

Mountain Shadows Resort

Traffic Impact Study

5445 E. Lincoln Drive in Town of Paradise Valley

November 2019 Project No. 17-0765

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For Submittal to: **Town of Paradise Valley**

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MOUNTAIN SHADOWS RESORT TRAFFIC IMPACT ANALYSIS

5445 East Lincoln Drive Town of Paradise Valley, Arizona

Prepared for:

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For Submittal to:

Town of Paradise Valley

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EXECUTIVE SUMMARY

This report documents a traffic impact analysis (TIA) performed for the Mountain Shadows Resort. The Resort consists of an existing 183-room hotel and 41 condominiums. A new Bianco restaurant is proposed for the northeast corner of the site and will consist of approximately 5,000 square feet.

CivTech, Inc. has been retained by Westroc Hospitality LLC to perform the TIA for the proposed median break at the west driveway. The purpose of this TIA is to determine if a westbound left turn lane is warranted at the existing Mountain Shadows West Driveway. This driveway currently operates as a right in/right out driveway only.

The following conclusions have been documented for this study.

General

• The proposed restaurant is anticipated to generate approximately 420 additional weekday daily trips, with 4 (3 in/1 out) additional trips occurring in the AM peak hour and 39 (26 in/13 out) additional trips occurring in the PM peak hour.

Existing Conditions

 The results of the existing conditions analysis summarized in **Table 2** indicates that all intersections currently operate at an overall acceptable level of service (LOS C or better) under existing lane configurations and signal timing.

Opening Year 2020

 The results of the 2020 opening year Synchro analysis indicates that all study intersections are anticipated to experience an acceptable level of service LOS C or better.

Left Turn Lane Analysis

- The Town of Paradise Valley does not provide explicit criteria for the installation of dedicated left turn lanes; they typically defer to City of Scottsdale design standards.
 - Per the City of Scottsdale Design Standards and Policy Manual (DS&PM), driveways located on arterial roadways must be spaced 330 feet apart, with a minimum spacing of 250 feet with no restrictions. The eastern and western driveways for the Mountain Shadows Resort are spaced approximately 530 feet apart, meaning that a left turn lane into the site at the western driveway should be permitted. A dedicated westbound left turn lane is also safer than making a U-turn at an unprotected signalized intersection or at a median break, which was observed to be happening during both the AM and PM peak hours.



Queue Storage

- The recommended storage lengths are provided for horizon year 2020 using the total traffic projections.
 - There is an existing median break along Lincoln Drive directly east of the proposed median break at the west driveway. This median break is utilized as emergency access for the Colonia Miramonte housing development north of the Mountain Shadows Resort. Since this is a gated emergency access only not available to residents for daily usage, it was determined this median break can be utilized in the design of a westbound left turn lane into the Mountain Shadows Resort west driveway. The determination was provided by the Town based on concept designs provided by the Town's consultant as part of earlier review comments for this project. A minimum of 75 feet of storage is recommended with a 90-foot taper for a total length of 165 feet.
 - It is recommended that the median be constructed with mountable curb in the areas proximate to the Colonia Miramonte emergency access.



INTRODUCTION

Mountain Shadows Resort currently consists of a 183-room hotel and 41 condominiums. A new Bianco Restaurant is proposed for the northeast corner of the existing site. The Mountain Shadows Resort is located at 5445 E Lincoln Drive in the Town of Paradise Valley, Arizona.

Study Requirements

The purpose of this study is to determine if a median break and dedicated westbound left turn lane is warranted at the Mountain Shadows West Driveway on Lincoln Drive. CivTech has been informed that vehicles utilize the signalized intersection of Desert Fairways Drive and Lincoln Drive to access the west driveway after driving past 56th Street and the Mountain Shadows East Driveway. Vehicles travelling west on Lincoln Drive and miss the entrance to the resort, either from 56th Street or the east driveway, typically attempt to make a U-turn at the first opportunity. Most of the U-turns observed occurred at the median break west of the site and east of the signalized intersection of Desert Fairways Drive and Lincoln Drive; there is a "No U-turn" sign at this location. The U-turns in this area have caused issues with the neighbors in the area and also create an unsafe situation for other vehicles on Lincoln Drive.

The specific objectives of the study are:

- To determine if enough vehicles are making U-turns at Desert Fairways Drive on Lincoln Drive to warrant the addition of a median break and dedicated westbound left turn lane at the Mountain Shadows West Driveway.
- To recommend additional street or traffic control improvements, where necessary, to mitigate existing and future delays at all study intersections.

Study Area

This study technically does not require a full TIA based alone on the addition of the new Bianco Restaurant. However, since U-turns have been observed at the signalized intersection of Desert Fairways Drive and Lincoln Drive and the need for a westbound left turn lane into the site at the west driveway is being assessed, a full TIA has been performed. The following study area intersections have been evaluated:

- Desert Fairways Drive & Lincoln Drive
- Mountain Shadows West & Lincoln Drive
- Mountain Shadows East & Lincoln Drive
- 56th Street & Lincoln Drive

Horizon Years

The proposed opening of the new Bianco Restaurant is spring of 2020, therefore an opening year of 2020 will be analyzed.

A location map of the study area is provided in **Figure 1**.



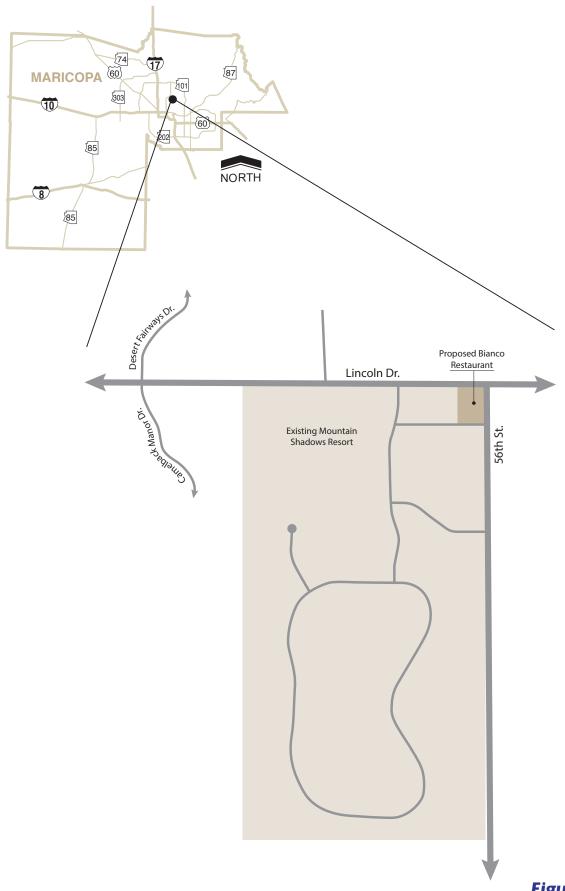


Figure 1: Vicinity Map



EXISTING CONDITIONS

SURROUNDING LAND USE

The surrounding area includes various land uses. Directly north of the site, on the north side of Lincoln Drive, is the site for the Colonia Miramonte housing development and El Chorro Restaurant. South, East, and West of the site are detached single-family homes.

EXISTING ROADWAY NETWORK

The existing roadway network analyzed in this study includes Desert Fairways Drive, Lincoln Drive, and 56th Street.

Desert Fairways Drive is a north-south road with one lane in each travelling direction. Desert Fairways Drive begins at the intersection with Tatum Boulevard and continues east for approximately 0.25 miles before looping to the south and continuing south until the intersection with Lincoln Drive, at which point it transitions to Camelback Manor Drive. The posted speed limit is 25 miles per hour along Desert Fairways Drive. Once becoming Camelback Manor there is no posted speed limit.

Lincoln Drive is an east-west four-lane major arterial with two lanes in each travelling direction. Within the vicinity of the site, there are raised medians along portions of the road. Lincoln Drive begins just east of the State Route 51 freeway and continues east for approximately 7 miles before terminating at the intersection with Cattletrack Road, just west of the Arizona Canal. The posted speed limit is 40 miles per hour (mph) within the vicinity of the site.

56th Street is a north-south two-lane collector road with one lane in each travelling direction. 56th Street begins just north of Lincoln Drive at the El Chorro Restaurant driveway and continues south for approximately 0.5 miles before terminating at the intersection with McDonald Drive. The posted speed limit is 25 mph within the vicinity of the site.



EXISTING INTERSECTION CONFIGURATION

The intersection of **Desert Fairways Drive and Lincoln Drive** is a four-legged signalized intersection with permissive/protected left turns on the eastbound approach and permissive phasing on the northbound, southbound and westbound approaches. The northbound approach consists of one shared left-turn/through/right-turn lane. The southbound approach consists of one dedicated left-turn lane and a shared through/right-turn lane. The eastbound and westbound approaches each have one dedicated left-turn lane, one through lane and a shared through/right-turn lane. There are pedestrian crosswalks across all legs of the intersection.

The intersection of **Mountain Shadows West and Lincoln Drive** is a three-legged, stop-controlled "T" intersection with free movements on the eastbound and westbound approaches and a stop sign on the northbound approach. The northbound approach consists of one right turn lane. The eastbound approach consists of two through lanes and a dedicated right-turn lane. The westbound approach has two through lanes and a median to restrict left-turns into the site.

The intersection of **Mountain Shadows East and Lincoln Drive** is a three-legged, stop-controlled "T" intersection with free movements on the eastbound and westbound approaches and a stop sign on the northbound approach. The northbound approach consists of one shared left-turn/right-turn lane. The eastbound approach consists of two through lanes and a dedicated right-turn lane. The westbound approach consists of one dedicated left-turn lane and two through lanes.

