL } \\ 0 \end{gathered}$ | $\begin{gathered} \text { WT } \\ 2 \end{gathered}$ | WR |  |
| 6:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:15 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:30 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:45 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7:00 AM | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 308 | 7 | 0 | 228 | 0 | 544 |
| 7:15 AM | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 338 | 7 | 0 | 262 | 0 | 612 |
| 7:30 AM | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 346 | 6 | 0 | 299 | 0 | 654 |
| 7:45 AM | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 391 | 5 | 0 | 295 | 0 | 694 |
| 8:00 AM | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 370 | 10 | 0 | 272 | 0 | 656 |
| 8:15 AM | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 322 | 4 | 0 | 310 | 0 | 640 |
| 8:30 AM | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 341 | 4 | 0 | 247 | 0 | 596 |
| 8:45 AM | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 374 | 6 | 0 | 289 | 0 | 674 |
| 9:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 9:15 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 9:30 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 9:45 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10:15 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10:30 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10:45 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11:15 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11:30 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11:45 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |


| TOTAL | NL | NT | NR | SL | ST | SR | EL | ET | ER | WL | WT | WR | TOTAL |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Volumes | 0 | $\begin{array}{cc} \hline 0 & 29 \\ 0.00 & 100.00 \end{array}$ |  | 0 0 0 <br> \#\#\#\# \#\#\#\# \#\#\# |  |  | $\begin{aligned} & \hline 0 \\ & 0.00 \end{aligned}$ | $\begin{aligned} & 2790 \\ & 98.27 \end{aligned}$ | 49 | $\begin{array}{lc} \hline 0 & 2202 \\ 0.00 & 100.00 \end{array}$ |  | $\begin{array}{l\|} \hline 0 \\ 0.00 \\ \hline \end{array}$ | 5070 |
| Approach \% | 0.00 |  |  | 1.73 |  |  |  |  |  |  |
| App/Depart | 29 | / | 0 |  |  |  | 0 | 1 | 49 | 2839 | 1 | 2819 |  | 2202 | 1 | 2202 |

AM Peak Hr Begins at: 730 AM
PEAK

| Volumes | 0 | 0 | 14 | 0 | 0 | 0 | 0 | 1429 | 25 | 0 | 1176 | 0 |
| :--- | :--- | :--- | :--- | :---: | :---: | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Approach \% | 0.00 | 0.00 | 100.00 | $\# \# \# \#$ | $\# \# \# \#$ | $\# \# \# \#$ | 0.00 | 98.28 | 1.72 | 0.00 | 100.00 | 0.00 | PEAK HR.


CONTROL: 1-Way Stop (NB)
COMMENT 1:
GPS: 33.531052,-111.963798

## I ntersection Turning Movement

## Field Data Services of Arizona, Inc. 520.316.6745

## verac itytraffic group

N-S STREET: Mountain Shadows West

DATE: 03/07/19
DAY: THURSDAY

LOCATION: Paradise Valley

PROJECT\# 19-1139-002

| LANES: | NORTHBOUND |  |  | SOUTHBOUND |  |  | EASTBOUND |  |  | WESTBOUND |  |  | TOTAL |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{gathered} \mathrm{NL} \\ 0 \end{gathered}$ | $\begin{gathered} \text { NT } \\ 0 \end{gathered}$ | $\begin{gathered} \text { NR } \\ 1 \end{gathered}$ | SL 0 | ST 0 | $\begin{gathered} \text { SR } \\ 0 \end{gathered}$ | EL | $\begin{gathered} \text { ET } \\ 2 \end{gathered}$ | $\begin{gathered} \text { ER } \\ 1 \end{gathered}$ | $\begin{gathered} \text { WL } \\ 0 \end{gathered}$ | $\begin{gathered} \text { WT } \\ 2 \end{gathered}$ | $\begin{gathered} \text { WR } \\ 0 \end{gathered}$ |  |
| 1:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1:15 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1:30 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1:45 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2:15 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2:30 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2:45 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3:15 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3:30 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3:45 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4:00 PM | 0 | 0 | 7 | 0 | 0 | 0 | 0 | 238 | 3 | 0 | 338 | 0 | 586 |
| 4:15 PM | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 289 | 6 | 0 | 387 | 0 | 685 |
| 4:30 PM | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 292 | 11 | 0 | 379 | 0 | 687 |
| 4:45 PM | 0 | 0 | 12 | 0 | 0 | 0 | 0 | 297 | 8 | 0 | 418 | 0 | 735 |
| 5:00 PM | 0 | 0 | 9 | 0 | 0 | 0 | 0 | 321 | 9 | 0 | 393 | 0 | 732 |
| 5:15 PM | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 325 | 9 | 0 | 384 | 0 | 722 |
| 5:30 PM | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 327 | 11 | 0 | 298 | 0 | 641 |
| 5:45 PM | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 313 | 16 | 0 | 304 | 0 | 635 |
| 6:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:15 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:30 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:45 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |


| TOTAL | NL | NT | NR | SL | ST | SR | EL | ET | ER | WL | WT | WR | TOTAL |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Volumes | 0 | 0.0010000 |  | 0 0 0 <br> \#\#\#\# \#\#\#\# \#\#\#\#  |  |  | $\begin{aligned} & \hline 0 \\ & 0.00 \end{aligned}$ | $\begin{aligned} & \hline 2402 \\ & 97.05 \\ & \hline \end{aligned}$ | 73 | $\begin{aligned} & \hline 0 \\ & 0.00 \end{aligned}$ | 2901 | $\begin{aligned} & 0 \\ & 0.00 \end{aligned}$ | 5423 |
| Approach \% | 0.00 |  |  | 2.95 | 100.00 |  |  |  |  |  |
| App/Depart | 47 | / | 0 |  |  |  | 0 | 1 | 73 | 2475 | 1 | 2449 |  | 2901 | 1 | 2901 |

PM Peak Hr Begins at: 430 PM
PEAK

| Volumes | 0 | 0 | 30 | 0 | 0 | 0 | 0 | 1235 | 37 | 0 | 1574 | 0 |
| :--- | :--- | :--- | :---: | :---: | :---: | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| 2876 |  |  |  |  |  |  |  |  |  |  |  |  |
| Approach \% | 0.00 | 0.00 | 100.00 | \#\#\#\# \#\#\#\# \#\#\#\# | 0.00 | 97.09 | 2.91 | 0.00 | 100.00 | 0.00 |  |  | PEAK HR.



CONTROL: 1-Way Stop (NB)
COMMENT 1: 0
GPS:
33.531052, -111.963798

## Project \#: 19-1139-003

TMC SUMMARY OF Mountain Shadows East \& Lincoln Dr.


## veracitytraffic group

N-S STREET: Mountain Shadows East
DATE: 03/07/19
LOCATION: Paradise Valley
E-W STREET: Lincoln Dr.
DAY: THURSDAY
PROJECT\# 19-1139-003

| LANES: | NORTHBOUND |  |  | SOUTHBOUND |  |  | EASTBOUND |  |  | WESTBOUND |  |  | TOTAL |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{gathered} \mathrm{NL} \\ 0 \end{gathered}$ | $\begin{gathered} \text { NT } \\ 1 \end{gathered}$ | NR 0 | SL 0 | ST 0 | $\begin{gathered} \text { SR } \\ 0 \end{gathered}$ | EL | $\begin{gathered} \text { ET } \\ 2 \end{gathered}$ | ER 1 | $\begin{gathered} \text { WL } \\ 1 \end{gathered}$ | $\begin{gathered} \text { WT } \\ 2 \end{gathered}$ | $\begin{gathered} \text { WR } \\ 0 \end{gathered}$ |  |
| 6:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:15 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:30 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:45 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7:00 AM | 7 | 0 | 1 | 0 | 0 | 0 | 0 | 307 | 9 | 3 | 211 | 0 | 538 |
| 7:15 AM | 4 | 0 | 2 | 0 | 0 | 0 | 0 | 330 | 10 | 2 | 260 | 0 | 608 |
| 7:30 AM | 7 | 0 | 4 | 0 | 0 | 0 | 0 | 309 | 9 | 4 | 289 | 0 | 622 |
| 7:45 AM | 3 | 0 | 3 | 0 | 0 | 0 | 0 | 381 | 8 | 4 | 288 | 0 | 687 |
| 8:00 AM | 10 | 0 | 3 | 0 | 0 | 0 | 0 | 359 | 7 | 3 | 275 | 0 | 657 |
| 8:15 AM | 5 | 0 | 5 | 0 | 0 | 0 | 0 | 327 | 5 | 1 | 310 | 0 | 653 |
| 8:30 AM | 3 | 0 | 2 | 0 | 0 | 0 | 0 | 334 | 11 | 4 | 246 | 0 | 600 |
| 8:45 AM | 2 | 0 | 5 | 0 | 0 | 0 | 0 | 357 | 6 | 1 | 281 | 0 | 652 |
| 9:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 9:15 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 9:30 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 9:45 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10:15 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10:30 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10:45 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11:15 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11:30 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11:45 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |


| TOTAL | NL | NT | NR | SL | ST | SR | EL | ET | ER | WL | WT | WR | TOTAL |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| VolumesApproach \%App/Depart | 41 | 0 | 25 | 0 | 0 | 0 | 0 | 2704 | 65 | 22 | 2160 | 0 | 5017 |
|  | 62.12 | 0.00 | 37.88 | \#\#\#\# | \#\#\#\# | \#\#\#\# | 0.00 | 97.65 | 2.35 | 1.01 | 98.99 | 0.00 |  |
|  | 66 | I | 0 | 0 | 1 | 87 | 2769 | / | 2729 | 2182 | / | 2201 |  |

AM Peak Hr Begins at: 730 AM
PEAK

| Volumes | 25 | 0 | 15 | 0 | 0 | 0 | 0 | 1376 | 29 | 12 | 1162 | 0 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| 2619 | 2619 |  |  |  |  |  |  |  |  |  |  |  |
| Approach \% | 62.50 | 0.00 | 37.50 | \#\#\#\# \#\#\#\# \#\#\#\# | 0.00 | 97.94 | 2.06 | 1.02 | 98.98 | 0.00 |  |  | PEAK HR.



```
CONTROL: 1-Way Stop (NB)
COMMENT 1:
GPS: 33.531052,-111.963798
```


## I ntersection Turning Movement

## Field Data Services of Arizona, Inc. 520.316.6745

## veracitytraffic group

N-S STREET: Mountain Shadows East

DATE: 03/07/19
DAY: THURSDAY

LOCATION: Paradise Valley

PROJECT\# 19-1139-003

| LANES: | NORTHBOUND |  |  | SOUTHBOUND |  |  | EASTBOUND |  |  | WESTBOUND |  |  | TOTAL |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{gathered} \mathrm{NL} \\ 0 \end{gathered}$ | $\begin{gathered} \text { NT } \\ 1 \end{gathered}$ | NR 0 | SL 0 | ST 0 | $\begin{gathered} \text { SR } \\ 0 \end{gathered}$ | EL | $\begin{gathered} \text { ET } \\ 2 \end{gathered}$ | $\begin{gathered} \text { ER } \\ 1 \end{gathered}$ | $\begin{gathered} \text { WL } \\ 1 \end{gathered}$ | $\begin{gathered} \text { WT } \\ 2 \end{gathered}$ | $\begin{gathered} \text { WR } \\ 0 \end{gathered}$ |  |
| 1:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1:15 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1:30 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1:45 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2:15 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2:30 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2:45 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3:15 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3:30 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3:45 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4:00 PM | 8 | 0 | 2 | 0 | 0 | 0 | 0 | 236 | 5 | 7 | 321 | 0 | 579 |
| 4:15 PM | 5 | 0 | 4 | 0 | 0 | 0 | 0 | 287 | 2 | 5 | 373 | 0 | 676 |
| 4:30 PM | 4 | 0 | 7 | 0 | 0 | 0 | 0 | 284 | 3 | 3 | 383 | 0 | 684 |
| 4:45 PM | 5 | 0 | 4 | 0 | 0 | 0 | 0 | 304 | 6 | 2 | 425 | 0 | 746 |
| 5:00 PM | 4 | 0 | 11 | 0 | 0 | 0 | 0 | 323 | 5 | 6 | 400 | 0 | 749 |
| 5:15 PM | 2 | 0 | 9 | 0 | 0 | 0 | 0 | 335 | 6 | 3 | 369 | 0 | 724 |
| 5:30 PM | 4 | 0 | 4 | 0 | 0 | 0 | 0 | 329 | 3 | 7 | 392 | 0 | 739 |
| 5:45 PM | 1 | 0 | 4 | 0 | 0 | 0 | 0 | 313 | 8 | 14 | 401 | 0 | 741 |
| 6:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:15 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:30 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:45 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |


| TOTAL | NL | NT | NR | SL | ST | SR | EL | ET | ER | WL | WT | WR | TOTAL |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Volumes | 33 | 0 | 45 | 0 | 0 | 0 | 0 | 2411 | 38 | 47 | 3064 | 0 | 5638 |
| Approach \% | 42.31 | 0.00 | 57.69 | \#\#\#\# | \#\#\#\# | \#\#\#\# | 0.00 | 98.45 | 1.55 | 1.51 | 98.49 | 0.00 |  |
| App/Depart | 78 | 1 | 0 | 0 | 1 | 85 | 2449 | / | 2456 | 3111 | 1 | 3097 |  |

PM Peak Hr Begins at: 445 PM
PEAK

| Volumes | 15 | 0 | 28 | 0 | 0 | 0 | 0 | 1291 | 20 | 18 | 1586 | 0 | 2958 |
| :--- | :--- | :--- | :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Approach \% | 34.88 | 0.00 | 65.12 | \#\#\#\# | \#\#\#\# \#\#\#\# | 0.00 | 98.47 | 1.53 | 1.12 | 98.88 | 0.00 |  |  | PEAK HR. FACTOR $\square$ 0.000 | 0.961

0.939

CONTROL: 1-Way Stop (NB)
COMMENT 1: 0
GPS:
33.531052, -111.963798

## Project \#: 19-1139-004

TMC SUMMARY OF 56th St. \& Lincoln Dr.