The intersection of **56**th **Street and Lincoln Drive** is a four-legged signalized intersection with permissive phasing on all approaches. The northbound approach consists of one shared left-turn/through lane and a dedicated right-turn lane. The southbound approach consists of one shared left-turn/through/right-turn lane. The eastbound and westbound approaches each consist of a dedicated left-turn lane, one through lane and a shared through/right-turn lane.

The existing intersection configurations and traffic control is illustrated in Figure 2.

EXISTING TRAFFIC VOLUMES

CivTech engaged Field Data Services of Arizona, Inc. to record traffic volumes at the four study intersections within the project vicinity. Peak hour volume turning movement counts were performed from 7:00-9:00 AM and 4:00-6:00 PM on Thursday, March 7, 2019. Peak hour turning movement counts were conducted at the following study intersections:

- Desert Fairways Drive & Lincoln Drive
- Mountain Shadows West Driveway & Lincoln Drive
- Mountain Shadows East Driveway & Lincoln Drive
- 56th Street & Lincoln Drive



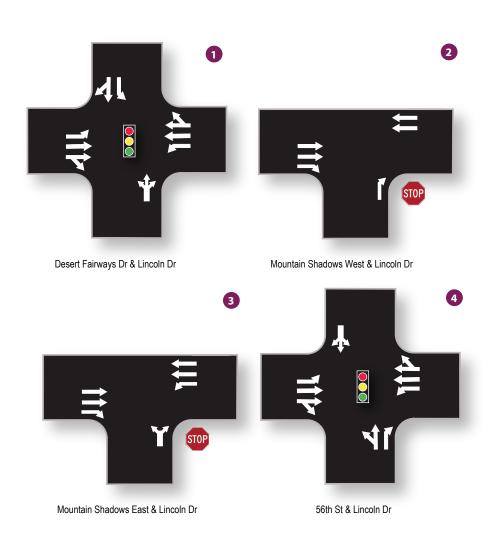
Due to the nature of this study, U-turn data was also collected at the following intersections to determine if an eastbound left turn lane into the Mountain Shadows Resort site would be warranted. Field Data Services of Arizona, Inc. recorded U-turns at the following intersection from 7:00-9:00 AM and 4:00-6:00 PM on Thursday, March 7, 2019.

Desert Fairways Drive & Lincoln Drive

CivTech recorded U-turn data at the median break west of the Mountain Shadows West Driveway, but east of the intersection with Desert Fairways Drive from 7:00-9:00 AM and 4:00-6:00 PM on Thursday, March 7, 2019. These counts were conducted because it was noted by the client that U-turns also occur at this location.

The Town of Paradise Valley requires that a seasonal adjustment factor be applied to existing traffic counts taken outside of typical months. These traffic counts were conducted in March, and winter months typically have higher amounts of traffic due to the high number of seasonal visitors. The City of Scottdale 2016 Average Daily Traffic Volumes seasonal adjustment factor for March is 0.93. Existing 2019 traffic volumes with the seasonal adjustment factor applied are presented in **Figure 3** for the weekday AM and PM peak hours. Raw traffic volume data obtained for this study has been included in **Appendix B**.





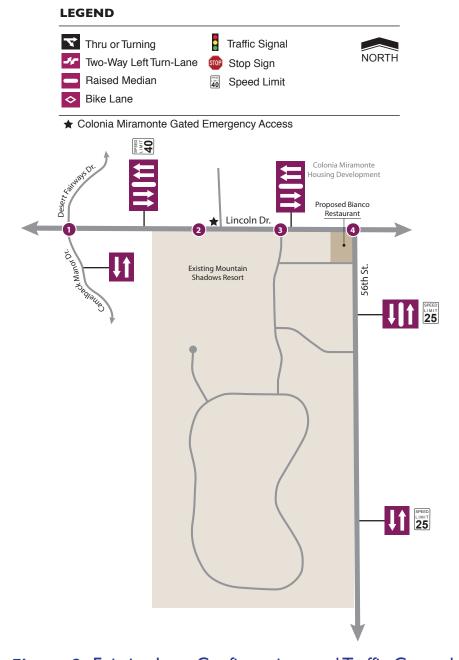
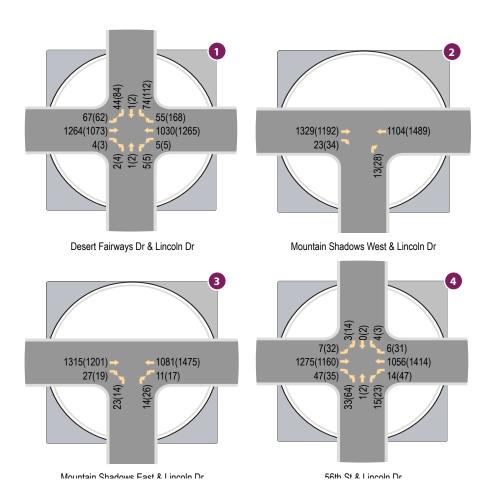


Figure 2: Existing Lane Configurations and Traffic Controls









XX(XX) - AM(PM) Peak Hour Traffic Volumes

►X(X) - AM(PM) Peak Hour U-Turn Volumes

Note: U-Turns not included in the left turning Volumes at Intersection "1"

★ Colonia Miramonte Gated Emergency Access

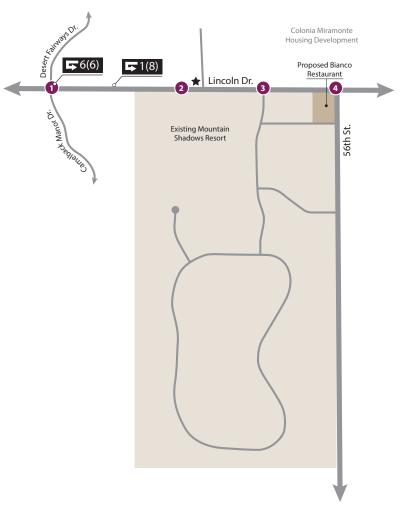


Figure 3: Existing Traffic Volumes



EXISTING CAPACITY ANALYSIS

Peak hour capacity analyses have been conducted for the study intersections based on existing intersection configurations and traffic volumes. All intersections have been analyzed using the methodologies presented in the *Highway Capacity Manual (HCM), Special Report 209,* and Updated 2016 and using Synchro software, version 10.0 under the HCM 6th edition methodology.

The concept of level of service (LOS) uses qualitative measures that characterize operational conditions within the traffic stream. The individual levels of service are described by factors that include speed, travel time, freedom to maneuver, traffic interruptions, and comfort and convenience. Six levels of service are defined for each type of facility for which analysis procedures are available. They are given letter designations A through F, with LOS A representing the best operating conditions and LOS F the worst. Each level of service represents a range of operating conditions. Levels of service for intersections are defined in terms of delay ranges. **Table 1** lists the level of service criteria for signalized and unsignalized intersections, respectively.

Table 1: Level of Service Criteria

Control Delay (seconds/yehi

Level of Service	Control Delay (seconds/vehicle)		
Level of Service	Signalized Intersections	Unsignalized Intersections	
А	≤ 10	≤ 10	
В	> 10-20	> 10-15	
С	> 20-35	> 15-25	
D	> 35-55	> 25-35	
E	> 55-80	> 35-50	
F	> 80	> 50	

Source: Exhibit 19-8, Exhibit 20-2, Exhibit 21-8 and Exhibit 22-8, Highway Capacity Manual 2017

Synchro 10.0 software calculates the LOS per the HCM 6th edition methodology. The 6th edition HCM documents the signalized LOS calculation methodology which takes into account lane geometry, traffic volumes and cycle length/phasing to compute LOS. Synchro analysis worksheets report individual movement delay/LOS and overall delay/LOS for signalized intersections; unsignalized intersection worksheets report the worst-case delay/LOS and the average overall intersection delay. Signal timing data for the intersections of Lincoln Drive/Desert Fairways Drive and Lincoln Drive/56th Street were provided by the Town of Paradise Valley. Results of the existing level of service analyses are shown in **Table 2** for both AM and PM peak hours. The existing conditions analysis worksheets have been included in **Appendix C**.



Intersection **Existing LOS** Approach/ ID Intersection Control Movement AM (PM) C(C) NB C(C)SB A(A)1 EΒ Desert Fairways Dr & Lincoln Dr Signal WB B(B) Overall B(B) Mountain Shadows West Drwy & 1-way stop 2 **NB** Right B(B) Lincoln Dr (NB) Mountain Shadows East Drwy & 1-way stop NB Shared C(A) 3 Lincoln Dr (NB) WB Left A(A)NB C(C)SB C(C) 56th St & Lincoln Dr A(A) 4 Signal EΒ A(A) WB Overall A(A)

Table 2: Existing Peak Hour Levels of Service

The results of the existing conditions analysis summarized in **Table 2** indicates that all intersections currently operate at an overall acceptable level of service (LOS C or better) under existing lane configurations and signal timing.

PROPOSED DEVELOPMENT

SITE LOCATION

The proposed development will be located at 5445 East Lincoln Drive in the Town of Paradise Valley, Arizona on the same site as the existing Mountain Shadows Resort.

SITE ACCESS

There are two existing access points for this development along Lincoln Drive, described as follows:

- <u>The West Access</u> is an existing right in/right out only access along Lincoln Drive located approximately 855 feet east of the intersection of Desert Fairways Drive and Lincoln Drive.
- <u>The East Access</u> is an existing full movement access along Lincoln Drive located approximately 500 feet east of the west access.

The two existing Mountain Shadows Driveways will remain in their existing location, as well as a third access on 56th Street on the eastern border of the site. Access to 56th Street from the Mountain Shadows Resort was not evaluated based on a scoping meeting for this study with Town staff. Vehicular movement to/from the 56th Street access are anticipated to remain the same and operate well in the existing conditions. The existing west access is proposed to include a dedicated westbound left turn lane to allow for left turns into the site while still restricting northbound left turns out of the site. All other access points are proposed to remain the same.

The site plan and access points are depicted in Figure 4.





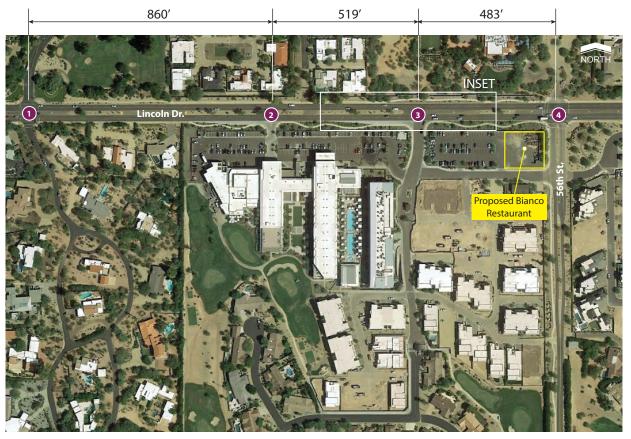


Figure 4: Site Plan and Access



TRIP GENERATION

The potential trip generation for the proposed development was estimated utilizing the Institute of Transportation Engineers (ITE) *Trip Generation Manual*, 10th Edition and *Trip Generation Handbook*, 3rd Edition. The ITE *Trip Generation Manual* contains data collected by various transportation professionals for a wide range of different land uses. The data are summarized in the report and average rates and equations have been established that correlate the relationship between an independent variable that describes the development size and generated trips for each categorized land use. The report provides information for daily and peak hour trips.

Mountain Shadows Resort is an already existing resort with a 183-room hotel and 41 condominiums. A new Bianco restaurant is proposed for the northeast corner of the site. The restaurant will consist of 3,500 square feet (sf) of indoor seating and 1,500 sf of patio seating. The land use code (LUC) used for this restaurant is 931 for a quality restaurant. For this ITE LUC, there is no data provided for the entering/exiting percentage during the AM peak hour. This is the case for the last three editions of the Trip Generation Manual. Since no data is provided, the percentage of vehicles entering during the AM peak hour of generator was used. The peak hour of generator is typically the peak hour calculated based on the specific land use and not the typical peak hour between 7:00 and 9:00 AM. **Table 3** depicts the trip generation summary for the proposed development. Trip generation calculations are provided in **Appendix D**.

Weekday Trips Daily AM PM ITE **Proposed Use** Size Units Total Out Total LUC In In Out Total 5,000 SF 3 **Quality Restaurant** 931 420 1 4 26 13 39 3 1 Subtotals 420 4 26 13 39

Table 3: Trip Generation Summary

As shown in **Table 3**, the proposed development is anticipated to generate approximately 420 additional weekday daily trips, with 4 (3 in/1 out) additional trips occurring in the AM peak hour and 39 (26 in/13 out) additional trips occurring in the PM peak hour.

TRIP DISTRIBUTION AND ASSIGNMENT

A single trip distribution pattern was assumed for the proposed development. It is expected that the quality restaurant will generate trips based on future population within a 10-mile radius of the site. Future total population within a 10-mile radius of the site, as predicted by the 2030 socio-economic data compiled by the Maricopa Association of Governments (MAG), was used as a basis to estimate trip distribution for the resort development

The resulting trip distribution percentages for the study area are shown in **Table 4**. The trip distribution calculations are included in **Appendix E**.



Table 4: Site Trip Distribution

Direction (To/From)	Trip Distribution
East on Lincoln Drive	37%
West on Lincoln Drive	49%
North on Desert Fairways Drive	2%
South on Camelback Manor Drive	2%
South on 56th Street	10%
Total	100%

Figure 5 illustrates the trip distribution percentages shown in **Table 4** on the existing roadway network with the study area. The percentages presented in **Figure 5** were applied to the site trips generated to determine the AM and PM peak hour site traffic at the intersections within the study area. The resulting site generated traffic for the proposed development are presented in **Figure 6**.



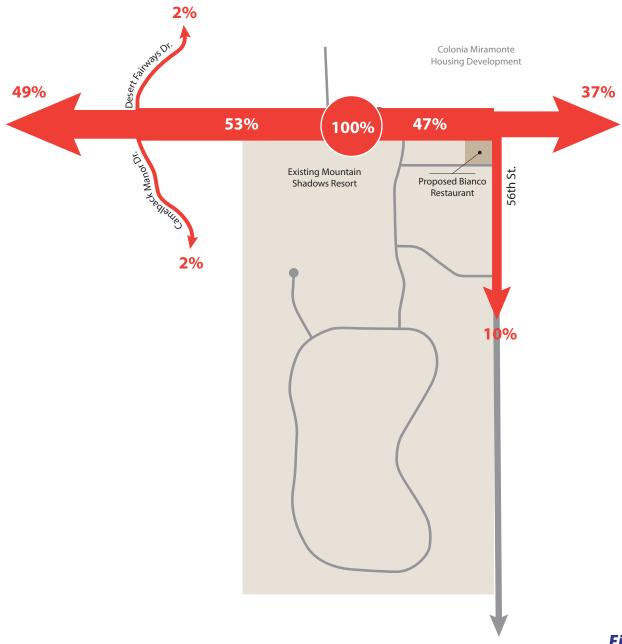
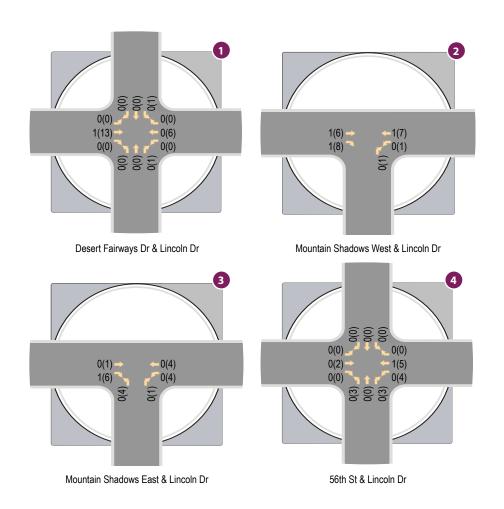


Figure 5: Trip Distribution





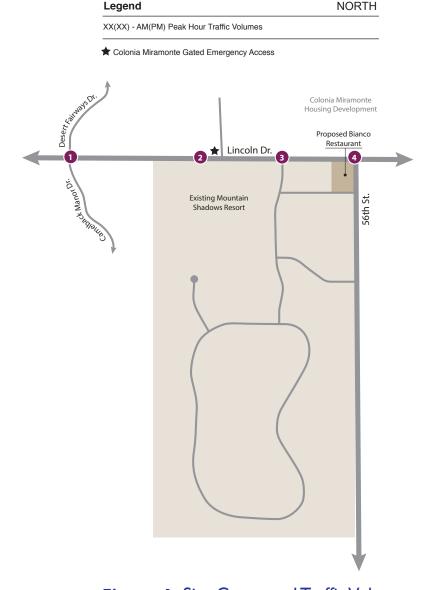


Figure 6: Site Generated Traffic Volumes



FUTURE BACKGROUND TRAFFIC

CivTech applied a growth rate to the seasonally adjusted traffic counts for this study in order to obtain the background traffic volumes along the adjacent roadway network. The Town of Paradise Valley does not publish historic traffic data; for this project, the growth rates were determined using the City of Scottsdale traffic data. In reviewing the City of Scottsdale Traffic Counts Map, a 1.7% average growth rate was found on Scottsdale Road between Indian Bend Road and Lincoln Drive. **Table 5** shows the expansion factors used for the proposed opening year 2020.

Table 5: Growth Rate Expansion Factors

Horizon Year	Expansion Factor
2020	1.017

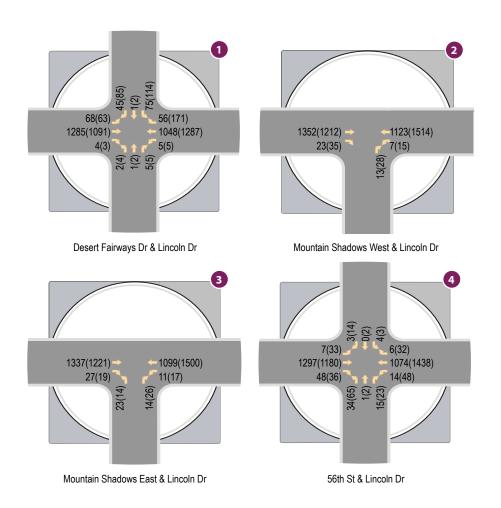
Applying the growth rate expansion factors to the seasonally adjusted existing traffic volumes predicts the volume of traffic anticipated on the surrounding area roads for opening year 2020. It was assumed that all vehicles making U-turns at the median break west of the site and at the intersection of Desert Fairways Drive and Lincoln Drive were negotiating the movement to return to the Mountain Shadows Resort because they had missed an earlier left-in access. Therefore, the U-turns counted at the median break west of the site and at the intersection of Desert Fairways Drive and Lincoln Drive were re-assigned as westbound left turns into the site at the Mountain Shadows West Driveway in order to determine the feasibility of a median break and the addition of westbound left turn lane.

Calculated background traffic for the opening year 2020 is included in **Figure 7**. More detailed calculation sheets are included in **Appendix F**.