## Intersection Turning Movement Prepared by:

$\mathcal{F}_{\text {verac itytraffic group }}$

N-S STREET: 56th St.
DATE: 03/07/19
LOCATION: Paradise Valley
E-W STREET: Lincoln Dr.
DAY: THURSDAY
PROJECT\# 19-1139-004

| LANES: | NORTHBOUND |  |  | SOUTHBOUND |  |  | EASTBOUND |  |  | WESTBOUND |  |  | TOTAL |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{gathered} \mathrm{NL} \\ 0 \end{gathered}$ | $\begin{gathered} \text { NT } \\ 1 \end{gathered}$ | $\begin{gathered} \text { NR } \\ 0 \end{gathered}$ | SL 0 | $\begin{gathered} \mathrm{ST} \\ 1 \end{gathered}$ | SR 0 | EL 1 | $\begin{gathered} \text { ET } \\ 2 \end{gathered}$ | ER 0 | $\begin{gathered} \text { WL } \\ 1 \end{gathered}$ | $\begin{gathered} \text { WT } \\ 2 \end{gathered}$ | $\begin{gathered} \text { WR } \\ 0 \end{gathered}$ |  |
| 6:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:15 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:30 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:45 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7:00 AM | 8 | 0 | 3 | 0 | 0 | 1 | 3 | 287 | 16 | 3 | 209 | 0 | 530 |
| 7:15 AM | 10 | 0 | 3 | 0 | 0 | 0 | 1 | 311 | 16 | 7 | 252 | 0 | 600 |
| 7:30 AM | 6 | 0 | 6 | 1 | 0 | 1 | 0 | 303 | 13 | 5 | 286 | 2 | 623 |
| 7:45 AM | 10 | 0 | 4 | 0 | 0 | 1 | 5 | 373 | 16 | 7 | 285 | 1 | 702 |
| 8:00 AM | 8 | 0 | 5 | 1 | 0 | 1 | 1 | 345 | 12 | 1 | 265 | 1 | 640 |
| 8:15 AM | 11 | 1 | 1 | 2 | 0 | 0 | 1 | 329 | 10 | 2 | 299 | 2 | 658 |
| 8:30 AM | 11 | 0 | 3 | 1 | 0 | 1 | 0 | 322 | 13 | 7 | 238 | 0 | 596 |
| 8:45 AM | 14 | 0 | 6 | 2 | 0 | 0 | 2 | 347 | 21 | 8 | 274 | 0 | 674 |
| 9:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 9:15 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 9:30 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 9:45 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10:15 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10:30 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10:45 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11:15 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11:30 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 11:45 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |


| TOTAL | NL | NT | NR | SL | ST | SR | EL | ET | ER | WL | WT | WR | TOTAL |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Volumes | 78 | 1 | 31 | 7 | 0 | 5 | 13 | 2617 | 117 | 40 | 2108 | 6 | 5023 |
| Approach \% | 70.91 | 0.91 | 28.18 | 58.33 | 0.00 | 41.67 | 0.47 | 95.27 | 4.26 | 1.86 | 97.86 | 0.28 |  |
| App/Depart | 110 | I | 20 | 12 | 1 | 157 | 2747 | 1 | 2655 | 2154 | 1 | 2191 |  |

AM Peak Hr Begins at: 730 AM
PEAK

| Volumes | 35 | 1 | 16 | 4 | 0 | 3 | 7 | 1350 | 51 | 15 | 1135 | 6 | 2623 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Approach \% | 67.31 | 1.92 | 30.77 | 57.14 | 0.00 | 42.86 | 0.50 | 95.88 | 3.62 | 1.30 | 98.18 | 0.52 |  | PEAK HR.


CONTROL:
COMMENT 1:
GPS: 33.531070,-111.960495

## I ntersection Turning Movement

## Field Data Services of Arizona, Inc. 520.316.6745

veracitytraffic group

N-S STREET: 56th St. DATE: 03/07/19
0
E-W STREET: Lincoln Dr.
DAY: THURSDAY

LOCATION: Paradise Valley
PROJECT\# 19-1139-004

| LANES: | NORTHBOUND |  |  | SOUTHBOUND |  |  | EASTBOUND |  |  | WESTBOUND |  |  | TOTAL |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{gathered} \mathrm{NL} \\ 0 \end{gathered}$ | NT | NR 0 | SL 0 | $\begin{gathered} \text { ST } \\ 1 \end{gathered}$ | $\begin{gathered} \text { SR } \\ 0 \end{gathered}$ | $\begin{gathered} \mathrm{EL} \\ 1 \end{gathered}$ | $\begin{gathered} \text { ET } \\ 2 \end{gathered}$ | $\begin{gathered} \text { ER } \\ 0 \end{gathered}$ | $\begin{gathered} \text { WL } \\ 1 \end{gathered}$ | $\begin{gathered} \text { WT } \\ 2 \end{gathered}$ | $\begin{gathered} \text { WR } \\ 0 \end{gathered}$ |  |
| 1:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1:15 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1:30 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1:45 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2:15 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2:30 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2:45 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3:15 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3:30 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3:45 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4:00 PM | 11 | 0 | 8 | 3 | 0 | 1 | 4 | 225 | 9 | 1 | 338 | 2 | 602 |
| 4:15 PM | 10 | 0 | 2 | 2 | 0 | 3 | 9 | 279 | 3 | 5 | 389 | 6 | 708 |
| 4:30 PM | 18 | 0 | 5 | 1 | 0 | 1 | 6 | 280 | 6 | 10 | 385 | 8 | 720 |
| 4:45 PM | 21 | 2 | 2 | 0 | 1 | 3 | 11 | 287 | 10 | 6 | 423 | 9 | 775 |
| 5:00 PM | 15 | 0 | 6 | 2 | 0 | 6 | 8 | 316 | 14 | 10 | 403 | 10 | 790 |
| 5:15 PM | 15 | 0 | 12 | 0 | 1 | 5 | 9 | 333 | 8 | 24 | 372 | 6 | 785 |
| 5:30 PM | 9 | 0 | 13 | 2 | 0 | 1 | 3 | 325 | 6 | 13 | 309 | 5 | 686 |
| 5:45 PM | 10 | 3 | 8 | 2 | 0 | 1 | 10 | 301 | 7 | 17 | 327 | 9 | 695 |
| 6:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:15 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:30 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:45 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |


| TOTAL | NL | NT | NR | SL | ST | SR | EL | ET | ER | WL | WT | WR | TOTAL |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Volumes | 109 | 5 | 56 | 12 | 2 | 21 | 60 | 2346 | 63 | 86 | 2946 | 55 | 5761 |
| Approach \% | 64.12 | 2.94 | 32.94 | 34.29 | 5.71 | 60.00 | 2.43 | 95.02 | 2.55 | 2.79 | 95.43 | 1.78 |  |
| App/Depart | 170 | / | 120 | 35 | / | 151 | 2469 | I | 2414 | 3087 | 1 | 3076 |  |

PM Peak Hr Begins at: $\quad 430$ PM
PEAK

| Volumes | 69 | 2 | 25 | 3 | 2 | 15 | 34 | 1216 | 38 | 50 | 1583 | 33 | 3070 |
| :--- | :--- | :--- | :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Approach \% | 71.88 | 2.08 | 26.04 | 15.00 | 10.00 | 75.00 | 2.64 | 94.41 | 2.95 | 3.00 | 95.02 | 1.98 |  |

PEAK HR.

CONTROL: Signal
COMMENT 1: 0
GPS:
33.531070, -111.960495

## APPENDIX C

## EXISTING PEAK HOUR ANALYSIS

## CivTech



| Mountain Shadows Existing AM | Resort |  |  | : Can | lbac | Mano |  | sert HCM | $\begin{aligned} & \text { airwa } \\ & \text { S Signal } \end{aligned}$ | s Dr red Inte | Lin <br> ection |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | $\rightarrow$ | 7 | 7 | $\leftarrow$ |  |  | 4 |  | $\downarrow$ | $\downarrow$ | $\checkmark$ |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\uparrow{ }^{\text {¢ }}$ |  | ${ }^{7}$ | $\uparrow{ }^{\text {¢ }}$ |  |  | ¢ |  | \% | $\dagger$ |  |
| Traffic Volume (veh/h) | 67 | 1264 | 4 | 5 | 1030 | 55 | 2 | , | 5 | 74 | 1 | 44 |
| Future Volume (veh/h) | 67 | 1264 | 4 | 5 | 1030 | 55 | 2 | 1 | 5 | 74 | 1 | 44 |
| Initial $\mathrm{Q}(\mathrm{Qb})$, veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach |  | No |  |  | No |  |  | No |  |  | No |  |
| Adj Sat Flow, veh/h/n | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1945 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 74 | 1389 | 0 | , | 1144 | 28 | 5 | 2 | 2 | 88 | 1 | 28 |
| Peak Hour Factor | 0.91 | 0.91 | 0.91 | 0.90 | 0.90 | 0.90 | 0.40 | 0.40 | 0.40 | 0.84 | 0.84 | 0.84 |
| Percent Heavy Veh, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 346 | 2554 | 0 | 320 | 2169 | 53 | 151 | 59 | 35 | 249 | 5 | 149 |
| Arrive On Green | 0.05 | 0.72 | 0.00 | 0.20 | 0.20 | 0.20 | 0.10 | 0.10 | 0.10 | 0.10 | 0.10 | 0.10 |
| Sat Flow, veh/h | 1781 | 3647 | 0 | 389 | 3545 | 87 | 669 | 609 | 365 | 1412 | 55 | 1538 |
| Grp Volume(v), veh/h | 74 | 1389 | 0 | - | 573 | 599 | 9 | 0 | 0 | 88 | 0 | 29 |
| Grp Sat Flow(s),veh/h/ln | 1781 | 1777 | 0 | 389 | 1777 | 1855 | 1643 | - | 0 | 1412 | 0 | 1593 |
| Q Serve(g_s), s | 0.9 | 11.7 | 0.0 | 0.8 | 18.7 | 18.7 | 0.0 | 0.0 | 0.0 | 3.5 | 0.0 | 1.1 |
| Cycle Q Clear(g_c), s | 0.9 | 11.7 | 0.0 | 5.6 | 18.7 | 18.7 | 0.3 | 0.0 | 0.0 | 3.8 | 0.0 | 1.1 |
| Prop In Lane | 1.00 |  | 0.00 | 1.00 |  | 0.05 | 0.56 |  | 0.22 | 1.00 |  | 0.97 |
| Lane Grp Cap(c), veh/h | 346 | 2554 | 0 | 320 | 1087 | 1135 | 245 | 0 | 0 | 249 | 0 | 154 |
| V/C Ratio(X) | 0.21 | 0.54 | 0.00 | 0.02 | 0.53 | 0.53 | 0.04 | 0.00 | 0.00 | 0.35 | 0.00 | 0.19 |
| Avail Cap(c_a), veh/h | 375 | 2554 | 0 | 320 | 1087 | 1135 | 674 | - | 0 | 634 | 0 | 588 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 0.33 | 0.33 | 0.33 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(1) | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.00 | 0.00 | 1.00 | 0.00 | 1.00 |
| Uniform Delay (d), s/veh | 7.2 | 4.2 | 0.0 | 14.3 | 17.5 | 17.5 | 26.7 | 0.0 | 0.0 | 28.2 | 0.0 | 27.0 |
| Incr Delay (d2), s/veh | 0.1 | 0.8 | 0.0 | 0.1 | 1.8 | 1.8 | 0.1 | 0.0 | 0.0 | 0.9 | 0.0 | 0.6 |
| Initial Q Delay (d3),S/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \%ile Q BackOfQ(50\%), veh/ln 0.2 2.3 0.0 0.1 9.1 9.5 0.1 0.0 0.0 1.3 0.0 0.4 <br> Unsig. Movement Delay, s/veh             |  |  |  |  |  |  |  |  |  |  |  |  |
| Unsig. Movement Delay, s/veh |  |  |  |  |  |  |  |  |  |  |  |  |
| LnGrp Delay(d),s/veh | 7.3 | 5.1 | 0.0 | 14.4 | 19.4 | 19.3 | 26.7 | 0.0 | 0.0 | 29.1 | 0.0 | 27.6 |
| LnGrp LOS | A | A | A | B | B | B | c | A | A | C | A | C |
| Approach Vol, veh/h |  | 1463 |  |  | 1178 |  |  | , |  |  | 117 |  |
| Approach Delay, s/veh |  | 5.2 |  |  | 19.3 |  |  | 26.7 |  |  | 28.7 |  |
| Approach LOS |  | A |  |  | B |  |  | C |  |  | C |  |
| Timer - Assigned Phs | 1 | 2 |  | 4 |  | 6 |  | 8 |  |  |  |  |
| Phs Duration ( $G+Y+R \mathrm{C})$, s | 6.9 | 45.8 |  | 12.3 |  | 52.7 |  | 12.3 |  |  |  |  |
| Change Period ( $Y+R \mathrm{C}$ ), s | 4.0 | 6.0 |  | 6.0 |  | 6.0 |  | 6.0 |  |  |  |  |
| Max Green Setting (Gmax), s | 4.0 | 21.0 |  | 24.0 |  | 29.0 |  | 24.0 |  |  |  |  |
| Max Q Clear Time ( $\left.\mathrm{g}_{\text {c }} \mathrm{c}+11\right)$, s | 2.9 | 20.7 |  | 2.3 |  | 13.7 |  | 5.8 |  |  |  |  |
| Green Ext Time (p_c), s | 0.0 | 0.2 |  | 0.0 |  | 8.5 |  | 0.3 |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| HCM 6th Ctrl Delay 12.3 <br> HCM 6th LOS B |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |

11/15/2019 Synchro 10 Report

Civech


Mountain Shadows Resort Existing AM


## 11/15/2019

Synchro 10 Repor
Page


| Mountain Shadows Resort Existing AM |  |  |  |  |  |  |  | 4: 56th St/56th St \& Lincoln Dr HCM 6th Signalized Intersection Summary |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | $\rightarrow$ | 7 | $t$ | $\leftarrow$ | 4 | 4 | $\uparrow$ |  | $\checkmark$ | $\downarrow$ | $\downarrow$ |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 7 | $\uparrow{ }^{\text {¢ }}$ |  | \% | $\uparrow{ }^{\text {¢ }}$ |  |  | $\uparrow$ | 「 |  | \$ |  |
| Traffic Volume (veh/h) | 7 | 1275 | 47 | 14 | 1056 | 6 | 33 | 1 | 15 | 4 | , |  |
| Future Volume (veh/h) | 7 | 1275 | 47 | 14 | 1056 | 6 | 33 | 1 | 15 | 4 | 0 | 3 |
| Initial $Q(Q b)$, veh | 0 | 0 | 0 | , | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Ped-Bike Adj(A_pbT) | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach |  | No |  |  | No |  |  | No |  |  | No |  |
| Adj Sat Flow, veh/h/n | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 8 | 1433 | 25 | 15 | 1112 | 2 | 35 | 1 | 7 | 5 | 0 |  |
| Peak Hour Factor | 0.89 | 0.89 | 0.89 | 0.95 | 0.95 | 0.95 | 0.93 | 0.93 | 0.93 | 0.88 | 0.88 | 0.88 |
| Percent Heavy Veh, \% | 2 | 2 | 2 | 2 | 2 |  | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 431 | 2663 | 46 | 320 | 2712 | 5 | 202 | 4 | 99 | 171 | 0 | 0 |
| Arrive On Green | 0.75 | 0.75 | 0.75 | 0.75 | 0.75 | 0.75 | 0.06 | 0.06 | 0.06 | 0.06 | 0.00 | 0.00 |
| Sat Flow, veh/h | 506 | 3574 | 62 | 364 | 3639 | 7 | 1489 | 69 | 1585 | 973 | , | 0 |
| Grp Volume(v), veh/h | 8 | 712 | 746 | 15 | 543 | 571 | 36 | 0 | 7 | 5 | 0 | 0 |
| Grp Sat Flow(s),veh/h/n | 506 | 1777 | 1859 | 364 | 1777 | 1869 | 1558 | 0 | 1585 | 973 | 0 | 0 |
| Q Serve(g_s), s | 0.4 | 11.1 | 11.1 | 1.2 | 7.3 | 7.3 | 0.0 | 0.0 | 0.3 | 0.2 | 0.0 | 0.0 |
| Cycle Q Clear(g_c), s | 7.7 | 11.1 | 11.1 | 12.3 | 7.3 | 7.3 | 1.3 | 0.0 | 0.3 | 1.5 | 0.0 | 0.0 |
| Prop In Lane | 1.00 |  | 0.03 | 1.00 |  | 0.00 | 0.97 |  | 1.00 | 1.00 |  | 0.00 |
| Lane Grp Cap(c), veh/h | 431 | 1324 | 1386 | 320 | 1324 | 1393 | 206 | 0 | 99 | 172 | 0 | 0 |
| V/C Ratio(X) | 0.02 | 0.54 | 0.54 | 0.05 | 0.41 | 0.41 | 0.17 | 0.00 | 0.07 | 0.03 | 0.00 | 0.00 |
| Avail Cap(c_a), veh/h | 431 | 1324 | 1386 | 320 | 1324 | 1393 | 622 | - | 561 | 582 | 0 | 0 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(1) | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 0.00 | 0.00 |
| Uniform Delay (d), s/veh | 4.4 | 3.5 | 3.5 | 6.1 | 3.0 | 3.0 | 29.2 | 0.0 | 28.7 | 29.9 | 0.0 | 0.0 |
| Incr Delay (d2), s/veh | 0.1 | 1.6 | 1.5 | 0.3 | 0.9 | 0.9 | 0.4 | 0.0 | 0.3 | 0.1 | 0.0 | 0.0 |
| Initial Q Delay (d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \%oile BackOfQ(50\%), veh/ln | 0.0 | 2.1 | 2.1 | 0.1 | 1.3 | 1.4 | 0.5 | 0.0 | 0.1 | 0.1 | 0.0 | 0.0 |
| Unsig. Movement Delay, s/veh |  |  |  |  |  |  |  |  |  |  |  |  |
| LnGrp Delay(d),s/veh | 4.5 | 5.1 | 5.0 | 6.4 | 4.0 | 3.9 | 29.6 | 0.0 | 29.0 | 29.9 | 0.0 | 0.0 |
| LnGrp LOS | A | A | A | A | A | A | C | A | C | C | A | A |
| Approach Vol, veh/h |  | 1466 |  |  | 1129 |  |  | 43 |  |  | 5 |  |
| Approach Delay, s/veh |  | 5.1 |  |  | 4.0 |  |  | 29.5 |  |  | 29.9 |  |
| Approach LOS |  | A |  |  | A |  |  | C |  |  | C |  |
| Timer - Assigned Phs |  | 2 |  | 4 |  | 6 |  | 8 |  |  |  |  |
| Phs Duration ( $G+Y+R \mathrm{C})$, S |  | 54.9 |  | 10.1 |  | 54.9 |  | 10.1 |  |  |  |  |
| Change Period ( $Y+R \mathrm{C}$ ), s |  | 6.5 |  | 6.0 |  | 6.5 |  | 6.0 |  |  |  |  |
| Max Green Setting (Gmax), s |  | 29.5 |  | 23.0 |  | 29.5 |  | 23.0 |  |  |  |  |
| Max Q Clear Time ( $\mathrm{g}_{\text {c }} \mathrm{c}+11$ ), s |  | 14.3 |  | 3.3 |  | 13.1 |  | 3.5 |  |  |  |  |
| Green Ext Time (p_c), s |  | 6.3 |  | 0.1 |  | 8.9 |  | 0.0 |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| HCM 6th Crtr Delay |  |  | 5.0 |  |  |  |  |  |  |  |  |  |
| HCM 6th LOS |  |  | A |  |  |  |  |  |  |  |  |  |

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| Mountain Shadows Existing PM | Resort |  |  | : Can |  |  |  | sert <br> HCM | airwa S Signali | s Dr <br> ed Int | Lin <br> ection |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | $\rightarrow$ | 7 | 7 | $\leftarrow$ | 4 | 4 | $\uparrow$ |  | $\checkmark$ | $\downarrow$ | $\downarrow$ |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | $\uparrow{ }^{\text {¢ }}$ |  | \% | $\uparrow{ }^{\text {¢ }}$ |  |  | ¢ |  | \% | A |  |
| Traffic Volume (veh/h) | 62 | 1073 | 3 |  | 1265 | 168 | 4 | 2 | 5 | 112 | , | 84 |
| Future Volume (veh/h) | 62 | 1073 | 3 | 5 | 1265 | 168 | 4 | 2 | 5 | 112 |  | 84 |
| Initial $Q(Q b)$, veh | 0 | , | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach |  | No |  |  | No |  |  | No |  |  | No |  |
| Adj Sat Flow, veh/h/n | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1945 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 67 | 1166 | 0 | 5 | 1360 | 95 | 6 | 3 | 3 | 151 | 3 | 73 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.93 | 0.93 | 0.93 | 0.69 | 0.69 | 0.69 | 0.74 | 0.74 | 0.74 |
| Percent Heavy Veh, \% | 2 | 2 | 2 | 2 | 2 |  | 2 | 2 |  | 2 | 2 |  |
| Cap, veh/h | 275 | 2429 | 0 | 365 | 1950 | 136 | 152 | 75 | 48 | 313 | 8 | 202 |
| Arrive On Green | 0.04 | 0.68 | 0.00 | 0.39 | 0.39 | 0.39 | 0.13 | 0.13 | 0.13 | 0.13 | 0.13 | 0.13 |
| Sat Flow, veh/h | 1781 | 3647 | 0 | 481 | 3370 | 235 | 521 | 570 | 364 | 1410 | 63 | 1532 |
| Grp Volume(v), veh/h | 67 | 1166 | 0 | 5 | 715 | 740 | 12 | 0 | 0 | 151 | 0 | 76 |
| Grp Sat Flow(s),veh/h/n | 1781 | 1777 | 0 | 481 | 1777 | 1828 | 1454 | 0 | 0 | 1410 | - | 1595 |
| Q Serve(g_s), s | 0.9 | 10.0 | 0.0 | 0.4 | 21.9 | 22.1 | 0.0 | 0.0 | 0.0 | 3.1 | 0.0 | 2.8 |
| Cycle Q Clear(g_c), s | 0.9 | 10.0 | 0.0 | 3.7 | 21.9 | 22.1 | 2.8 | 0.0 | 0.0 | 5.9 | 0.0 | 2.8 |
| Prop In Lane | 1.00 |  | 0.00 | 1.00 |  | 0.13 | 0.50 |  | 0.25 | 1.00 |  | 0.96 |
| Lane Grp Cap(c), veh/h | 275 | 2429 | 0 | 365 | 1028 | 1058 | 275 | 0 | 0 | 313 | 0 | 210 |
| V/C Ratio(X) | 0.24 | 0.48 | 0.00 | 0.01 | 0.70 | 0.70 | 0.04 | 0.00 | 0.00 | 0.48 | 0.00 | 0.36 |
| Avail Cap(c_a), veh/h | 308 | 2429 | 0 | 365 | 1028 | 1058 | 644 | , | 0 | 648 | 0 | 589 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 0.67 | 0.67 | 0.67 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(1) | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.00 | 0.00 | 1.00 | 0.00 | 1.00 |
| Uniform Delay (d), s/veh | 9.5 | 4.8 | 0.0 | 10.6 | 15.1 | 15.1 | 24.7 | 0.0 | 0.0 | 26.9 | 0.0 | 25.7 |
| Incr Delay (d2), s/veh | 0.2 | 0.7 | 0.0 | 0.1 | 3.9 | 3.8 | 0.1 | 0.0 | 0.0 | 1.2 | 0.0 | 1.0 |
| Initial Q Delay (d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \%oile BackOfQ(50\%), veh/ln | 0.3 | 2.3 | 0.0 | 0.0 | 9.8 | 10.1 | 0.2 | 0.0 | 0.0 | 2.3 | 0.0 | 1.1 |
| Unsig. Movement Delay, slveh |  |  |  |  |  |  |  |  |  |  |  |  |
| LnGrp Delay(d),s/veh | 9.7 | 5.5 | 0.0 | 10.6 | 19.0 | 19.0 | 24.7 | 0.0 | 0.0 | 28.0 | 0.0 | 26.8 |
| LnGrp LOS | A | A | A | B | B | B | C | A | A | C | A | C |
| Approach Vol, veh/h |  | 1233 |  |  | 1460 |  |  | 12 |  |  | 227 |  |
| Approach Delay, s/veh |  | 5.8 |  |  | 19.0 |  |  | 24.7 |  |  | 27.6 |  |
| Approach LOS |  | A |  |  | B |  |  | C |  |  | C |  |
| Timer - Assigned Phs | 1 | 2 |  | 4 |  | 6 |  | 8 |  |  |  |  |
| Phs Duration ( $G+Y+R \mathrm{C})$, S | 6.8 | 43.6 |  | 14.6 |  | 50.4 |  | 14.6 |  |  |  |  |
| Change Period ( $Y+R \mathrm{C}$ ), s | 4.0 | 6.0 |  | 6.0 |  | 6.0 |  | 6.0 |  |  |  |  |
| Max Green Setting (Gmax), s | 4.0 | 21.0 |  | 24.0 |  | 29.0 |  | 24.0 |  |  |  |  |
| Max Q Clear Time ( $\mathrm{g}_{\text {c }} \mathrm{c}+11$ ), s | 2.9 | 24.1 |  | 4.8 |  | 12.0 |  | 7.9 |  |  |  |  |
| Green Ext Time (p_c), s | 0.0 | 0.0 |  | 0.0 |  | 7.5 |  | 0.8 |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| HCM 6th Crtr Delay |  |  | 14.1 |  |  |  |  |  |  |  |  |  |
| HCM 6th LOS |  |  | B |  |  |  |  |  |  |  |  |  |

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| Mountain Shadows Resort Existing PM |  |  |  |  |  |  |  | 4: 56th St/56th St \& Lincoln Dr HCM 6th Signalized Intersection Summary |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | $\rightarrow$ | 7 | 7 | $\leftarrow$ | 4 | 4 | $\uparrow$ |  |  | $\downarrow$ | $\downarrow$ |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\uparrow{ }^{\text {¢ }}$ |  | \% | $\uparrow{ }^{\text {¢ }}$ |  |  | $\uparrow$ | 「 |  | \$ |  |
| Traffic Volume (veh/h) | 32 | 1160 | 35 | 47 | 1414 | 31 | 64 | 2 | 23 | 3 | , | 14 |
| Future Volume (veh/h) | 32 | 1160 | 35 | 47 | 1414 | 31 | 64 | 2 | 23 | 3 | 2 | 14 |
| Initial $Q(Q b)$, veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach |  | No |  |  | No |  |  | No |  |  | No |  |
| Adj Sat Flow, veh/h/n | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 35 | 1261 | 27 | 49 | 1488 | 22 | 72 | 2 | 15 | 5 | 3 | 14 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.95 | 0.95 | 0.95 | 0.89 | 0.89 | 0.89 | 0.63 | 0.63 | 0.63 |
| Percent Heavy Veh, \% | 2 | 2 | 2 | 2 | 2 |  | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 288 | 2542 | 54 | 417 | 2561 | 38 | 240 | 5 | 148 | 85 | 38 | 96 |
| Arrive On Green | 1.00 | 1.00 | 1.00 | 0.71 | 0.71 | 0.71 | 0.09 | 0.09 | 0.09 | 0.09 | 0.09 | 0.09 |
| Sat Flow, veh/h | 347 | 3557 | 76 | 429 | 3585 | 53 | 1406 | 56 | 1585 | 179 | 411 | 1033 |
| Grp Volume(v), veh/h | 35 | 630 | 658 | 49 | 737 | 773 | 74 | - | 15 | 22 | 0 | 0 |
| Grp Sat Flow(s),veh/h/n | 347 | 1777 | 1857 | 429 | 1777 | 1861 | 1462 | 0 | 1585 | 1623 | 0 | 0 |
| Q Serve(g_s), s | 2.2 | 0.0 | 0.0 | 2.4 | 13.2 | 13.2 | 2.1 | 0.0 | 0.6 | 0.0 | 0.0 | 0.0 |
| Cycle Q Clear(g_c), s | 15.4 | 0.0 | 0.0 | 2.4 | 13.2 | 13.2 | 3.0 | 0.0 | 0.6 | 0.8 | 0.0 | 0.0 |
| Prop In Lane | 1.00 |  | 0.04 | 1.00 |  | 0.03 | 0.97 |  | 1.00 | 0.23 |  | 0.64 |
| Lane Grp Cap(c), veh/h | 288 | 1270 | 1327 | 417 | 1270 | 1330 | 245 | 0 | 148 | 219 | 0 |  |
| V/C Ratio(X) | 0.12 | 0.50 | 0.50 | 0.12 | 0.58 | 0.58 | 0.30 | 0.00 | 0.10 | 0.10 | 0.00 | 0.00 |
| Avail Cap(c_a), veh/h | 288 | 1270 | 1327 | 417 | 1270 | 1330 | 633 | , | 585 | 652 | 0 | 0 |
| HCM Platoon Ratio | 2.00 | 2.00 | 2.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(1) | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 0.00 | 0.00 |
| Uniform Delay (d), s/veh | 2.2 | 0.0 | 0.0 | 3.0 | 4.5 | 4.5 | 28.0 | 0.0 | 27.0 | 27.1 | 0.0 | 0.0 |
| Incr Delay (d2), s/veh | 0.9 | 1.4 | 1.3 | 0.6 | 1.9 | 1.9 | 0.7 | 0.0 | 0.3 | 0.2 | 0.0 | 0.0 |
| Initial Q Delay (d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \%oile BackOfQ(50\%), veh/ln | 0.1 | 0.5 | 0.5 | 0.2 | 3.0 | 3.1 | 1.1 | 0.0 | 0.2 | 0.3 | 0.0 | 0.0 |
| Unsig. Movement Delay, s/veh |  |  |  |  |  |  |  |  |  |  |  |  |
| LnGrp Delay(d),s/veh | 3.0 | 1.4 | 1.3 | 3.6 | 6.5 | 6.4 | 28.7 | 0.0 | 27.3 | 27.3 | 0.0 | 0.0 |
| LnGrp LOS | A | A | A | A | A | A | c | A | C | C | A | A |
| Approach Vol, veh/h |  | 1323 |  |  | 1559 |  |  | 89 |  |  | 22 |  |
| Approach Delay, s/veh |  | 1.4 |  |  | 6.3 |  |  | 28.5 |  |  | 27.3 |  |
| Approach LOS |  | A |  |  | A |  |  | C |  |  | C |  |
| Timer - Assigned Phs |  | 2 |  | 4 |  | 6 |  | 8 |  |  |  |  |
| Phs Duration ( $G+Y+R \mathrm{C})$, S |  | 52.9 |  | 12.1 |  | 52.9 |  | 12.1 |  |  |  |  |
| Change Period ( $Y+R \mathrm{C}$ ), s |  | 6.5 |  | 6.0 |  | 6.5 |  | 6.0 |  |  |  |  |
| Max Green Setting (Gmax), s |  | 28.5 |  | 24.0 |  | 28.5 |  | 24.0 |  |  |  |  |
| Max Q Clear Time ( $\mathrm{g}_{\text {c }} \mathrm{c}+11$ ), s |  | 15.2 |  | 5.0 |  | 17.4 |  | 2.8 |  |  |  |  |
| Green Ext Time (p_c), s |  | 8.4 |  | 0.3 |  | 6.3 |  | 0.1 |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| HCM 6th Crtr Delay |  |  | 5.0 |  |  |  |  |  |  |  |  |  |
| HCM 6th LOS |  |  | A |  |  |  |  |  |  |  |  |  |

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## APPENDIX D

## TRIP GENERATION

## CivTech

## Westeroc Mountain Shadows

This form facilitates trip generation estimation using data within the Institute of Transportation Engineer's (ITE) Trip Generation Manual , 10th Edition and methodology described within ITE Generation Handbook, 3rd Edition. These references will be referred to as Manual and Handbook, respectively. The Manual contains data collected by various transportation professiona wide range of different land uses, with each land use category represented by a land use code (LUC). Average rates and equations have been established that correlate the relationship b an independent variable that describes the development size and generated trips for each categorized LUC in various settings and time periods. The Handbook indicates an established methodology for how to use data contained within the Manual when to use the fitted curve instead of the average rate and when to adjustments to the volume of trips are appropriate and $r$ do so. The methodology steps are represented visually in boxes in Figure 3.1. This worksheet applies calculations for each box if applicable
Box 1 - Define Study Site Land Use Type \& Site Characteristics, Box 2 - Define Site Context and Box 3 - Define Analysis Objectives Types of Trips \& Time Period
The analyst is to pick an appropriate LUC(s) based on the subject's zoning/land use(s)/future land use(s). The size of the land use(s) is described in reference to an independent variable(s specific to (each) the land use (example: 1,000 square feet of building area is relatively common).

Context assessment is to "simply determine whether the study sites is in a multimodal setting" and "could have persons accessing the site by walking, bicycling, or riding transit." This assessment is used in Box 4. The Manual separates data into 4 setting categories - Rural, General Urban/Suburban, Dense Multi-Urban Use and Center City Core. This worksheet us the following abbreviations, respectively: $\boldsymbol{R}, \boldsymbol{G}, \boldsymbol{D}$, and $\boldsymbol{C}$. The Manual does not have data for all settings of all land use codes. The "General Urban/Suburban" setting is used by default. This tool will focus on vehicular trips for a 24 -hour period on a typical weekday as well as its AM peak hour and PM peak hour. Other time period(s) may be of interest.
Land Use Types and Size

| Proposed Use | Amount Units | ITE LUC | ITE Land Use Name |
| :---: | :---: | :---: | :---: |
| Quality Restaurant | $5.000 \quad 1,000$ square feet | 931 |  |

Equation Type: Equation Used [Equated Rate] (Type Abbreviations: Weighted Average Rate ("WA"), Fitted Curve Type: Equation Used [Equated Rate]

| Proposed Use | ADT | AM Peak Hour | PM Peak Hour | (not used) |
| :---: | :---: | :---: | :---: | :---: |
| Quality Restaurant | WA: T $=X^{*} 83.84[83.84]$ | WA: T=X*0.73 [0.73] | WA: T=X*7.8 [7.80] |  |

Box 5/Box 9 - Estimate Baseline Trips/Estimate Vehicular Trips (Apply Equations and in/out Distributions)
Baseline Vehicular Trips

|  | ADT |  |  |  | AM Peak Hour |  |  |  | PM Peak Hour |  |  |  | (not used) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Proposed Use | \% In | In | Out | Total | \% In | In | Out | Total | \% In | In | Out | Total |  |
| Quality Restaurant | 50\% | 210 | 210 | 420 | 80\% | 3 | 1 | 4 | 67\% | 26 | 13 | 39 |  |
| Totals |  | 210 | 210 | 420 |  | 3 | 1 | 4 |  | 26 | 13 | 39 |  |

## APPENDIX E

## TRIP DISTRIBUTION

## CivTech

Mountain Shadows Resort
Trip Distribution - Summaries


Population: 10-mile Radius


## Mountain Shadows Resort

| 10-mile Radius |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| RAZ | MPA | $2020$ <br> Population | $2030$ <br> Population | $\begin{aligned} & \text { \% of } \\ & \text { TAZ } \end{aligned}$ | $2020$ <br> Adjusted | 2030 <br> Adjusted | RAZ | MPA | $2020$ <br> Population | $2030$ <br> Population | $\begin{aligned} & \text { \% of } \\ & \text { TAZ } \end{aligned}$ | $2020$ <br> Adjusted | $2030$ <br> Adjusted |
| NNW |  |  |  |  |  |  | NNE |  |  |  |  |  |  |
| 280 | GO | 52,543 | 52,543 | 50\% | 26,272 | 26,272 | 291 | ME | 56,729 |  | 5\% | 2,836 |  |
| 290 | ME | 90,250 | 90,250 | 70\% | 63,175 | 63,175 | 298 | ME | 56,065 |  | 80\% | 44,852 |  |
| 297 | TE | 52,336 | 52,336 | 5\% | 2,617 | 2,617 | 299 | ME | 41,300 |  | 80\% | 33,040 |  |
| 298 | ME | 56,065 | 56,065 | 20\% | 11,213 | 11,213 | 311 | GI | 79,589 |  | 40\% | 31,836 |  |
| 309 | ME | 52,159 | 52,159 | 90\% | 46,943 | 46,943 | 312 | GI | 36,760 |  | 75\% | 27,570 |  |
| 310 | CH | 54,854 | 54,854 | 20\% | 10,971 | 10,971 | 318 | GI | 49,414 |  | 40\% | 19,766 |  |
| 311 | GI | 79,589 | 79,589 | 60\% | 47,753 | 47,753 |  | - | - |  |  | - |  |
| 317 | CH | 34,879 | 34,879 | 20\% | 6,976 | 6,976 |  | - | - |  |  | - |  |
| 318 | GI | 49,414 | 49,414 | 20\% | 9,883 | 9,883 |  | - | - |  |  | - |  |
|  | - | - | - |  | - | - |  | - | - |  |  | - | - |
|  | - | - | - |  | - | - |  | - | - |  |  | - | - |
|  | - | - | - |  | - | - |  | - | - | - |  | - | - |
|  | - | - | - |  | - | - |  | - | - | - |  | - | - |
|  | - | - | - |  | - | - |  | - | - | - |  | - | - |
|  | - | - | - |  | - | - |  | - | - | - |  | - | - |
|  | - | - | - |  | - | - |  | - | - | - |  | - | - |
|  | - | - | - |  | - | - |  | - | - | - |  | - | - |
|  | - | - | - |  | - | - |  | - | - | - |  | - | - |
|  | - | - | - |  | - | - |  | - | - | - |  | - | - |
|  | - | - | - |  | - | - |  | - | - | - |  | - | - |
|  | - | - | - |  | - | - |  | - | - | - |  | - |  |
|  | - | - | - |  | - | - |  | - | - | - |  | - |  |
|  | - | - | - |  | - | - |  | - | - | - |  | - |  |
|  | - | - | - |  | - | - |  | - | - | - |  | - | - |
|  | - | - | - |  | - | - |  | - | - | - |  | - | - |
| From North |  |  |  |  | 225,802 | 225,802 |  |  |  |  |  | 159,900 |  |
|  |  |  |  |  |  |  |  |  |  |  |  | 385,702 | 225,802 |

## Mountain Shadows Resort

Trip Distribution - Pop from East

| 10-mile Radius |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| RAZ | MPA | $2020$ <br> Population | $2030$ <br> Population | $\begin{aligned} & \text { \% of } \\ & \text { TAZ } \end{aligned}$ | 2020 <br> Adjusted | $2030$ <br> Adjusted | RAZ | MPA | $2020$ <br> Population | $2030$ <br> Population | $\begin{aligned} & \text { \% of } \\ & \text { TAZ } \end{aligned}$ | $2020$ <br> Adjusted | $2030$ <br> Adjusted |
| ENE |  |  |  |  |  |  | ESE |  |  |  |  |  |  |
| 312 | GI | 36,760 | 36,760 | 25\% | 9,190 | 9,190 | 318 | GI | 49,414 |  | 10\% | 4,941 |  |
| 318 | GI | 49,414 | 49,414 | 20\% | 9,883 | 9,883 | 319 | GI | 80,978 |  | 40\% | 32,391 |  |
| 319 | GI | 80,978 | 80,978 | 60\% | 48,587 | 48,587 | 329 | GI | 39,078 |  | 30\% | 11,723 |  |
| 320 | ME | 2,129 | 2,129 |  | 2,129 | 2,129 | 339 | QC | 55,529 |  | 75\% | 41,647 |  |
| 321 | ME | 24,334 | 24,334 |  | 24,334 | 24,334 | 129 | PC | 18,869 |  | 10\% | 1,887 |  |
| 322 | ME | 38,353 | 38,353 | 40\% | 15,341 | 15,341 |  | - | - |  |  | - |  |
|  | - | - | - |  | - | - |  | - | - |  |  | - | - |
|  | - | - | - |  | - | - |  | - | - |  |  | - | - |
|  | - | - | - |  | - | - |  | - | - |  |  | - | - |
|  | - | - | - |  | - | - |  | - | - |  |  | - | - |
|  | - | - | - |  | - | - |  | - | - |  |  | - | - |
|  | - | - | - |  | - | - |  | - | - | - |  | - | - |
|  | - | - | - |  | - | - |  | - | - | - |  | - | - |
|  | - | - | - |  | - | - |  | - | - | - |  | - | - |
|  | - | - | - |  | - | - |  | - | - | - |  | - | - |
|  | - | - | - |  | - | - |  | - | - | - |  | - | - |
|  | - | - | - |  | - | - |  | - | - | - |  | - | - |
|  | - | - | - |  | - | - |  | - | - | - |  | - | - |
|  | - | - | - |  | - | - |  | - | - | - |  | - | - |
|  | - | - | - |  | - | - |  | - | - | - |  | - | - |
|  | - | - | - |  | - | - |  | - | - | - |  | - |  |
|  | - | - | - |  | - | - |  | - | - | - |  | - |  |
|  | - | - | - |  | - | - |  | - | - | - |  | - |  |
|  | - | - | - |  | - | - |  | - | - | - |  | - |  |
|  | - | - | - |  | - | - |  | - | - | - |  | - | - |
|  | om Ea |  |  |  | 109,464 | 109,464 |  |  |  |  |  | 92,590 |  |
|  |  |  |  |  |  |  |  |  |  |  |  | 202,053 | 109,464 |

## Mountain Shadows Resort

## Trip Distribution - Pop from South

10-mile Radius

| RAZ | MPA | 2020 <br> Population | 2030 <br> Population | \% of <br> TAZ | 2020 <br> Adjusted | 2030 <br> Adjusted |
| :---: | :---: | ---: | ---: | ---: | ---: | ---: |
| SSE |  |  |  |  |  |  |
| 328 | CH | 45,639 | 45,639 | $20 \%$ | 9,128 | 9,128 |
| 329 | GI | 39,078 | 39,078 | $70 \%$ | 27,355 | 27,355 |
| 122 | GC | 75 | 147 | $50 \%$ | 38 | 74 |
| 123 | GC | 2,045 | 4,008 | $10 \%$ | 205 | 401 |
| 129 | PC | 18,869 | 36,984 | $10 \%$ | 1,887 | 3,698 |


| RAZ | MPA | $2020$ <br> Population | $2030$ <br> Population | $\begin{aligned} & \text { \% of } \\ & \text { TAZ } \end{aligned}$ | 2020 <br> Adjusted | $\begin{gathered} 2030 \\ \text { Adjusted } \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| SSW |  |  |  |  |  |  |
| 325 | CH | 43,457 |  | 10\% | 4,346 |  |
| 327 | CH | 23,575 |  | 30\% | 7,073 |  |
| 328 | CH | 45,639 |  | 80\% | 36,511 |  |
| 120 | GC | 600 |  | 75\% | 450 |  |
| 121 | GC | - |  |  | - |  |
| 122 | GC | 75 |  | 10\% | 8 | - |
| 123 | GC | 2,045 |  | 4\% | 82 |  |
|  | - | - |  |  | - |  |
|  | - | - |  |  | - |  |
|  | - | - |  |  | - |  |
|  | - | - |  |  | - |  |
|  | - | - | - |  | - | - |
|  | - | - | - |  | - |  |
|  | - | - | - |  | - |  |
|  | - | - | - |  | - | - |
|  | - | - | - |  | - | - |
|  | - | - | - |  | - | - |
|  | - | - | - |  | - | - |
|  | - | - | - |  | - | - |
|  | - | - | - |  | - | - |
|  | - | - | - |  | - | - |
|  | - | - | - |  | - | - |
|  | - | - | - |  | - | - |
|  | - | - | - |  | - | - |
|  | - | - | - |  | - | - |
|  |  |  |  |  | 48,469 | - |
|  |  |  |  |  | 87,080 | 40,655 |