TOTAL TRAFFIC

Total traffic was determined by adding the site generated traffic to the projected background traffic. Total peak hour traffic volumes for the opening year 2020 are shown in **Figure 8**.





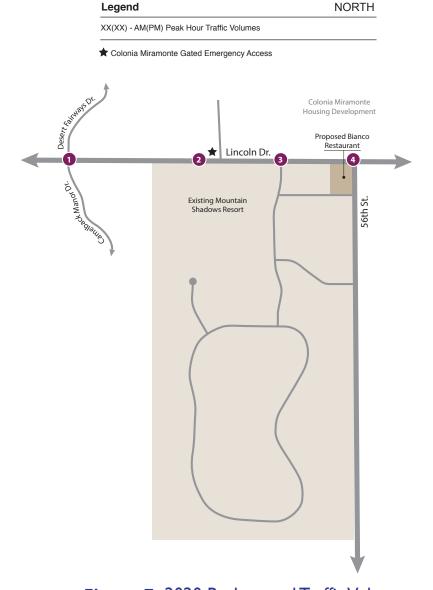
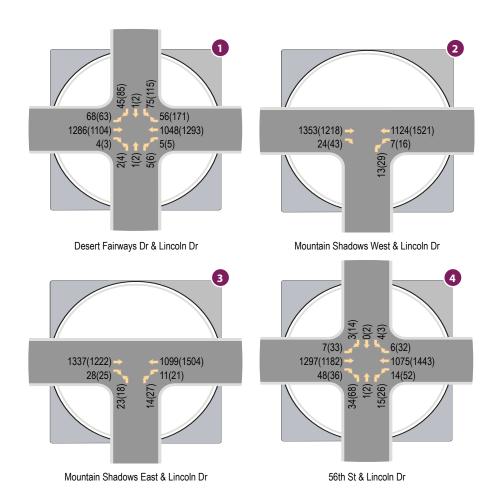
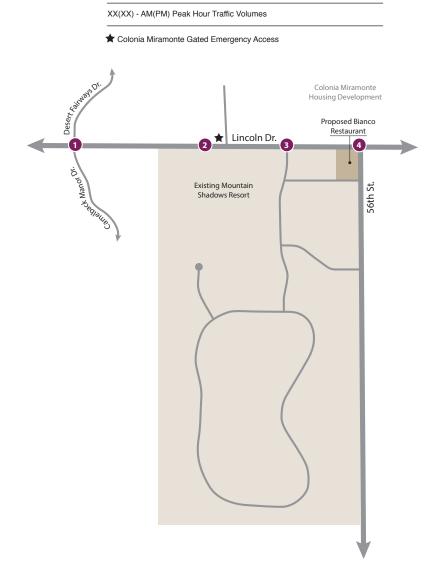


Figure 7: 2020 Background Traffic Volumes







Legend

Figure 8: 2020 Total Traffic Volumes



TRAFFIC AND IMPROVEMENT ANALYSIS

INTERSECTION CAPACITY ANALYSIS

Peak hour capacity analyses have been conducted for all of the intersections within the study area. All study area intersections were analyzed using Synchro 10.0 analysis software and the methodologies previously presented. The mitigation measures proposed for the existing conditions was applied to the future conditions. The overall intersection and approach levels of service are summarized in **Table 6** for the 2020 opening year. Detailed analysis worksheets can be found in **Appendix G**.

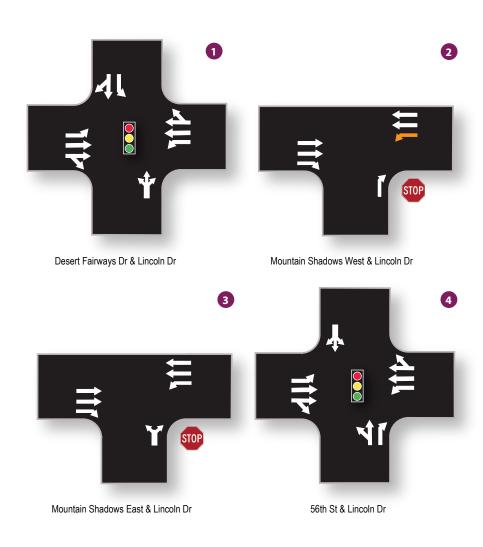
Table 6: 2020 Peak Hour Analysis

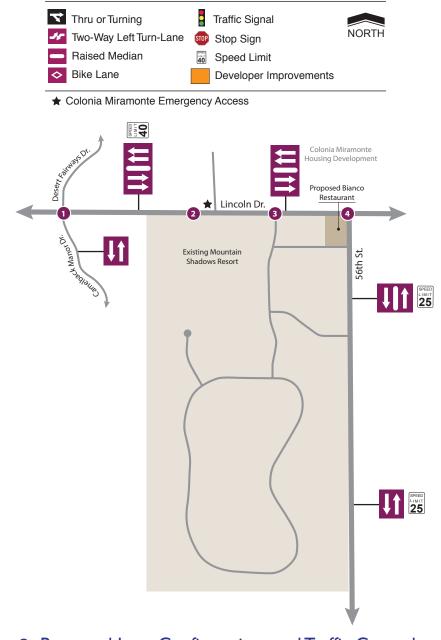
	Intersection Approach/ 2020 LOS		S AM(PM)		
ID	Intersection	Control	Movement	No-Build	Build
			NB	C(C)	C(C)
	Docort Fairways Dr 8		SB	C(C)	C(C)
1	Desert Fairways Dr & Lincoln Dr	Signal	EB	A(A)	A(A)
	LINCOIN DI		WB	B(B)	B(B)
			Overall	B(B)	B(B)
2	Mountain Shadows West	1-way stop	NB Right	B(B)	B(B)
	Drwy & Lincoln Dr	(NB)	WB Left	A(A)	A(A)
3	Mountain Shadows East	1-way stop	NB Shared	C(A)	C(A)
3	Drwy & Lincoln Dr	(NB)	WB Left	A(A)	A(A)
			NB	C(C)	C(C)
	56 th St & Lincoln Dr	Signal	SB	C(C)	C(C)
4			EB	A(A)	A(A)
			WB	A(A)	A(A)
			Overall	A(A)	A(A)

The results of the 2020 opening year Synchro analysis summarized in **Table 6** indicates that all study intersections are anticipated to experience an acceptable level of service LOS C or better.

The proposed lane configurations are presented in Figure 9.







LEGEND

Figure 9: Proposed Lane Configurations and Traffic Controls



LEFT TURN LANE ANALYSIS

The Town of Paradise Valley does not provide explicit criteria for the installation of dedicated left turn lanes; they typically defer to City of Scottsdale design standards.

According to the City of Scottsdale Design Standards and Policies Manual (DS&PM), left turn lanes should be determined based on anticipated turning volume and whether there is signalized or unsignalized traffic control. Per the DS&PM, driveways located on arterial roadways must be spaced 330 feet apart, with a minimum spacing of 250 feet with no restrictions. The eastern and western driveways for the Mountain Shadows Resort are spaced approximately 530 feet apart, meaning that a left turn lane into the site at the western driveway could be permitted. A dedicated westbound left turn lane is also safer than making a U-turn at an unprotected signalized intersection or at a median break, which was observed to be happening during both the AM and PM peak hours.

The U-turns counted at the intersection of Desert Fairways Drive and Lincoln Drive, as well as the U-turns counted at the median break just east of this intersection, were assumed to become westbound left turns at the Mountain Shadows West Driveway. These U-turns and the site trips assigned to this driveway were used to determine the need for a left turn lane. In total, there are assumed to be approximately 16 westbound left turns at the West Driveway during the PM peak hour with 1,175 oncoming eastbound trips.

City of Scottsdale DS&PM Chapter 5, Section 5-3.201, Driveway Spacing has been included in **Appendix H**. The proposed westbound left turn lane concept prepared by CivTech has been included in **Appendix I**.



QUEUE LENGTH ANALYSIS

Adequate turn storage should be supplied on any approach where turn lanes are permitted and/or warranted. A queuing analysis was performed for all warranted/recommended and existing intersection turn lanes where site traffic is expected. According to the methodology documented in *A Policy on Geometric Design of Highways and Streets* (the AASHTO "Green Book"), the storage length for a turn lane is typically estimated as the length required to hold the average number of arriving vehicles per two minutes, where unsignalized, or per one-and-a half signal cycles, where signalized. The formulas used for the calculations are shown below.

For signalized intersections, the storage length is determined by the following formula:

Storage Length = [1.5 x (veh/hr)/(cycles/hr)] x 25 feet

For unsignalized intersections, the storage length is determined by the following formula:

Storage Length = [(veh/hr)/(30 periods/hr)] x 25 feet

Using the traffic volumes and lane configurations projected for the 2020 horizon year, the resulting turn lane storage for turn movements affiliated with the site using AASHTO guidelines were calculated with a 65-second cycle length and are summarized in **Table** 7. Calculations for the queue storage length recommendations are provided in **Appendix J.**

				Queue Storage			
ID	Intersection	Intersection Control	Movement	Existing (1)	AASHTO	95 th Percentile	Recommended
1	Desert Fairways Dr & Lincoln Dr	Signalized	SB Left EB Left WB Left	75' 150' 55'	125' 75' 25'	80' 30' <25'	⁽²⁾ 75' 150' 55'
2	Mountain Shadows West Drwy & Lincoln Dr	1-way stop (NB)	WB Left EB Right	- 115'	25' 50'	<25' <25'	⁽³⁾ 75' 115'
3	Mountain Shadows East Drwy & Lincoln Dr	1-way stop (NB)	WB Left EB Right	75' 100'	25' 25'	<25' <25'	75' 100'
4	56 th St & Lincoln Dr	Signalized	EB Left WB Left NB Right	45' 75' 285'	50' 50' 25'	<25' 30' <25'	45' 75' 285'

Table 7: Queue Storage Lengths

The recommended storage lengths in **Table 7** are provided for horizon year 2020 using the total traffic projections. The minimum storage length recommendation for the proposed westbound left turn lane at the Mountain Shadows West Driveway is 75 feet, or enough storage for 3 vehicles at a time.