## APPENDIX F

## BACKGROUND TRAFFIC

## CivTech

# Location of counts: Scottsdale Road between Indian Bend and Lincoln 

Source(s): https://www.scottsdaleaz.gov/transportation/studies-reports/traffic-volume

|  | Year | Volume | Avg Growth <br> Rate to 2012 | Expansion <br> Factor to <br> 2012 |
| ---: | ---: | ---: | ---: | ---: |
| Beginning | 2012 | 43,500 |  |  |
| End | 2014 | 45,000 | $1.7 \%$ | 0.967 |
|  |  |  |  |  |


| Growth Rate Used | $1.7 \%$ |
| :--- | ---: |
| Per-Year Multiplier | 1.017 |


| Year | Expansion <br> Factor(s) |
| :--- | :---: |
| 2019 | 1.000 |
| 2020 | 1.017 |
| 2021 | 1.034 |
| 2022 | 1.052 |
| 2023 | 1.070 |
| 2024 | 1.088 |
| 2025 | 1.106 |
| 2026 | 1.125 |
| 2027 | 1.144 |
| 2028 | 1.164 |
| 2029 | 1.184 |
| 2030 | 1.204 |
| 2031 | 1.224 |
| 2032 | 1.245 |
| 2033 | 1.266 |
| 2034 | 1.288 |
| 2035 | 1.310 |
| 2036 | 1.332 |
| 2037 | 1.354 |
| 2038 | 1.378 |
| 2039 | 1.401 |

## APPENDIX G



| Mountain Shadows Background AM | Resort |  |  | : Can | lbac | Mano |  | sert <br> HCM | $\begin{aligned} & \text { airwa } \\ & \text { S Signal } \end{aligned}$ | s Dr red Inte | Lin <br> ection |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | $\rightarrow$ | 7 | 7 | $\leftarrow$ |  |  | 4 |  | $\checkmark$ | $\downarrow$ | $\checkmark$ |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | 个令 |  | \％ | 个令 |  |  | ¢ |  | \％ | F |  |
| Traffic Volume（veh／h） | 68 | 1285 | 4 | 5 | 1048 | 56 | 2 | ， | 5 | 75 | 1 | 45 |
| Future Volume（veh／h） | 68 | 1285 | 4 | 5 | 1048 | 56 | 2 | 1 | 5 | 75 | 1 | 45 |
| Initial $\mathrm{Q}(\mathrm{Qb})$ ，veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped－Bike Adj（A＿pbT） | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 |
| Parking Bus，Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach |  | No |  |  | No |  |  | No |  |  | No |  |
| Adj Sat Flow，veh／h／n | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1945 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate，veh／h | 75 | 1412 | 0 | ， | 1164 | 29 | 5 | 2 | 2 | 89 | 1 | 30 |
| Peak Hour Factor | 0.91 | 0.91 | 0.91 | 0.90 | 0.90 | 0.90 | 0.40 | 0.40 | 0.40 | 0.84 | 0.84 | 0.84 |
| Percent Heavy Veh，\％ | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap，veh／h | 341 | 2552 | 0 | 313 | 2165 | 54 | 151 | 59 | 35 | 250 | 5 | 150 |
| Arrive On Green | 0.05 | 0.72 | 0.00 | 0.20 | 0.20 | 0.20 | 0.10 | 0.10 | 0.10 | 0.10 | 0.10 | 0.10 |
| Sat Flow，veh／h | 1781 | 3647 | 0 | 381 | 3543 | 88 | 669 | 607 | 365 | 1412 | 51 | 1542 |
| Grp Volume（v），veh／h | 75 | 1412 | 0 | 6 | 584 | 609 | 9 | 0 | 0 | 89 |  | 31 |
| Grp Sat Flow（s），veh／h／ln | 1781 | 1777 | 0 | 381 | 1777 | 1854 | 1641 | － | 0 | 1412 | 0 | 1593 |
| Q Serve（g＿s），s | 0.9 | 12.1 | 0.0 | 0.8 | 19.1 | 19.1 | 0.0 | 0.0 | 0.0 | 3.6 | 0.0 | 1.2 |
| Cycle Q Clear（g＿c），s | 0.9 | 12.1 | 0.0 | 6.0 | 19.1 | 19.1 | 0.3 | 0.0 | 0.0 | 3.9 | 0.0 | 1.2 |
| Prop In Lane | 1.00 |  | 0.00 | 1.00 |  | 0.05 | 0.56 |  | 0.22 | 1.00 |  | 0.97 |
| Lane Grp Cap（c），veh／h | 341 | 2552 | 0 | 313 | 1086 | 1133 | 246 | 0 | 0 | 250 | 0 | 155 |
| V／C Ratio（X） | 0.22 | 0.55 | 0.00 | 0.02 | 0.54 | 0.54 | 0.04 | 0.00 | 0.00 | 0.36 | 0.00 | 0.20 |
| Avail Cap（c＿a），veh／h | 369 | 2552 | 0 | 313 | 1086 | 1133 | 673 | － | 0 | 634 | 0 | 588 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 0.33 | 0.33 | 0.33 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter（1） | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.00 | 0.00 | 1.00 | 0.00 | 1.00 |
| Uniform Delay（d），s／veh | 7.4 | 4.3 | 0.0 | 14.6 | 17.7 | 17.7 | 26.6 | 0.0 | 0.0 | 28.2 | 0.0 | 27.0 |
| Incr Delay（d2），s／veh | 0.1 | 0.9 | 0.0 | 0.1 | 1.9 | 1.8 | 0.1 | 0.0 | 0.0 | 0.9 | 0.0 | 0.6 |
| Initial Q Delay（d3），S／veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \％oile QackOfQ（50\％），veh／ln 0.2 2.4 0.0 0.1 9.3 9.7 0.1 0.0 0.0 1.4 0.0 0.5 <br> Unsig．Movement Delay，s／veh             |  |  |  |  |  |  |  |  |  |  |  |  |
| Unsig．Movement Delay，s／veh |  |  |  |  |  |  |  |  |  |  |  |  |
| LnGrp Delay（d），s／veh | 7.5 | 5.2 | 0.0 | 14.7 | 19.6 | 19.6 | 26.7 | 0.0 | 0.0 | 29.1 | 0.0 | 27.6 |
| LnGrp LOS | A | A | A | B | B | B | c | A | A | C | A | C |
| Approach Vol，veh／h |  | 1487 |  |  | 1199 |  |  | ， |  |  | 120 |  |
| Approach Delay，s／veh |  | 5.3 |  |  | 19.6 |  |  | 26.7 |  |  | 28.7 |  |
| Approach LOS |  | A |  |  | B |  |  | C |  |  | C |  |
| Timer－Assigned Phs | 1 | 2 |  | 4 |  | 6 |  | 8 |  |  |  |  |
| Phs Duration（ $G+Y+R \mathrm{C})$ ， s | 7.0 | 45.7 |  | 12.3 |  | 52.7 |  | 12.3 |  |  |  |  |
| Change Period（ $Y+R \mathrm{C}$ ）， s | 4.0 | 6.0 |  | 6.0 |  | 6.0 |  | 6.0 |  |  |  |  |
| Max Green Setting（Gmax），s | 4.0 | 21.0 |  | 24.0 |  | 29.0 |  | 24.0 |  |  |  |  |
| Max Q Clear Time（ $\left.\mathrm{g}_{\text {c }} \mathrm{c}+11\right)$ ，s | 2.9 | 21.1 |  | 2.3 |  | 14.1 |  | 5.9 |  |  |  |  |
| Green Ext Time（p＿c），s | 0.0 | 0.0 |  | 0.0 |  | 8.5 |  | 0.3 |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| HCM 6th Ctrl Delay 12.4 <br> HCM 6th LOS B |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |

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Mountain Shadows Resort


## 11/15/2019

Synchro 10 Repor
Page


| Mountain Shadows Resort Background AM |  |  |  |  |  |  |  | 4: 56th St/56th St \& Lincoln Dr HCM 6th Signalized Intersection Summary |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | $\rightarrow$ | 7 | 7 | $\leftarrow$ | 4 | 4 | $\uparrow$ |  | $\checkmark$ | $\downarrow$ | $\downarrow$ |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 7 | $\uparrow{ }^{\text {¢ }}$ |  | \% | $\uparrow{ }^{\text {¢ }}$ |  |  | $\uparrow$ | 「 |  | \$ |  |
| Traffic Volume (veh/h) | 7 | 1297 | 48 | 14 | 1074 | 6 | 34 | 1 | 15 | 4 | , |  |
| Future Volume (veh/h) | 7 | 1297 | 48 | 14 | 1074 | 6 | 34 | 1 | 15 | 4 | 0 | 3 |
| Initial $Q(Q b)$, veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Ped-Bike Adj(A_pbT) | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach |  | No |  |  | No |  |  | No |  |  | No |  |
| Adj Sat Flow, veh/h/n | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 8 | 1457 | 26 | 15 | 1131 | , | 37 | 1 | 7 | 5 | 0 |  |
| Peak Hour Factor | 0.89 | 0.89 | 0.89 | 0.95 | 0.95 | 0.95 | 0.93 | 0.93 | 0.93 | 0.88 | 0.88 | 0.88 |
| Percent Heavy Veh, \% | 2 | 2 | 2 | 2 | 2 |  | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 423 | 2656 | 47 | 312 | 2707 | 5 | 205 | 4 | 101 | 172 | 0 | 0 |
| Arrive On Green | 0.74 | 0.74 | 0.74 | 0.74 | 0.74 | 0.74 | 0.06 | 0.06 | 0.06 | 0.06 | 0.00 | 0.00 |
| Sat Flow, veh/h | 497 | 3572 | 64 | 356 | 3640 | 6 | 1495 | 65 | 1585 | 960 | , | 0 |
| Grp Volume(v), veh/h | 8 | 724 | 759 | 15 | 552 | 581 | 38 | - | 7 | 5 | 0 | 0 |
| Grp Sat Flow(s),veh/h/n | 497 | 1777 | 1859 | 356 | 1777 | 1869 | 1560 | 0 | 1585 | 960 | 0 | 0 |
| Q Serve(g_s), s | 0.4 | 11.5 | 11.5 | 1.2 | 7.5 | 7.5 | 0.0 | 0.0 | 0.3 | 0.2 | 0.0 | 0.0 |
| Cycle Q Clear(g_c), s | 7.9 | 11.5 | 11.5 | 12.7 | 7.5 | 7.5 | 1.3 | 0.0 | 0.3 | 1.5 | 0.0 | 0.0 |
| Prop In Lane | 1.00 |  | 0.03 | 1.00 |  | 0.00 | 0.97 |  | 1.00 | 1.00 |  | 0.00 |
| Lane Grp Cap(c), veh/h | 423 | 1321 | 1382 | 312 | 1321 | 1390 | 209 | 0 | 101 | 172 | 0 | 0 |
| V/C Ratio(X) | 0.02 | 0.55 | 0.55 | 0.05 | 0.42 | 0.42 | 0.18 | 0.00 | 0.07 | 0.03 | 0.00 | 0.00 |
| Avail Cap(c_a), veh/h | 423 | 1321 | 1382 | 312 | 1321 | 1390 | 623 | - | 561 | 580 | 0 | 0 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(1) | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 0.00 | 0.00 |
| Uniform Delay (d), s/veh | 4.6 | 3.6 | 3.6 | 6.4 | 3.1 | 3.1 | 29.1 | 0.0 | 28.6 | 29.8 | 0.0 | 0.0 |
| Incr Delay (d2), s/veh | 0.1 | 1.6 | 1.6 | 0.3 | 1.0 | 0.9 | 0.4 | 0.0 | 0.3 | 0.1 | 0.0 | 0.0 |
| Initial Q Delay (d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \%oile BackOfQ(50\%), veh/ln | 0.0 | 2.2 | 2.2 | 0.1 | 1.4 | 1.5 | 0.6 | 0.0 | 0.1 | 0.1 | 0.0 | 0.0 |
| Unsig. Movement Delay, s/veh |  |  |  |  |  |  |  |  |  |  |  |  |
| LnGrp Delay(d),s/veh | 4.7 | 5.2 | 5.2 | 6.7 | 4.1 | 4.0 | 29.5 | 0.0 | 28.9 | 29.9 | 0.0 | 0.0 |
| LnGrp LOS | A | A | A | A | A | A | C | A | C | C | A | A |
| Approach Vol, veh/h |  | 1491 |  |  | 1148 |  |  | 45 |  |  | 5 |  |
| Approach Delay, s/veh |  | 5.2 |  |  | 4.1 |  |  | 29.4 |  |  | 29.9 |  |
| Approach LOS |  | A |  |  | A |  |  | C |  |  | C |  |
| Timer - Assigned Phs |  | 2 |  | 4 |  | 6 |  | 8 |  |  |  |  |
| Phs Duration ( $G+Y+R \mathrm{C})$, S |  | 54.8 |  | 10.2 |  | 54.8 |  | 10.2 |  |  |  |  |
| Change Period ( $Y+R \mathrm{C}$ ), s |  | 6.5 |  | 6.0 |  | 6.5 |  | 6.0 |  |  |  |  |
| Max Green Setting (Gmax), s |  | 29.5 |  | 23.0 |  | 29.5 |  | 23.0 |  |  |  |  |
| Max Q Clear Time ( $\mathrm{g}_{\text {c }} \mathrm{c}+11$ ), s |  | 14.7 |  | 3.3 |  | 13.5 |  | 3.5 |  |  |  |  |
| Green Ext Time (p_c), s |  | 6.4 |  | 0.1 |  | 8.9 |  | 0.0 |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| HCM 6th Crtr Delay |  |  | 5.2 |  |  |  |  |  |  |  |  |  |
| HCM 6th LOS |  |  | A |  |  |  |  |  |  |  |  |  |

11/15/2019 Synchro 10 Report

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| Mountain Shadows Background PM |  |  |  | Cam |  |  |  |  | $\begin{aligned} & \text { airw } \\ & \text { h Sign: } \end{aligned}$ | s Dr ed Inter | Linc ction |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | $\rightarrow$ | 7 | 7 | $\leftarrow$ | 4 | 4 | 4 |  | $\checkmark$ | $\downarrow$ | $\checkmark$ |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | 个分 |  | \％ | 个官 |  |  | ¢ |  | \％ | t |  |
| Traffic Volume（veh／h） | 63 | 1091 | 3 | 5 | 1287 | 171 | 4 | 2 | 5 | 114 |  | 85 |
| Future Volume（veh／h） | 63 | 1091 | 3 | 5 | 1287 | 171 | 4 | 2 | 5 | 114 | 2 | 85 |
| Initial Q（Qb），veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Ped－Bike Adj（A＿pbT） | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 |
| Parking Bus，Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach |  | No |  |  | No |  |  | No |  |  | No |  |
| Adj Sat Flow，veh／h／ln | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1945 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate，veh／h | 68 | 1186 | 0 | 5 | 1384 | 98 | 6 | 3 | 3 | 154 | 3 | 74 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.93 | 0.93 | 0.93 | 0.69 | 0.69 | 0.69 | 0.74 | 0.74 | 0.74 |
| Percent Heavy Veh，\％ | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |  |
| Cap，veh／h | 269 | 2421 | 0 | 357 | 1940 | 137 | 153 | 76 | 49 | 316 | 8 | 206 |
| Arrive On Green | 0.04 | 0.68 | 0.00 | 0.39 | 0.39 | 0.39 | 0.13 | 0.13 | 0.13 | 0.13 | 0.13 | 0.13 |
| Sat Flow，veh／h | 1781 | 3647 | 0 | 472 | 3367 | 238 | 522 | 566 | 363 | 1410 | 62 | 1532 |
| Grp Volume（v），veh／h | 68 | 1186 | 0 | 5 | 728 | 754 | 12 | 0 | 0 | 154 | － | 77 |
| Grp Sat Flow（s），veh／h／n | 1781 | 1777 | 0 | 472 | 1777 | 1828 | 1451 | 0 | 0 | 1410 | 0 | 1595 |
| Q Serve（g＿s），s | 0.9 | 10.4 | 0.0 | 0.5 | 22.5 | 22.7 | 0.0 | 0.0 | 0.0 | 3.2 | 0.0 | 2.9 |
| Cycle Q Clear（g＿c），s | 0.9 | 10.4 | 0.0 | 4.0 | 22.5 | 22.7 | 2.9 | 0.0 | 0.0 | 6.1 | 0.0 | 2.9 |
| Prop In Lane | 1.00 |  | 0.00 | 1.00 |  | 0.13 | 0.50 |  | 0.25 | 1.00 |  | 0.96 |
| Lane $\operatorname{Grp} \operatorname{Cap}(\mathrm{c}$ ）veh／h | 269 | 2421 | 0 | 357 | 1024 | 1053 | 278 | 0 | 0 | 316 | 0 | 214 |
| V／C Ratio（X） | 0.25 | 0.49 | 0.00 | 0.01 | 0.71 | 0.72 | 0.04 | 0.00 | 0.00 | 0.49 | 0.00 | 0.36 |
| Avail Cap（c＿a），veh／h | 301 | 2421 | 0 | 357 | 1024 | 1053 | 643 | 0 | 0 | 648 | 0 | 589 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 0.67 | 0.67 | 0.67 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter（I） | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.00 | 0.00 | 1.00 | 0.00 | 1.00 |
| Uniform Delay（d），s／veh | 9.9 | 5.0 | 0.0 | 10.8 | 15.4 | 15.4 | 24.5 | 0.0 | 0.0 | 26.8 | 0.0 | 25.6 |
| Incr Delay（d2），s／veh | 0.2 | 0.7 | 0.0 | 0.1 | 4.2 | 4.2 | 0.1 | 0.0 | 0.0 | 1.2 | 0.0 | 1.0 |
| Initial Q Delay（d3），S／veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \％ile BackOfQ（50\％），veh／ln | 0.3 | 2.4 | 0.0 | 0.0 | 10.1 | 10.5 | 0.2 | 0.0 | 0.0 | 2.3 | 0.0 | 1.1 |
| Unsig．Movement Delay，s／veh |  |  |  |  |  |  |  |  |  |  |  |  |
| LnGrp Delay（d），s／veh | 10.1 | 5.7 | 0.0 | 10.9 | 19.6 | 19.6 | 24.6 | 0.0 | 0.0 | 28.0 | 0.0 | 26.6 |
| LnGrp LOS | B | A | A | B | B | B | C | A | A | C | A | C |
| Approach Vol，veh／h |  | 1254 |  |  | 1487 |  |  | 12 |  |  | 231 |  |
| Approach Delay，s／veh |  | 5.9 |  |  | 19.6 |  |  | 24.6 |  |  | 27.5 |  |
| Approach LOS |  | A |  |  | B |  |  | C |  |  | C |  |
| Timer－Assigned Phs | 1 | 2 |  | 4 |  | 6 |  | 8 |  |  |  |  |
| Phs Duration（ $(+Y+R \mathrm{Cc}$ ）， s | 6.8 | 43.5 |  | 14.7 |  | 50.3 |  | 14.7 |  |  |  |  |
| Change Period（ $\mathrm{Y}+\mathrm{Rc}$ ）， s | 4.0 | 6.0 |  | 6.0 |  | 6.0 |  | 6.0 |  |  |  |  |
| Max Green Setting（Gmax），s | 4.0 | 21.0 |  | 24.0 |  | 29.0 |  | 24.0 |  |  |  |  |
| Max Q Clear Time（g＿c＋11），s | 2.9 | 24.7 |  | 4.9 |  | 12.4 |  | 8.1 |  |  |  |  |
| Green Ext Time（p＿c），s | 0.0 | 0.0 |  | 0.0 |  | 7.5 |  | 0.8 |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| HCM 6th Ctrr Delay |  |  | 14.5 |  |  |  |  |  |  |  |  |  |
| HCM 6th LOS |  |  | B |  |  |  |  |  |  |  |  |  |

11／15／2019 Synchro 10 Report

CivTech


Mountain Shadows Resort


## 11/15/2019

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| Mountain Shadows Resort Background PM |  |  |  |  |  |  |  | 4: 56th St/56th St \& Lincoln Dr HCM 6th Signalized Intersection Summary |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | $\rightarrow$ | 7 | $t$ | $\leftarrow$ | 4 | 4 | $\uparrow$ |  |  | $\downarrow$ | $\downarrow$ |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\uparrow{ }^{\text {¢ }}$ |  | \% | $\uparrow{ }^{\text {¢ }}$ |  |  | $\uparrow$ | 「 |  | \$ |  |
| Traffic Volume (veh/h) | 33 | 1180 | 36 | 48 | 1438 | 32 | 65 | 2 | 23 | 3 | , | 14 |
| Future Volume (veh/h) | 33 | 1180 | 36 | 48 | 1438 | 32 | 65 | 2 | 23 | 3 | 2 | 14 |
| Initial $Q(Q b)$, veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach |  | No |  |  | No |  |  | No |  |  | No |  |
| Adj Sat Flow, veh/h/n | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 36 | 1283 | 28 | 51 | 1514 | 23 | 73 | 2 | 15 | 5 | 3 | 14 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.95 | 0.95 | 0.95 | 0.89 | 0.89 | 0.89 | 0.63 | 0.63 | 0.63 |
| Percent Heavy Veh, \% | 2 | 2 | 2 | 2 | 2 |  | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 281 | 2540 | 55 | 410 | 2559 | 39 | 240 | 5 | 148 | 85 | 38 | 96 |
| Arrive On Green | 1.00 | 1.00 | 1.00 | 0.71 | 0.71 | 0.71 | 0.09 | 0.09 | 0.09 | 0.09 | 0.09 | 0.09 |
| Sat Flow, veh/h | 338 | 3556 | 78 | 419 | 3583 | 54 | 1401 | 55 | 1585 | 178 | 411 | 1030 |
| Grp Volume(v), veh/h | 36 | 641 | 670 | 51 | 750 | 787 | 75 | - | 15 | 22 | 0 | 0 |
| Grp Sat Flow(s),veh/h/n | 338 | 1777 | 1856 | 419 | 1777 | 1861 | 1457 | 0 | 1585 | 1618 | 0 | 0 |
| Q Serve(g_s), s | 2.4 | 0.0 | 0.0 | 2.6 | 13.6 | 13.6 | 2.3 | 0.0 | 0.6 | 0.0 | 0.0 | 0.0 |
| Cycle Q Clear(g_c), s | 16.0 | 0.0 | 0.0 | 2.6 | 13.6 | 13.6 | 3.1 | 0.0 | 0.6 | 3.0 | 0.0 | 0.0 |
| Prop In Lane | 1.00 |  | 0.04 | 1.00 |  | 0.03 | 0.97 |  | 1.00 | 0.23 |  | 0.64 |
| Lane Grp Cap (c), veh/h | 281 | 1269 | 1326 | 410 | 1269 | 1329 | 245 | 0 | 148 | 219 | 0 |  |
| V/C Ratio(X) | 0.13 | 0.50 | 0.51 | 0.12 | 0.59 | 0.59 | 0.31 | 0.00 | 0.10 | 0.10 | 0.00 | 0.00 |
| Avail Cap(c_a), veh/h | 281 | 1269 | 1326 | 410 | 1269 | 1329 | 633 | - | 585 | 652 | 0 | 0 |
| HCM Platoon Ratio | 2.00 | 2.00 | 2.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(1) | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 0.00 | 0.00 |
| Uniform Delay (d), s/veh | 2.3 | 0.0 | 0.0 | 3.0 | 4.6 | 4.6 | 28.0 | 0.0 | 27.0 | 27.1 | 0.0 | 0.0 |
| Incr Delay (d2), s/veh | 0.9 | 1.4 | 1.4 | 0.6 | 2.0 | 1.9 | 0.7 | 0.0 | 0.3 | 0.2 | 0.0 | 0.0 |
| Initial Q Delay (d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \%oile BackOfQ(50\%), veh/ln | 0.1 | 0.5 | 0.5 | 0.2 | 3.1 | 3.2 | 1.1 | 0.0 | 0.2 | 0.3 | 0.0 | 0.0 |
| Unsig. Movement Delay, s/veh |  |  |  |  |  |  |  |  |  |  |  |  |
| LnGrp Delay(d),s/veh | 3.3 | 1.4 | 1.4 | 3.6 | 6.6 | 6.5 | 28.7 | 0.0 | 27.3 | 27.3 | 0.0 | 0.0 |
| LnGrp LOS | A | A | A | A | A | A | c | A | C | C | A | A |
| Approach Vol, veh/h |  | 1347 |  |  | 1588 |  |  | 90 |  |  | 22 |  |
| Approach Delay, s/veh |  | 1.5 |  |  | 6.5 |  |  | 28.5 |  |  | 27.3 |  |
| Approach LOS |  | A |  |  | A |  |  | C |  |  | C |  |
| Timer - Assigned Phs |  | 2 |  | 4 |  | 6 |  | 8 |  |  |  |  |
| Phs Duration ( $G+Y+R \mathrm{C})$, S |  | 52.9 |  | 12.1 |  | 52.9 |  | 12.1 |  |  |  |  |
| Change Period ( $Y+R \mathrm{C}$ ), s |  | 6.5 |  | 6.0 |  | 6.5 |  | 6.0 |  |  |  |  |
| Max Green Setting (Gmax), s |  | 28.5 |  | 24.0 |  | 28.5 |  | 24.0 |  |  |  |  |
| Max Q Clear Time ( $\mathrm{g}_{\text {c }} \mathrm{c}+11$ ), s |  | 15.6 |  | 5.1 |  | 18.0 |  | 5.0 |  |  |  |  |
| Green Ext Time (p_c), s |  | 8.4 |  | 0.3 |  | 6.2 |  | 0.1 |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| HCM 6th Crtr Delay |  |  | 5.1 |  |  |  |  |  |  |  |  |  |
| HCM 6th LOS |  |  | A |  |  |  |  |  |  |  |  |  |

11/15/2019 Synchro 10 Report

Civech


11/15/2019 Synchro 10 Report

Civech


Mountain Shadows Resort Total AM

| Total AM |  |  |  |  |  |  |  |  |
| :--- | ---: | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
|  |  |  |  |  |  |  |  |  |

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| Mountain Shadows Resort Total AM |  |  |  |  |  |  |  | 4: 56th St/56th St \& Lincoln Dr HCM 6th Signalized Intersection Summary |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | $\rightarrow$ | 7 | $t$ | $\leftarrow$ | 4 | 4 | $\uparrow$ |  | $\checkmark$ | $\downarrow$ | $\downarrow$ |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 7 | $\uparrow{ }^{\text {¢ }}$ |  | \% | $\uparrow{ }^{\text {¢ }}$ |  |  | $\uparrow$ | 「 |  | \$ |  |
| Traffic Volume (veh/h) | 7 | 1297 | 48 | 14 | 1075 | 6 | 34 | 1 | 15 | 4 | , |  |
| Future Volume (veh/h) | 7 | 1297 | 48 | 14 | 1075 | 6 | 34 | 1 | 15 | 4 | 0 | 3 |
| Initial $Q(Q b)$, veh | 0 | 0 | 0 | , | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Ped-Bike Adj(A_pbT) | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach |  | No |  |  | No |  |  | No |  |  | No |  |
| Adj Sat Flow, veh/h/ln | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 8 | 1457 | 26 | 15 | 1132 | , | 37 | 1 | 7 | 5 | 0 |  |
| Peak Hour Factor | 0.89 | 0.89 | 0.89 | 0.95 | 0.95 | 0.95 | 0.93 | 0.93 | 0.93 | 0.88 | 0.88 | 0.88 |
| Percent Heavy Veh, \% | 2 | 2 | 2 | 2 | 2 |  | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 422 | 2656 | 47 | 312 | 2707 | 5 | 205 | 4 | 101 | 172 | 0 | 0 |
| Arrive On Green | 0.74 | 0.74 | 0.74 | 0.74 | 0.74 | 0.74 | 0.06 | 0.06 | 0.06 | 0.06 | 0.00 | 0.00 |
| Sat Flow, veh/h | 496 | 3572 | 64 | 356 | 3640 | 6 | 1495 | 65 | 1585 | 960 | , | 0 |
| Grp Volume(v), veh/h | 8 | 724 | 759 | 15 | 553 | 581 | 38 | - | 7 | 5 | 0 | 0 |
| Grp Sat Flow(s),veh/h/n | 496 | 1777 | 1859 | 356 | 1777 | 1869 | 1560 | 0 | 1585 | 960 | 0 | 0 |
| Q Serve(g_s), s | 0.4 | 11.5 | 11.5 | 1.2 | 7.5 | 7.5 | 0.0 | 0.0 | 0.3 | 0.2 | 0.0 | 0.0 |
| Cycle Q Clear(g_c), s | 7.9 | 11.5 | 11.5 | 12.7 | 7.5 | 7.5 | 1.3 | 0.0 | 0.3 | 1.5 | 0.0 | 0.0 |
| Prop In Lane | 1.00 |  | 0.03 | 1.00 |  | 0.00 | 0.97 |  | 1.00 | 1.00 |  | 0.00 |
| Lane Grp Cap(c), veh/h | 422 | 1321 | 1382 | 312 | 1321 | 1390 | 209 | 0 | 101 | 172 | 0 |  |
| V/C Ratio(X) | 0.02 | 0.55 | 0.55 | 0.05 | 0.42 | 0.42 | 0.18 | 0.00 | 0.07 | 0.03 | 0.00 | 0.00 |
| Avail Cap(c_a), veh/h | 422 | 1321 | 1382 | 312 | 1321 | 1390 | 623 | - | 561 | 580 | 0 | 0 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(1) | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 0.00 | 0.00 |
| Uniform Delay (d), s/veh | 4.6 | 3.6 | 3.6 | 6.4 | 3.1 | 3.1 | 29.1 | 0.0 | 28.6 | 29.8 | 0.0 | 0.0 |
| Incr Delay (d2), s/veh | 0.1 | 1.6 | 1.6 | 0.3 | 1.0 | 0.9 | 0.4 | 0.0 | 0.3 | 0.1 | 0.0 | 0.0 |
| Initial Q Delay (d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \%oile BackOfQ(50\%), veh/ln | 0.0 | 2.2 | 2.2 | 0.1 | 1.4 | 1.5 | 0.6 | 0.0 | 0.1 | 0.1 | 0.0 | 0.0 |
| Unsig. Movement Delay, s/veh |  |  |  |  |  |  |  |  |  |  |  |  |
| LnGrp Delay(d),s/veh | 4.7 | 5.2 | 5.2 | 6.7 | 4.1 | 4.0 | 29.5 | 0.0 | 28.9 | 29.9 | 0.0 | 0.0 |
| LnGrp LOS | A | A | A | A | A | A | C | A | C | C | A | A |
| Approach Vol, veh/h |  | 1491 |  |  | 1149 |  |  | 45 |  |  | 5 |  |
| Approach Delay, s/veh |  | 5.2 |  |  | 4.1 |  |  | 29.4 |  |  | 29.9 |  |
| Approach LOS |  | A |  |  | A |  |  | C |  |  | C |  |
| Timer - Assigned Phs |  | 2 |  | 4 |  | 6 |  | 8 |  |  |  |  |
| Phs Duration ( $G+Y+R \mathrm{C})$, S |  | 54.8 |  | 10.2 |  | 54.8 |  | 10.2 |  |  |  |  |
| Change Period ( $Y+R \mathrm{C}$ ), s |  | 6.5 |  | 6.0 |  | 6.5 |  | 6.0 |  |  |  |  |
| Max Green Setting (Gmax), s |  | 29.5 |  | 23.0 |  | 29.5 |  | 23.0 |  |  |  |  |
| Max Q Clear Time ( $\mathrm{g}_{\text {c }} \mathrm{c}+11$ ), s |  | 14.7 |  | 3.3 |  | 13.5 |  | 3.5 |  |  |  |  |
| Green Ext Time (p_c), s |  | 6.4 |  | 0.1 |  | 8.9 |  | 0.0 |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| HCM 6th Crtr Delay |  |  | 5.2 |  |  |  |  |  |  |  |  |  |
| HCM 6th LOS |  |  | A |  |  |  |  |  |  |  |  |  |