¹ The American Association of Highway and Transportation Officials on pages 714-715 of its publication, *Geometric Design of Highways and Streets* ("AASHTO Green Book"), indicates that storage length for a turn lane, exclusive of taper, "should usually be based on one and one-half to two times the average number of vehicles that would store per cycle" at a signalized intersection.



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⁽¹⁾ Measured from stop bar to end of storage length

⁽²⁾ Not the responsibility of the developer

⁽³⁾ Minimum recommended storage length

There is an existing median break along Lincoln Drive directly east of the proposed median break at the west driveway. This median break is utilized as emergency access for the Colonia Miramonte housing development north of the Mountain Shadows Resort. Since this is a gated emergency access only not available to residents for daily usage, it was determined this median break can be utilized in the design of a westbound left turn lane into the Mountain Shadows Resort west driveway. The determination was provided by the Town based on concept designs provided by the Town's consultant as part of earlier review comments for this project. A minimum of 75 feet of storage is recommended with a 90-foot taper for a total length of 165 feet. The median located in the vicinity of the Colonia Miramonte emergency access should be constructed with mountable curb to allow access, if needed, by emergency vehicles.



CONCLUSIONS

The following conclusions have been documented for this study.

General

• The proposed restaurant is anticipated to generate approximately 420 additional weekday daily trips, with 4 (3 in/1 out) additional trips occurring in the AM peak hour and 39 (26 in/13 out) additional trips occurring in the PM peak hour.

Existing Conditions

• The results of the existing conditions analysis summarized in **Table 2** indicates that all intersections currently operate at an overall acceptable level of service (LOS C or better) under existing lane configurations and signal timing.

Opening Year 2020

 The results of the 2020 opening year Synchro analysis indicates that all study intersections are anticipated to experience an acceptable level of service LOS C or better.

Left Turn Lane Analysis

- The Town of Paradise Valley does not provide explicit criteria for the installation of dedicated left turn lanes; they typically defer to City of Scottsdale design standards.
 - Per the City of Scottsdale Design Standards and Policy Manual (DS&PM), driveways located on arterial roadways must be spaced 330 feet apart, with a minimum spacing of 250 feet with no restrictions. The eastern and western driveways for the Mountain Shadows Resort are spaced approximately 530 feet apart, meaning that a left turn lane into the site at the western driveway should be permitted. A dedicated westbound left turn lane is also safer than making a U-turn at an unprotected signalized intersection or at a median break, which was observed to be happening during both the AM and PM peak hours.

Queue Storage

- The recommended storage lengths are provided for horizon year 2020 using the total traffic projections.
 - There is an existing median break along Lincoln Drive directly east of the proposed median break at the west driveway. This median break is utilized as emergency access for the Colonia Miramonte housing development north of the Mountain Shadows Resort. Since this is a gated emergency access only not available to residents for daily usage, it was



determined this median break can be utilized in the design of a westbound left turn lane into the Mountain Shadows Resort west driveway. The determination was provided by the Town based on concept designs provided by the Town's consultant as part of earlier review comments for this project. A minimum of 75 feet of storage is recommended with a 90-foot taper for a total length of 165 feet.

o It is recommended that the median be constructed with mountable curb in the areas proximate to the Colonia Miramonte emergency access.



LIST OF REFERENCES

Highway Capacity Manual. Transportation Research Board, Washington, D.C., 2000.

Manual on Uniform Traffic Control Devices. U.S. Department of Transportation, Federal Highways Administration, Washington, D.C., 2009.

Roadway Design Manual, Maricopa County Department of Transportation, Phoenix, Arizona, Revised April 2004.

Trip Generation Manual, 10th Edition, Institute of Transportation Engineers, Washington, D.C., 2016.

Trip Generation Handbook, 3nd *Edition*, Institute of Transportation Engineers, Washington, D.C., 2016.



TECHNICAL APPENDIX

APPENDIX A: REVIEW COMMENTS AND RESPONSES

APPENDIX B: EXISTING TRAFFIC COUNTS

APPENDIX C: EXISTING PEAK HOUR ANALYSIS

APPENDIX D: TRIP GENERATION

APPENDIX E: TRIP DISTRIBUTION

APPENDIX F: BACKGROUND TRAFFIC

APPENDIX G: 2020 PEAK HOUR ANALYSIS

APPENDIX H: SCOTTSDALE DS&PM SECTION 5-3.201

APPENDIX I: LEFT TURN LANE DESIGN CONCEPT

APPENDIX J: QUEUE STORAGE ANALYSIS

APPENDIX K: SUP ACCESS DIAGRAMS



APPENDIX A

REVIEW COMMENTS AND RESPONSES



Mountain Shadows 2nd Submittal

CivTech, Inc.

Review Comments & Responses

Disposition Codes: (1) Will Comply (2) Will Evaluate (3) Delete Comment (4) Defer to Consultant/Owner

Reviewer Name, Agency: Kimley Horn on behalf of Town of Paradise Valley

Item	Review Comment	(Code) & Response
1.	Pg 1: Queue storage - based on assessor, this access [Marriott Resort emergency access] falls within the Colonia Miamonte Owners Association	(1) Marriott Resort reference was changed to Colonia Miramonte housing development.
2.	Pg 1: Queue Storage - Using ADOT's criteria for warranting left turn median break is really not appropriate. Access will be based on spacing, volumes, safety, proximity to other driveways and proximity to other signalized intersection. Like COS DS&PM indicated, the Transportation Research Board (TRB), Access Management manual would be a more appropriate document to utilize in the absence of access related criteria. It should also be noted that ADOT has access criteria for median break locations. Main concern is how the proposed left turn will work with the existing emergency access location.	(2) City of Scottsdale Design Standards and Policies Manual (DS&PM) was used to detrmine driveway spacing requirements instead of determining the need for a left turn lane using ADOT criteria. The recommendation for this turn lane is also based on the safety of vehicles since many vehicles now are making U-turns at Desert Fairways/Lincoln and Lincoln is not neccessarily wide enough to be making this movement. Analysis proposed for the access indicate acceptable operations and sight distance. The emergency access will continue to be operational, but will now be restricted by a median, as shown in the latest median design plans being prepared by Kimley Horn. A mountalbe median is recommended in this location.
3.	Pg 2: Queue storage - This is not an accurate statement as emergency response may come from either direction depending on which fire station responds based on call volume and if existing calls for service are responded to.	(1) Statement was revised to indicate that emergency vehicles could approch the access from either the east or the west
4.	Pg 2: Queue storage - see comments in Figure 3, the volumes are not balancing between access #2 and #3. Looking at aerials, I do not see any	(1) Volumes were balanced between both access points and 56th Street intersection on Lincoln Drive. The emergency access is gated without an opportunity for interim use by the residents.
5.	Pg 2: Queue storage - How would left turns out of Mountain Shadows be prohibited?	(1) Left turns out of the site will be prohibited by the median design, an Appendix K has been added to show the full design and the site plan figure will be updated with the median design.
6.	Pg 5: change "Marriott Resort" to "CM HOA and El Chorro Restaurant"	(1) Reference in text was changed
	Pg 5: Desert Fairways has a posted speed limit of 25 mph, Camelback Manor has no posted speed limit	(1) Speed limit for Desert Fairways Drive was updated



Reviewed Date: 06/03/2019 CivTech Received Date: 06/03/2019 CivTech Entered Date: 11/01/2019 CivTech Response Date: 11/20/2019

Mountain Shadows 2nd Submittal

CivTech, Inc.

Review Comments & Responses

Disposition Codes: (1) Will Comply (2) Will Evaluate (3) Delete Comment (4) Defer to Consultant/Owner

Reviewer Name, Agency: Kimley Horn on behalf of Town of Paradise Valley

Item	Review Comment	(Code) & Response
8.	Pg 5: change "a private property" to "El Chorro Restaurant"	(1) Reference in text was changed
9.	Pg 6: Please verify seasonal factor with MAG. The most recent MAG	(3) After discussion with Paul Mood from the Town of Paradise Valley, it
	volume map (2015) does not provide factor and the moset recent MAG	was agreed that the City of Scottsdale seasonal adjustment factor could
	map with adjust is 2011 and says seasonal factor is 1.037	be used in place of the MAG adjustment factor. An adjustment factor of
		0.93 was used per City of Scottsdale DS&PM.
10.	Figure 2: show CM HOA emergency access and internal roadways	(1) This location has been included on the figures
11.	Figure 3: The volumes between the intersections do not add up or	(1) Volumes were balanced between both access points and 56th Street
	balance. I am especially questioning what is occuring between access #2	intersection on Lincoln Drive. The emergency access is gated without an
	and #3. Is there more traffic using the emergency access?	opportunity for interim use by the residents.
	Figure 3: Clarify - is the 6 u-turns in addition to are part of the 5 lefts?	(1) The 6 U-turns observed at the intersection of Desert Fairways Drive
	Synchro output show 5 lefts.	and Lincoln Drive are not included in the existing westbound left turn
		volume presented in Figure 3. The U-turns were included as westbound
		left turns at the western Mountain Shadows driveway for the background
		and total scenarios.
	Pg 10: LOS D or LOS C?	(1) LOS D was changed to LOS C
14.	Pg 10: SUP access diagram has primary access off of 56th Street	(1) Text was added to indicate that Paul Mood has agreed to allow
		CivTech to not analyze the access off of 56th Street since the addition of
		the left turn will not change the operation of anticipated volumes at the driveway on 56th Street.
15	Pg 10: indicate both access points are off of Lincoln Drive	(1) Text included to indicate that both access points that were analyzed
10.	1 g 10. Indicate both access points are on or Emocrit brive	are on Lincoln Drive
16.	Pg 10: show volumes entering/exiting third access on 56th Street and	(3) Text was added to indicate that Paul Mood has agreed to allow
	include in traffic analysis.	CivTech to not analyze the access off of 56th Street since the addition of
		the left turn will not change the operation of anticipated volumes at the
		driveway on 56th Street.
17.	Pg 10: provide layout of proposed left turn median break	(1) Figure 4 will be updated to show the proposed left turn median break
		on Licoln Drive. The concept design is also included in Appendix K.