11/15/2019 Synchro 10 Report

Civech


| Mountain Shadows Total PM |  |  |  |  |  |  |  |  | airwa <br> Signal | Dr <br> ed Inte | Linc ction S | $\begin{aligned} & \text { In Dr } \\ & \text { mmary } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\stackrel{ }{ }$ | $\rightarrow$ | 7 | 7 | 4 |  |  | 4 | 7 |  | $\downarrow$ | $\downarrow$ |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | 个官 |  | \％ | 个官 |  |  | \＄ |  | \％ | F |  |
| Traffic Volume（veh／h） | 63 | 1104 | 3 | 5 | 1293 | 171 | 4 | 2 | 6 | 115 | 2 | 85 |
| Future Volume（veh／h） | 63 | 1104 | 3 | 5 | 1293 | 171 | 4 | 2 | 6 | 115 | 2 | 85 |
| Initial $\mathrm{Q}(\mathrm{Qb})$ ，veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped－Bike Adj（A＿pbT） | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 |
| Parking Bus，Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach |  | No |  |  | No |  |  | No |  |  | No |  |
| Adj Sat Flow，veh／h／ln | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1945 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate，veh／h | 68 | 1200 | 0 | 5 | 1390 | 98 | 6 | 3 | 5 | 155 | 3 | 74 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.93 | 0.93 | 0.93 | 0.69 | 0.69 | 0.69 | 0.74 | 0.74 | 0.74 |
| Percent Heavy Veh，\％ | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap，veh／h | 267 | 2418 | 0 | 352 | 1938 | 136 | 140 | 75 | 75 | 317 | 8 | 207 |
| Arrive On Green | 0.04 | 0.68 | 0.00 | 0.39 | 0.39 | 0.39 | 0.14 | 0.14 | 0.14 | 0.14 | 0.14 | 0.14 |
| Sat Flow，veh／h | 1781 | 3647 | 0 | 466 | 3368 | 237 | 453 | 553 | 559 | 1407 | 62 | 1532 |
| Grp Volume（v），veh／h | 68 | 1200 | 0 | 5 | 731 | 757 | 14 | 0 | 0 | 155 | 0 | 77 |
| Grp Sat Flow（s），veh／h／ln | 1781 | 1777 | 0 | 466 | 1777 | 1828 | 1565 | 0 | 0 | 1407 | 0 | 1595 |
| Q Serve（g＿s），s | 0.9 | 10.6 | 0.0 | 0.5 | 22.7 | 22.9 | 0.0 | 0.0 | 0.0 | 3.2 | 0.0 | 2.9 |
| Cycle Q Clear（g＿c），s | 0.9 | 10.6 | 0.0 | 4.2 | 22.7 | 22.9 | 2.9 | 0.0 | 0.0 | 6.1 | 0.0 | 2.9 |
| Prop In Lane | 1.00 |  | 0.00 | 1.00 |  | 0.13 | 0.43 |  | 0.36 | 1.00 |  | 0.96 |
| Lane Grp Cap（c），veh／h | 267 | 2418 | 0 | 352 | 1022 | 1051 | 290 | 0 | 0 | 317 | 0 | 215 |
| V／C Ratio（X） | 0.25 | 0.50 | 0.00 | 0.01 | 0.72 | 0.72 | 0.05 | 0.00 | 0.00 | 0.49 | 0.00 | 0.36 |
| Avail Cap（c＿a），veh／h | 299 | 2418 | 0 | 352 | 1022 | 1051 | 657 | 0 | 0 | 647 | 0 | 589 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 0.67 | 0.67 | 0.67 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter（I） | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.00 | 0.00 | 1.00 | 0.00 | 1.00 |
| Uniform Delay（d），s／veh | 10.1 | 5.0 | 0.0 | 11.0 | 15.5 | 15.5 | 24.5 | 0.0 | 0.0 | 26.8 | 0.0 | 25.5 |
| Incr Delay（d2），s／veh | 0.2 | 0.7 | 0.0 | 0.1 | 4.3 | 4.3 | 0.1 | 0.0 | 0.0 | 1.2 | 0.0 | 1.0 |
| Initial Q Delay（d3），S／veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \％ile BackOfQ（50\％），veh／n | 0.3 | 2.4 | 0.0 | 0.0 | 10.2 | 10.6 | 0.2 | 0.0 | 0.0 | 2.3 | 0.0 | 1.1 |
| Unsig．Movement Delay，s／veh |  |  |  |  |  |  |  |  |  |  |  |  |
| LnGrp Delay（d），s／veh | 10.2 | 5.7 | 0.0 | 11.1 | 19.7 | 19.8 | 24.6 | 0.0 | 0.0 | 27.9 | 0.0 | 26.6 |
| LnGrp LOS | B | A | A | B | B | B | C | A | A | C | A | C |
| Approach Vol，veh／h |  | 1268 |  |  | 1493 |  |  | 14 |  |  | 232 |  |
| Approach Delay，s／veh |  | 6.0 |  |  | 19.7 |  |  | 24.6 |  |  | 27.5 |  |
| Approach LOS |  | A |  |  | B |  |  | C |  |  | C |  |
| Timer－Assigned Phs | 1 | 2 |  | 4 |  | 6 |  | 8 |  |  |  |  |
| Phs Duration（ $\mathrm{G}+\mathrm{Y}+\mathrm{Rc}$ ）， s | 6.8 | 43.4 |  | 14.8 |  | 50.2 |  | 14.8 |  |  |  |  |
| Change Period（ $\mathrm{Y}+\mathrm{Rc}$ ）， s | 4.0 | 6.0 |  | 6.0 |  | 6.0 |  | 6.0 |  |  |  |  |
| Max Green Setting（Gmax），s | 4.0 | 21.0 |  | 24.0 |  | 29.0 |  | 24.0 |  |  |  |  |
| Max Q Clear Time（g＿c＋11），s | 2.9 | 24.9 |  | 4.9 |  | 12.6 |  | 8.1 |  |  |  |  |
| Green Ext Time（p＿c），s | 0.0 | 0.0 |  | 0.0 |  | 7.6 |  | 0.8 |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| HCM 6th Ctrl Delay |  |  | 14.6 | B |  |  |  |  |  |  |  |  |
|  |  |  | B |  |  |  |  |  |  |  |  |  |

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CivTech


Mountain Shadows Resort Total PM

| Total PM |  |  |  |  |  |  |  |  |
| :--- | ---: | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
|  |  |  |  |  |  |  |  |  |

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| Mountain Shadows Resort Total PM |  |  |  |  |  |  |  | 4: 56th St/56th St \& Lincoln Dr HCM 6th Signalized Intersection Summary |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4 | $\rightarrow$ | 7 | 7 | $\longleftarrow$ | 4 | 4 | $\uparrow$ |  |  | $\downarrow$ | $\downarrow$ |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\uparrow{ }^{\text {¢ }}$ |  | \% |  |  |  | $\uparrow$ | 「 |  | \$ |  |
| Traffic Volume (veh/h) | 33 | 1182 | 36 | 52 | 1443 | 32 | 68 | 2 | 26 | 3 | , | 14 |
| Future Volume (veh/h) | 33 | 1182 | 36 | 52 | 1443 | 32 | 68 | 2 | 26 | 3 | 2 | 14 |
| Initial $Q(Q b)$, veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach |  | No |  |  | No |  |  | No |  |  | No |  |
| Adj Sat Flow, veh/h/ln | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 36 | 1285 | 28 | 55 | 1519 | 23 | 76 | 2 | 18 | 5 | 3 | 14 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.95 | 0.95 | 0.95 | 0.89 | 0.89 | 0.89 | 0.63 | 0.63 | 0.63 |
| Percent Heavy Veh, \% | 2 | 2 | 2 | 2 | 2 |  | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 279 | 2535 | 55 | 409 | 2554 | 39 | 245 | 5 | 150 | 85 | 39 | 98 |
| Arrive On Green | 1.00 | 1.00 | 1.00 | 0.71 | 0.71 | 0.71 | 0.09 | 0.09 | 0.09 | 0.09 | 0.09 | 0.09 |
| Sat Flow, veh/h | 336 | 3556 | 77 | 419 | 3583 | 54 | 1432 | 50 | 1585 | 177 | 413 | 1033 |
| Grp Volume(v), veh/h | 36 | 642 | 671 | 55 | 753 | 789 | 78 | 0 | 18 | 22 | 0 | 0 |
| Grp Sat Flow(s),veh/h/n | 336 | 1777 | 1856 | 419 | 1777 | 1861 | 1481 | 0 | 1585 | 1624 | 0 | 0 |
| Q Serve(g_s), s | 2.4 | 0.0 | 0.0 | 2.8 | 13.7 | 13.8 | 1.9 | 0.0 | 0.7 | 0.0 | 0.0 | 0.0 |
| Cycle Q Clear(g_c), s | 16.2 | 0.0 | 0.0 | 2.8 | 13.7 | 13.8 | 3.1 | 0.0 | 0.7 | 0.8 | 0.0 | 0.0 |
| Prop In Lane | 1.00 |  | 0.04 | 1.00 |  | 0.03 | 0.97 |  | 1.00 | 0.23 |  | 0.64 |
| Lane Grp Cap(c), veh/h | 279 | 1267 | 1323 | 409 | 1267 | 1326 | 250 | 0 | 150 | 222 | 0 |  |
| V/C Ratio(X) | 0.13 | 0.51 | 0.51 | 0.13 | 0.59 | 0.60 | 0.31 | 0.00 | 0.12 | 0.10 | 0.00 | 0.00 |
| Avail Cap(c_a), veh/h | 279 | 1267 | 1323 | 409 | 1267 | 1326 | 635 | - | 585 | 652 | 0 | 0 |
| HCM Platoon Ratio | 2.00 | 2.00 | 2.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(1) | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 0.00 | 0.00 |
| Uniform Delay (d), s/veh | 2.4 | 0.0 | 0.0 | 3.1 | 4.7 | 4.7 | 27.9 | 0.0 | 26.9 | 27.0 | 0.0 | 0.0 |
| Incr Delay (d2), s/veh | 1.0 | 1.5 | 1.4 | 0.7 | 2.1 | 2.0 | 0.7 | 0.0 | 0.4 | 0.2 | 0.0 | 0.0 |
| Initial Q Delay (d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \%oile BackOfQ(50\%), veh/ln | 0.1 | 0.5 | 0.5 | 0.2 | 3.1 | 3.2 | 1.2 | 0.0 | 0.3 | 0.3 | 0.0 | 0.0 |
| Unsig. Movement Delay, s/veh |  |  |  |  |  |  |  |  |  |  |  |  |
| LnGrp Delay(d),s/veh | 3.4 | 1.5 | 1.4 | 3.8 | 6.7 | 6.6 | 28.6 | 0.0 | 27.3 | 27.2 | 0.0 | 0.0 |
| LnGrp LOS | A | A | A | A | A | A | C | A | C | C | A | A |
| Approach Vol, veh/h |  | 1349 |  |  | 1597 |  |  | 96 |  |  | 22 |  |
| Approach Delay, s/veh |  | 1.5 |  |  | 6.6 |  |  | 28.4 |  |  | 27.2 |  |
| Approach LOS |  | A |  |  | A |  |  | C |  |  | C |  |
| Timer - Assigned Phs |  | 2 |  | 4 |  | 6 |  | 8 |  |  |  |  |
| Phs Duration ( $G+Y+R \mathrm{C})$, S |  | 52.8 |  | 12.2 |  | 52.8 |  | 12.2 |  |  |  |  |
| Change Period ( $Y+R \mathrm{C}$ ), s |  | 6.5 |  | 6.0 |  | 6.5 |  | 6.0 |  |  |  |  |
| Max Green Setting (Gmax), s |  | 28.5 |  | 24.0 |  | 28.5 |  | 24.0 |  |  |  |  |
| Max Q Clear Time ( $\mathrm{g}_{\text {c }} \mathrm{c}+11$ ), s |  | 15.8 |  | 5.1 |  | 18.2 |  | 2.8 |  |  |  |  |
| Green Ext Time (p_c), s |  | 8.4 |  | 0.4 |  | 6.1 |  | 0.1 |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| HCM 6th Crtr Delay |  |  | 5.2 |  |  |  |  |  |  |  |  |  |
| HCM 6th LOS |  |  | A |  |  |  |  |  |  |  |  |  |

11/15/2019 Synchro 10 Report

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## APPENDIX H

SCOTTSDALE DS\&PM SECTION 5-3.201

## CivTech



FIGURE 5-3.34 ROUNDABOUT SIGHT DISTANCE

## STREET ACCESS AND DRIVEWAYS

Driveway types are determined by land use type and street classification. The standards for these driveway types are illustrated in Figure 5-3.38 through Figure 53.43. Refer to Figure 5-3.39 for driveway grade standards.