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Mountain Shadows 2nd Submittal

CivTech, Inc.

Review Comments & Responses

Disposition Codes: (1) Will Comply (2) Will Evaluate (3) Delete Comment (4) Defer to Consultant/Owner

Reviewer Name, Agency: Kimley Horn on behalf of Town of Paradise Valley

Item	Review Comment	(Code) & Response
18.	Figure 4: show 56th Street access point and add dimensions between the	(1) Dimensions have been added to Figure 4 showing the driveway
	access points	spacing
19.	Figure 4: labeled currently at Figure 5	(1) Figure label has been updated to show correct figure number
20.	Figure 6: show site generated traffic at 56th Street access point	(3) Text was added to indicate that Paul Mood has agreed to allow
		CivTech to not analyze the access off of 56th Street since the addition of
		the left turn will not change the operation of anticipated volumes at the
		driveway on 56th Street.
	Figure 6: with a driveway (#3) close to the proposed restaurant, why is	(2) While most vehicles will be making right turns at the second access
	right turn traffic using #2 versus #3 to gain access to the restaurant? They	<u> </u>
	have to drive through parking lot to get to site?	turn there without knowledge that there is a second access point on
		Lincoln Drive. This is typical of driver behavior. Moving right turn vehicles
		from access 2 to access 3 does not impact the operational analysis of
		either intersection.
	, , , , , , , , , , , , , , , , , , , ,	(1) Text was added to indicate that all U-turns were assumed to be
	just an assumption?	making right turns into the Mountain Shadows Resort and that this was an
		assumption.
23.	Pg 21: Town of Paradise Valley "typically" defers to City of Scottsdale	(1) "Typically" was added to text to be more descriptive.
24.	Pg 21: As previously commented, this is access control. City of Scottsdale	(1) City of Scottsdale access control standards were utilized instead of
		ADOT criteria.
	requires for median breaks. In the absence of criteria associated with	
	access use TRB	
25.	Pg 21: U-turn vehicles actually observed going to mountain shadows west	(2) As previously mentioned, this was an assumption and the text has
	driveway?	been added to clarify the assumption.
26.	Pg 22: 75 feet recommended in table, 50 feet recommended in text.	(1) Text was updated recommending 75 feet of queue storage.
	Update text to recommend 75 feet	
27.	Pg 22: update Marriott Resort references to CM HOA	(1) Reference changed.
Innovement of	Pg 23: LOS C or LOD D?	(1) LOS D was changed to LOS C.
29.	Pg 23: Was City of Phoenix turn lane criteria discussed in report?	(2) This statement was deleted as it was not relevent to the discussion in
		the TIA body text.



Reviewed Date: 06/03/2019 CivTech Received Date: 06/03/2019 CivTech Entered Date: 11/01/2019 CivTech Response Date: 11/20/2019

Mountain Shadows 2nd Submittal

CivTech, Inc.

Review Comments & Responses

Disposition Codes: (1) Will Comply (2) Will Evaluate (3) Delete Comment (4) Defer to Consultant/Owner

Reviewer Name, Agency: Kimley Horn on behalf of Town of Paradise Valley

Item	Review Comment	(Code) & Response
30. P	g 23: Left Turn Lane Analysis - Access will be dependent on what	(2) Discussion was added in the text addressing emergency access.
00	ccurs with the Colonia Mariamonte emergency access and treatment for	
cl	osure and maintaining emergency access.	



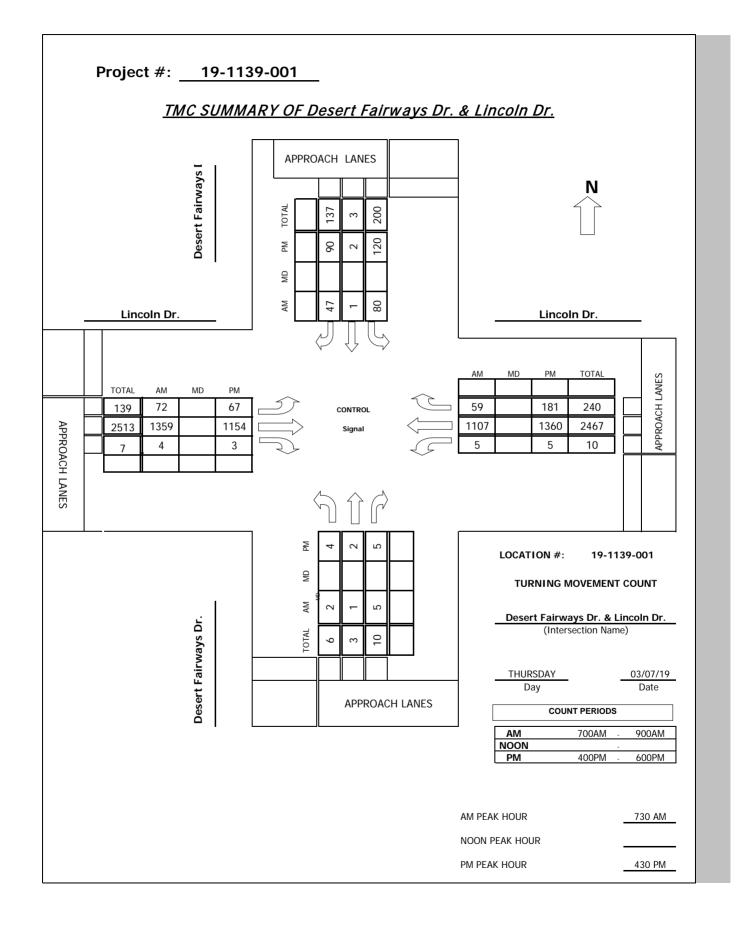
Reviewed Date: 06/03/2019 CivTech Received Date: 06/03/2019 CivTech Entered Date: 11/01/2019 CivTech Response Date: 11/20/2019

APPENDIX B

EXISTING TRAFFIC COUNTS











N-S STREET: Desert Fairways Dr. DATE: 03/07/19 LOCATION: Paradise Valley

E-W STREET: Lincoln Dr. DAY: THURSDAY PROJECT# 19-1139-001

	NORTHBOUND			SC	UTHBO	UND	E	ASTBOU	ND	W	ESTBOL	JND	
LANES:	NL 0	NT 1	NR 0	SL 1	ST 0.5	SR 0.5	EL 1	ET 2	ER 0	WL 1	WT 2	WR 0	TOTAL
6:00 AM 6:15 AM 6:30 AM 6:45 AM													
7:00 AM 7:15 AM 7:30 AM	2 1 1	0 0 0	0 3 4	14 16 15	0 0 0	6 24 16	11 10 21	307 317 375	0 0 0	3 2 3	239 252 303	6 13 18	588 638 756
7:45 AM 8:00 AM 8:15 AM	0 0 1	0 0 1	0 0 1	22 25 18	0 0 1	16 7 8	17 16 18	326 338 320	1 1 2	0 0 2	268 280 256	14 10 17	664 677 645
8:30 AM 8:45 AM	0	0	3	21 17	0	18 3	19 25	338 327	1	3 2	239 242	19 21	661 643
9:00 AM 9:15 AM 9:30 AM 9:45 AM													
10:00 AM 10:15 AM 10:30 AM 10:45 AM													
11:00 AM 11:15 AM 11:30 AM 11:45 AM													

TOTAL	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL	l
Volumes	6	1	14	148	2	98	137	2648	6	15	2079	118	5272	l
Approach %	28.57	4.76	66.67	59.68	0.81	39.52	4.91	94.88	0.21	0.68	93.99	5.33		l
App/Depart	21	/	256	248	/	23	2791	/	2810	2212	/	2183		l

AM Peak Hr Begins at: 730 AM

PEAK

Volumes 2 1 5 80 1 47 72 1359 4 5 1107 59 2742 Approach % 25.00 12.50 62.50 62.50 0.78 36.72 5.02 94.70 0.28 0.43 94.53 5.04

PEAK HR.

FACTOR: 0.400 0.842 0.906 0.904 0.907

CONTROL: Signal

COMMENT 1:

GPS: 33.531053, -111.966596

Intersection Turning Movement



N-S STREET: Desert Fairways Dr.

DATE: 03/07/19

LOCATION: Paradise Valley

E-W STREET: Lincoln Dr.

DAY: THURSDAY

PROJECT# 19-1139-001

	NO	RTHBO	UND	SO	UTHBO	UND	E	ASTBOU	ND	W	'ESTBOL	JND	
LANES:	NL O	NT 1	NR 0	SL 1	ST 0.5	SR 0.5	EL 1	ET 2	ER 0	WL 1	WT 2	WR 0	TOTAL
1:00 PM													
1:15 PM													
1:30 PM													
1:45 PM													
2:00 PM													
2:15 PM													
2:30 PM													
2:45 PM													
3:00 PM													
3:15 PM													
3:30 PM													
3:45 PM													
4:00 PM	0	1	1	18	1	15	22	245	2	1	313	30	649
4:15 PM	0	1	1	19	0	22	18	274	0	1	366	20	722
4:30 PM	0	1	1	20	0	22	10	289	0	2	383	32	760
4:45 PM	2	0	0	28	1	18	20	267	2	0	345	50	733
5:00 PM	0	1	2	48	0	24	17	285	1	2	313	40	733
5:15 PM	2	0	2	24	1	26	20	313	0	1	319	59	767
5:30 PM	1	0	1	43	1	23	21	305	1	1	294	34	725
5:45 PM	1	0	1	30	0	22	19	282	0	3	246	31	635
6:00 PM													
6:15 PM													
6:30 PM													
6:45 PM													
OTAL	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
olumes	6	4	9	230	4	172	147	2260	6	11	2579	296	5724
pproach %	31.58	21.05	47.37	56.65	0.99	42.36	6.09	93.66	0.25	0.38	89.36	10.26	
nn/Donart	10	1	117	104	1	21		/	2400		/	2757	

TOTAL	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
Volumes	6	4	9	230	4	172	147	2260	6	11	2579	296	5724
Approach %	31.58	21.05	47.37	56.65	0.99	42.36	6.09	93.66	0.25	0.38	89.36	10.26	
App/Depart	19	/	447	406	/	21	2413	/	2499	2886	/	2757	

PM Peak Hr Begins at: 430 PM

PEAK

120 90 67 1154 Volumes 2 Approach % 36.36 18.18 45.45 56.60 0.94 42.45 5.47 94.28 0.25 0.32 87.97 11.71

PEAK HR.

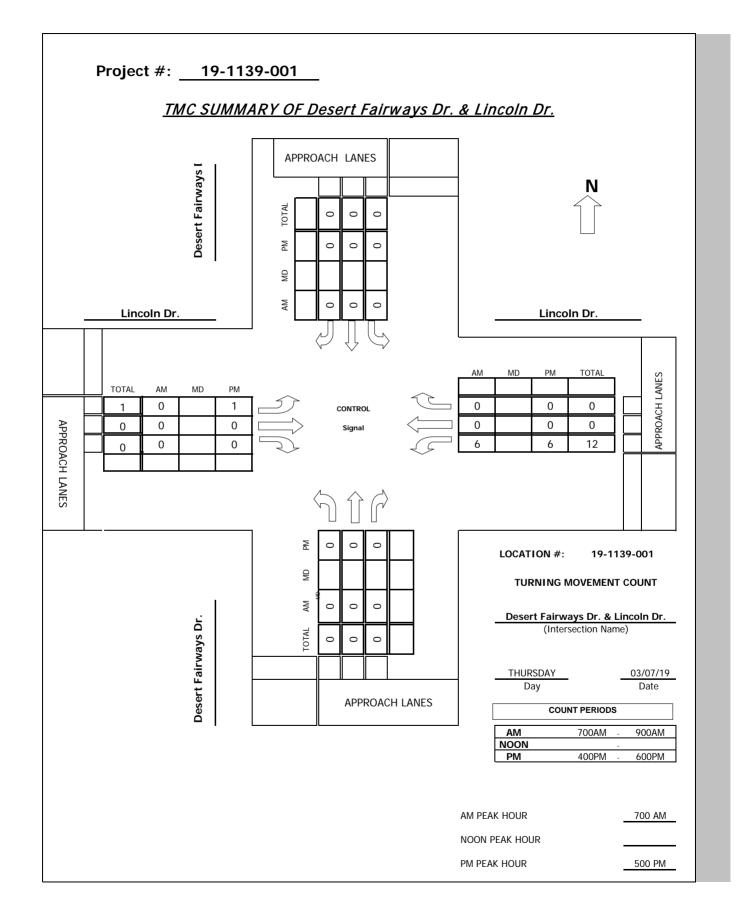
0.688 0.736 0.919 0.927 0.976 FACTOR:

CONTROL: Signal

COMMENT 1: 0

GPS: 33.531053, -111.966596









N-S STREET: Desert Fairways Dr. DATE: 03/07/19 LOCATION: Paradise Valley

E-W STREET: Lincoln Dr. DAY: THURSDAY PROJECT# 19-1139-001

	NC	ORTHBO	UND	SC	OUTHBO	UND	E	ASTBOL	JND	W	'ESTBOI	JND	
LANES:	NL 0	NT 1	NR 0	SL 1	ST 0.5	SR 0.5	EL 1	ET 2	ER 0	WL 1	WT 2	WR 0	TOTAL
6:00 AM 6:15 AM 6:30 AM		U-TURNS ONLY											
6:45 AM 7:00 AM 7:15 AM 7:30 AM 7:45 AM 8:00 AM 8:15 AM 8:30 AM 9:00 AM 9:15 AM 9:30 AM 9:45 AM 10:00 AM 10:15 AM	0 0 0 0 0 0	2 3 0 1 1 1 0 0	0 0 0 0 0 0	0 0 0 0 0 0	2 3 1 1 1								

TOTAL	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL	ĺ
Volumes	0	0	0	0	0	0	0	0	0	8	0	0	8	ĺ
Approach %	####	####	####	####	####	####	####	####	####	100.00	0.00	0.00		ĺ
App/Depart	0	/	0	0	/	8	0	/	0	8	/	0		ĺ

AM Peak Hr Begins at: 700 AM

PEAK

PEAK HR.

FACTOR: 0.000 0.000 0.500 0.500

CONTROL: Signal

COMMENT 1:

GPS: 33.531053, -111.966596

Intersection Turning Movement



N-S STREET: Desert Fairways Dr. DATE: 03/07/19 LOCATION: Paradise Valley

0

E-W STREET: Lincoln Dr. DAY: THURSDAY PROJECT# 19-1139-001

	NC	RTHBO	UND	SC	OUTHBO	UND	E	ASTBOL	JND	W	'ESTBOL	JND	
LANES:	NL 0	NT 1	NR 0	SL 1	ST 0.5	SR 0.5	EL 1	ET 2	ER 0	WL 1	WT 2	WR 0	TOTAL
1:00 PM													
1:15 PM					ι	J-TURN	IS ONL	Y					
1:30 PM													
1:45 PM													
2:00 PM													
2:15 PM													
2:30 PM													
2:45 PM													
3:00 PM													
3:15 PM													
3:30 PM													
3:45 PM													
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	
4:30 PM	0	0	0	0	0	0	1	0	0	1	0	0	2
4:45 PM	0	0	0	0	0	0	0	0	0	1	0	0	1
5:00 PM	0	0	0	0	0	0	0	0	0	2	0	0	2
5:15 PM	0	0	0	0	0	0	1	0	0	0	0	0	1
5:30 PM	0	0	0	0	0	0	0	0	0	2	0	0	2
5:45 PM	0	0	0	0	0	0	0	0	0	2	0	0	2
6:00 PM													
6:15 PM													
6:30 PM													
6:45 PM													
TOTAL	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL

TOTAL	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
Volumes	0	0	0	0	0	0	2	0	0	8	0	0	10
Approach %	####	####	####	####	####	####	100.00	0.00	0.00	100.00	0.00	0.00	
App/Depart	0	/	2	0	/	8	2	/	0	8	/	0	

PM Peak Hr Begins at: 500 PM

PEAK	
------	--

Volumes 0 0 0 0 0 0 1 0 0 6 0 7
Approach % #### #### #### #### #### 100.00 0.00 1.00.00 0.00

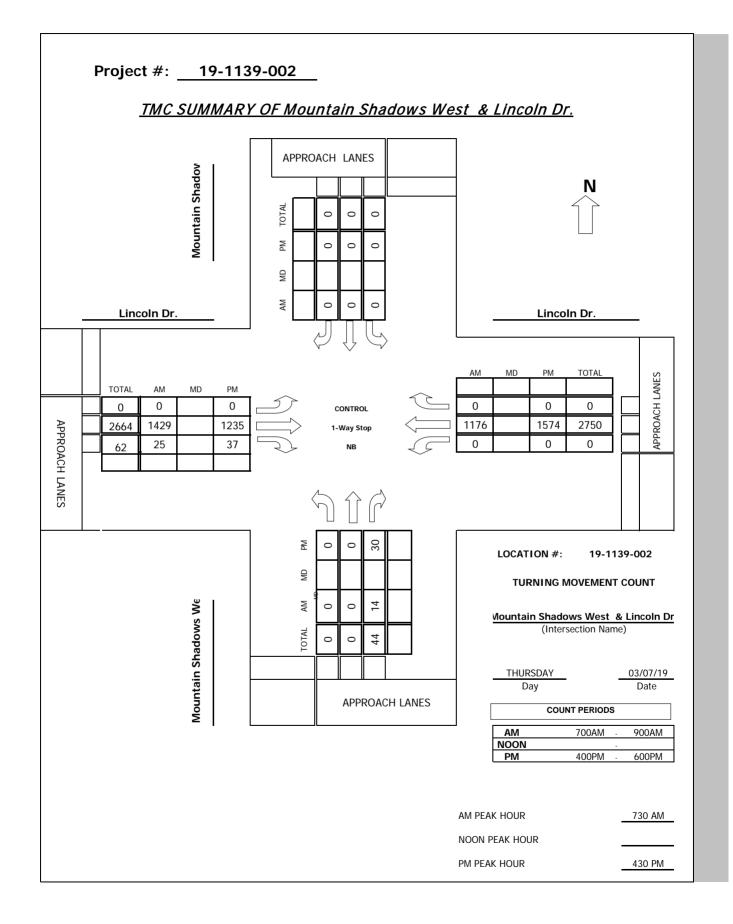
PEAK HR.