## DRIVEWAY SPACING

Standard and minimum driveway spacing will generally conform to the following standards. This minimum spacing applies to proposed site driveway separation as well
as separation from existing or planned driveways and streets on adjacent parcels. The spacing is measured to the driveway or street centerline.

| STREET TYPE | STANDARD DRIVEWAY <br> SPACING | MINIMUM DRIVEWAY <br> SPACING |
| :--- | :---: | :---: |
| Local Residential / Local <br> Collector | 50 feet | 50 feet |
| Local Industrial / Local <br> Commercial | 165 feet | 125 feet |
| Minor Collector | 165 feet | 125 feet |
| Major Collector | 250 feet | 150 feet |
| Minor Arterial | 330 feet | 250 feet |
| Major Arterial | 500 feet | 300 feet |

FIGURE 5-3.35 DRIVEWAY SPACING
Standard driveway spacing criteria shall apply for all new driveways where there are no conflicts with existing driveway and street intersections, site frontage is adequate, and there are no conflicts with natural features or drainage structures. The minimum driveway spacing may be allowed when approved by Transportation staff where those conflicts noted above exist or other site plan associated issues do not allow the standard driveway spacing to be implemented. In locations where the standard driveway spacing cannot be achieved, a deceleration lane may be required to mitigate the impact of the closer driveway spacing.
For sites that have frontage on two streets, primary access should be onto the minor street frontage. A maximum of two driveway openings is permitted to a site or parcel from the abutting street(s). The Transportation Department may permit additional driveway entrances when projected travel demands indicate it is in the interests of good traffic operation, and when adequate street frontage exists to maintain the above guidelines.
Where new development adjoins other similarly zoned property or compatible land uses, a cross access easement may be required to permit vehicular movement between the parcels or to reduce the number of access points required onto the adjacent public street. Combining driveways reduces the number of conflict points for pedestrians, bicyclists, and other vehicles. This may be required regardless of the development status of the adjoining property, unless the cross access is determined to be unfeasible by city staff.
New driveways on collector and arterial streets in areas that do not have raised medians shall align with existing or planned driveways and street intersections to avoid creating interlocking left turns and other conflicts. Offsets in the driveway centerlines may be allowed up to 6 feet. If the driveways cannot be aligned, the driveways should be offset a minimum distance of 125 feet along streets without a center turn lane, and a minimum 250 feet along streets with a center turn lane. When site driveway locations are modified, any existing driveways that are not going to be utilized for access must be removed and replaced with curb, gutter, and sidewalk to match the adjacent improvements.

## DRIVEWAY LOCATIONS

A new access driveway will not be allowed (measured to the driveway centerline):
A. Within 30 feet of any commercial property line, except when it is a joint-use driveway serving two abutting commercial properties and access agreements have been exchanged between, and recorded by, the two abutting property owners;
B. When the total width of all driveways serving a property exceeds 50 percent of the curb line frontage;
C. Within 50 feet of the rights-of-way line of an intersecting non-arterial street;
D. Within 100 feet of the rights-of-way line of an intersecting arterial street;
E. Within 100 feet of an approved median opening location on an arterial street;
F. Less than the minimum spacing as established under Section 5-3.201;

## VEHICULAR NON-ACCESS EASEMENT

For proper control of driveway access, a vehicular non-access easement (V.N.E.) is to be granted to the city, except at approved access points, along all collector and arterial streets when abutting property develops.

## RESIDENTIAL DRIVEWAYS

A. Single-family Residential Development

Driveways serving single-family residential units should be S-1 type driveways as shown in Figure 5-3.40. Only one driveway per lot street frontage is allowed except where the street frontage is of sufficient length to maintain a separation of 50 feet between driveways. The minimum driveway length is 18 feet, measured from the face of the garage opening to the back of sidewalk or the back of curb if no sidewalk is provided. Refer to Section 2-2.308 for additional discussion on driveways. Refer to Standard Detail Drawings (2200 Series) for access ramp design requirements.
B. Multi-family Residential Development

Driveways serving multi-family residential units should be CL and CH type driveways, as shown in Figure 5-3.41 through Figure 5-3.44. Type CL-1 and CL-2 are low-volume driveways to be used on local streets. Type $\mathrm{CH}-1,-2$ and -3 are high volume driveways to be used on collector and arterial streets. CL type driveways may be required along urban character collector and arterial streets with higher pedestrian traffic. The minimum driveway length is 50 feet, measured from the entrance to the off-street parking area to the back of sidewalk, or to the back of curb if no sidewalk is provided. Refer to Standard Detail Drawings (2200 Series) for access ramp design requirements.
C. Limitations on Residential Access

Residential properties that have frontage on a local street, an arterial, or collector street are limited to local street access.
In some instances, residential parcels fronting only on arterial or collector streets may be given access if alternate public access is not available. When such access is allowed, the driveway must be circular, or it must have a turn-around area to ensure there is no need for backing onto the street.

## APPENDIX I

## LEFT TURN LANE DESIGN CONCEPT

## CivTech



MOUNTAIN SHADOW

SHeETTTIE $\quad$ Plan Sheet Title
PRoJEGT TIIE
MOUNTAIN SHADOW
LEFT TURN


## APPENDIX J

## QUEUE STORAGE ANALYSIS

## CivTech

## Signalized Intersection 2020

Average Vehicle Length (ft): 25
Intersection Cycle Length (sec): 65
Equation Used: storage length $=2 \times$ (vehicles/hour)/(cycles/hour) $x$ average vehicle length

| Intersection | Approach | AM Peak (veh/hr) | Midday Peak | PM Peak (veh/hr) | Max vehs per 2 cycles | Max trucks per 2 cycles | Storage Lenath |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Desert Fairways Dr \& Lincoln Dr | NB Left | 2 | 0 | 4 | 1 | 0 | 25' |
|  | SB Left | 75 | 0 | 115 | 5 | 0 | 125' |
|  | EB Left | 68 | 0 | 63 | 3 | 0 | 75' |
|  | WB Left | 5 | 0 | 5 | 1 | 0 | 25' |
|  | NB Right | 5 | 0 | 6 | 1 | 0 | 25' |
|  | SB Right | 45 | 0 | 85 | 4 | 0 | 100' |
|  | EB Right | 4 | 0 | 3 | 1 | 0 | 25' |
|  | WB Right | 56 | 0 | 171 | 7 | 0 | 175' |
| 56th St \& Lincoln Dr | NB Left | 34 | 0 | 68 | 3 | 0 | 75' |
|  | SB Left | 4 | 0 | 3 | 1 | 0 | $25^{\prime}$ |
|  | EB Left | 7 | 0 | 33 | 2 | 0 | 50' |
|  | WB Left | 14 | 0 | 52 | 2 | 0 | 50' |
|  | NB Right | 15 | 0 | 26 | 1 | 0 | 25' |
|  | SB Right | 3 | 0 | 14 | 1 | 0 | 25' |
|  | EB Right | 48 | 0 | 36 | 2 | 0 | 50' |
|  | WB Right | 6 | 0 | 32 | 2 | 0 | 50' |

Average Vehicle Length (ft): 25
Equation Used: storage length $=2 \times$ (vehicles/hour)/(60 minutes/hour) $\times$ average vehicle length

| Intersection | Approach | AM Peak (veh/hr) | Midday Peak | PM Peak (veh/hr) | Veh per 2 minutes | Trucks per 2 minutes | Storage Lenath |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mountain Shadows West \& Lincoln Dr | NB Left | 0 | 0 | 0 | 0 | 0 | $0^{\prime}$ |
|  | SB Left | 0 | 0 | 0 | 0 | 0 | 0 |
|  | EB Left | 0 | 0 | 0 | 0 | 0 | $0 '$ |
|  | WB Left | 7 | 0 | 16 | 1 | 0 | 25' |
|  | NB Right | 13 | 0 | 29 | 1 | 0 | $25{ }^{\prime}$ |
|  | SB Right | 0 | 0 | 0 | 0 | 0 | $0^{\prime}$ |
|  | EB Right | 24 | 0 | 43 | 2 | 0 | $50^{\prime}$ |
|  | WB Right | 0 | 0 | 0 | 0 | 0 | $0^{\prime}$ |
| Mountain Shadows East \& Lincoln Dr | NB Left | 23 | 0 | 18 | 1 | 0 | $25^{\prime}$ |
|  | SB Left | 0 | 0 | 0 | 0 | 0 | $0 '$ |
|  | EB Left | 0 | 0 | 0 | 0 | 0 | $0{ }^{\prime}$ |
|  | WB Left | 11 | 0 | 21 | 1 | 0 | 25' |
|  | NB Right | 14 | 0 | 27 | 1 | 0 | $25^{\prime}$ |
|  | SB Right | 0 | 0 | 0 | 0 | 0 | $0^{\prime}$ |
|  | EB Right | 28 | 0 | 25 | 1 | 0 | 25' |
|  | WB Right | 0 | 0 | 0 | 0 | 0 | $0^{\prime}$ |


|  | 4 | $\rightarrow$ | $\downarrow$ | $\leftarrow$ | $\uparrow$ | - | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | WBL | WBT | NBT | SBL | SBT |
| Lane Group Flow (vph) | 75 | 1417 | 6 | 1226 | 21 | 89 | 55 |
| v/c Ratio | 0.22 | 0.55 | 0.03 | 0.55 | 0.08 | 0.43 | 0.19 |
| Control Delay | 5.4 | 7.1 | 8.0 | 11.0 | 15.1 | 30.5 | 9.2 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 5.4 | 7.1 | 8.0 | 11.0 | 15.1 | 30.5 | 9.2 |
| Queue Length 50th (tt) | 7 | 134 | 1 | 197 | 3 | 33 | 0 |
| Queue Length 95th (tt) | 22 | 233 | m4 | 319 | 6 | 61 | 23 |
| Internal Link Dist (tt) |  | 606 |  | 1662 | 828 |  | 853 |
| Turn Bay Length ( t ) | 150 |  | 50 |  |  | 75 |  |
| Base Capacity (vph) | 334 | 2557 | 216 | 2217 | 641 | 511 | 620 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.22 | 0.55 | 0.03 | 0.55 | 0.03 | 0.17 | 0.09 |
| Intersection Summary |  |  |  |  |  |  |  |


|  | 4 | $\rightarrow$ | 1 | $\leftarrow$ | $\uparrow$ |  | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | WBL | WBT | NBT | NBR | SBT |
| Lane Group Flow (vph) | 8 | 1489 | 15 | 1138 | 38 | 16 | 8 |
| V/c Ratio | 0.02 | 0.52 | 0.07 | 0.40 | 0.21 | 0.07 | 0.04 |
| Control Delay | 2.3 | 6.0 | 4.6 | 4.0 | 28.5 | 0.5 | 0.3 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 2.3 | 6.0 | 4.6 | 4.0 | 28.5 | 0.5 | 0.3 |
| Queue Length 50th (tt) | 1 | 224 | 2 | 83 | 14 | 0 | 0 |
| Queue Length 95th (tt) | m0 | 331 | 8 | 135 | 38 | 0 | 0 |
| Internal Link Dist (tt) |  | 411 |  | 603 | 435 |  | 338 |
| Turn Bay Length (t) | 50 |  | 80 |  |  | 285 |  |
| Base Capacity (vph) | 358 | 2849 | 225 | 2859 | 531 | 598 | 530 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.02 | 0.52 | 0.07 | 0.40 | 0.07 | 0.03 | 0.02 |
| Intersection Summary |  |  |  |  |  |  |  |


|  |  |  | 7 |  | $\uparrow$ |  | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | WBL | WBT | NBT | SBL | SBT |
| Lane Group Flow (vph) | 68 | 1203 | 5 | 1574 | 18 | 155 | 118 |
| v/c Ratio | 0.26 | 0.55 | 0.02 | 0.86 | 0.05 | 0.57 | 0.29 |
| Control Delay | 7.8 | 9.2 | 22.2 | 30.1 | 13.9 | 31.0 | 6.7 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 7.8 | 9.2 | 22.2 | 30.1 | 13.9 | 31.0 | 6.7 |
| Queue Length 50th ( t ) | 8 | 124 | 2 | 333 | 3 | 56 | 1 |
| Queue Length 95th (t) | 26 | 226 | m3 | \#580 | 11 | 76 | 21 |
| Internal Link Dist (t) |  | 606 |  | 1662 | 828 |  | 853 |
| Turn Bay Length (t) | 150 |  | 50 |  |  | 75 |  |
| Base Capacity (vph) | 266 | 2193 | 229 | 1839 | 637 | 513 | 659 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.26 | 0.55 | 0.02 | 0.86 | 0.03 | 0.30 | 0.18 |
| Intersection Summary |  |  |  |  |  |  |  |
| \# 95th percentile volume exceeds capacity, queue may be longer. |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| $m$ Volume for 95th percentile queue is metered by upstream signal. |  |  |  |  | Queue shown is maximum after two cycles. |  |  |


|  | $\rangle$ | $\rightarrow$ | 7 | $\leftrightarrow$ | $\dagger$ | $>$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | WBL | WBT | NBT | NBR | SBT |
| Lane Group Flow (vph) | 36 | 1291 | 55 | 1615 | 78 | 29 | 30 |
| V/c Ratio | 0.24 | 0.51 | 0.22 | 0.63 | 0.41 | 0.10 | 0.13 |
| Control Delay | 6.7 | 3.5 | 8.1 | 8.1 | 31.1 | 3.4 | 19.7 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 6.7 | 3.5 | 8.1 | 8.1 | 31.1 | 3.4 | 19.7 |
| Queue Length 50th (tt) | 3 | 66 | 7 | 168 | 29 | 0 | 8 |
| Queue Length 95th (ft) | m7 | 82 | 28 | 290 | 61 | 9 | 18 |
| Internal Link Dist (tt) |  | 411 |  | 603 | 435 |  | 338 |
| Turn Bay Length ( t ) | 50 |  | 80 |  |  | 285 |  |
| Base Capacity (vph) | 151 | 2548 | 250 | 2552 | 488 | 621 | 581 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.24 | 0.51 | 0.22 | 0.63 | 0.16 | 0.05 | 0.05 |
| Intersection Summary |  |  |  |  |  |  |  |
| m Volume for 95th percentile queue is metered by upstream signal. |  |  |  |  |  |  |  |

## APPENDIX K

SUP ACCESS DIAGRAMS



[^0]:    ${ }^{1}$ The American Association of Highway and Transportation Officials on pages 714-715 of its publication, Geometric Design of Highways and Streets ("AASHTO Green Book"), indicates that storage length for a turn lane, exclusive of taper, "should usually be based on one and one-half to two times the average number of vehicles that would store per cycle" at a signalized intersection.