FACTOR: 0.000 0.000 0.250 0.750 0.875

CONTROL: Signal COMMENT 1: 0

GPS: 33.531053, -111.966596









N-S STREET: Mountain Shadows West DATE: 03/07/19 LOCATION: Paradise Valley

E-W STREET: Lincoln Dr. DAY: THURSDAY PROJECT# 19-1139-002

	NC	ORTHBO	UND	SC	UTHBO	UND	E	ASTBOU	IND	W	/ESTBOL	JND	
LANES:	NL 0	NT 0	NR 1	SL 0	ST 0	SR 0	EL 0	ET 2	ER 1	WL 0	WT 2	WR 0	TOTAL
6:00 AM 6:15 AM 6:30 AM 6:45 AM													
7:00 AM 7:15 AM 7:30 AM	0 0 0	0 0 0	1 5 3	0 0 0	0 0 0	0 0 0	0 0 0	308 338 346	7 7 6	0 0 0	228 262 299	0 0 0	544 612 654
7:45 AM 8:00 AM	0	0	3 4	0	0 0	0 0	0	391 370	5 10	0	295 272	0	694 656
8:15 AM 8:30 AM 8:45 AM	0 0 0	0 0 0	4 4 5	0 0 0	0 0 0	0 0 0	0 0 0	322 341 374	4 4 6	0 0 0	310 247 289	0 0 0	640 596 674
9:00 AM 9:15 AM 9:30 AM 9:45 AM													
10:00 AM 10:15 AM 10:30 AM 10:45 AM													
11:00 AM 11:15 AM 11:30 AM 11:45 AM													

TOTAL	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL	
Volumes	0	0	29	0	0	0	0	2790	49	0	2202	0	5070	
Approach %	0.00	0.00	100.00	####	####	####	0.00	98.27	1.73	0.00	100.00	0.00		
App/Depart	29	/	0	0	/	49	2839	/	2819	2202	/	2202		

AM Peak Hr Begins at: 730 AM

PEAK

Volumes 0 0 14 0 0 0 0 1429 25 0 1176 0 2644 Approach % 0.00 0.00 100.00 #### #### ### 0.00 98.28 1.72 0.00 100.00 0.00

PEAK HR.

FACTOR: 0.875 0.000 0.918 0.948 0.952

CONTROL: 1-Way Stop (NB)

COMMENT 1:

GPS: 33.531052, -111.963798

Intersection Turning Movement



N-S STREET: Mountain Shadows West

DATE: 03/07/19

LOCATION: Paradise Valley

E-W STREET: Lincoln Dr.

DAY: THURSDAY

PROJECT# 19-1139-002

	NO	RTHBO	UND	SC	UTHBO	UND	E	ASTBOU	ND	W	/ESTBOU	IND	
	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
LANES:	0	0	1	0	0	0	0	2	1	0	2	0	
1:00 PM													
1:15 PM													
1:30 PM													
1:45 PM													
2:00 PM													
2:15 PM													
2:30 PM													
2:45 PM													
3:00 PM													
3:15 PM													
3:30 PM													
3:45 PM	0	0	7	0	0	0	0	220	2	0	220	0	F0/
4:00 PM	0	0	7	0	0	0	0	238	3	0	338 387	0	586 685
4:15 PM 4:30 PM	0 0	0 0	3 5	0 0	0 0	0 0	0 0	289 292	6 11	0 0	367 379	0 0	687
4:30 PM	0	0	12	0	0	0	0	2 9 2 297	8	0	379 418	0	735
5:00 PM	0	0	9	0	0	0	0	321	9	0	393	0	732
5:15 PM	0	0	4	0	0	0	0	325	9	0	384	0	722
5:30 PM	0	0	5	0	0	0	0	327	11	0	298	0	641
5:45 PM	0	0	2	0	0	0	0	313	16	0	304	0	635
6:00 PM	Ŭ	Ü	_	Ü	Ŭ	Ü	J	0.0		J	00.	Ü	000
6:15 PM													
6:30 PM													
6:45 PM													
OTAL	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
olumes	0	0	47	0	0	0	0	2402	73	0	2901	0	5423
nnroach %	0.00	0.00	100.00	####	####	####	0.00	97 N5	2 95	0.00	100.00	0.00	

TOTAL	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
Volumes	0	0	47	0	0	0	0	2402	73	0	2901	0	5423
Approach %	0.00	0.00	100.00	####	####	####	0.00	97.05	2.95	0.00	100.00	0.00	
App/Depart	47	/	0	0	/	73	2475	/	2449	2901	/	2901	

PM Peak Hr Begins at: 430 PM

PEAK

Volumes 30 0 1235 0.00 0.00 100.00 #### #### #### 0.00 97.09 2.91 0.00 100.00

PEAK HR.

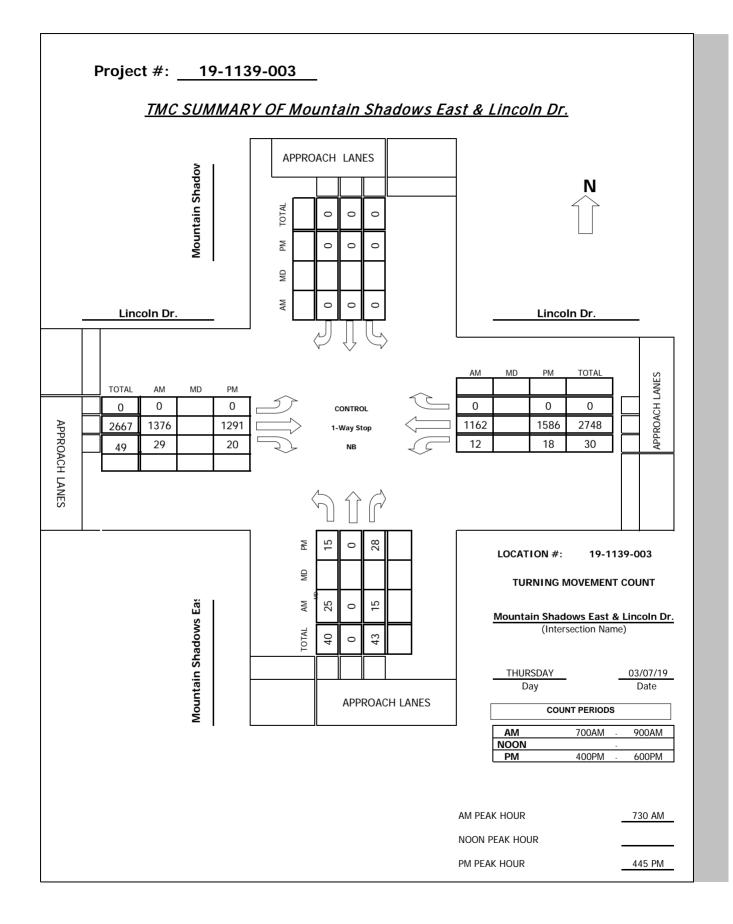
0.625 0.000 0.952 0.941 0.978 FACTOR:

1-Way Stop (NB) CONTROL:

COMMENT 1: 0

GPS: 33.531052, -111.963798









N-S STREET: Mountain Shadows East DATE: 03/07/19 LOCATION: Paradise Valley

E-W STREET: Lincoln Dr. DAY: THURSDAY PROJECT# 19-1139-003

	NC	NORTHBOUND		SC	UTHBO	UND	E	ASTBOU	IND	W	/ESTBOL	JND	
LANES:	NL O	NT 1	NR 0	SL 0	ST 0	SR 0	EL 0	ET 2	ER 1	WL 1	WT 2	WR 0	TOTAL
6:00 AM 6:15 AM 6:30 AM 6:45 AM													
7:00 AM 7:15 AM 7:30 AM	7 4 7	0 0 0	1 2 4	0 0 0	0 0 0	0 0 0	0 0 0	307 330 309	9 10 9	3 2 4	211 260 289	0 0 0	538 608 622
7:45 AM 8:00 AM 8:15 AM	3 10	0	3	0	0 0	0 0	0	381 359 327	8 7 5	4 3 1	288 275 310	0	687 657
8:30 AM 8:45 AM	5 3 2	0 0 0	5 2 5	0 0 0	0 0 0	0 0 0	0 0 0	334 357	11 6	1 4 1	246 281	0 0 0	653 600 652
9:00 AM 9:15 AM 9:30 AM 9:45 AM													
10:00 AM 10:15 AM 10:30 AM 10:45 AM													
11:00 AM 11:15 AM 11:30 AM 11:45 AM													

TOTAL	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
Volumes	41	0	25	0	0	0	0	2704	65	22	2160	0	5017
Approach %	62.12	0.00	37.88	####	####	####	0.00	97.65	2.35	1.01	98.99	0.00	
App/Depart	66	/	0	0	/	87	2769	/	2729	2182	/	2201	

AM Peak Hr Begins at: 730 AM

PEAK

Volumes 25 0 15 0 0 0 0 1376 29 12 1162 0 2619 Approach % 62.50 0.00 37.50 #### #### #### 0.00 97.94 2.06 1.02 98.98 0.00

PEAK HR.

FACTOR: 0.769 0.000 0.903 0.944 0.953

CONTROL: 1-Way Stop (NB)

COMMENT 1:

GPS: 33.531052, -111.963798

Intersection Turning Movement



N-S STREET: Mountain Shadows East

DATE: 03/07/19

LOCATION: Paradise Valley

E-W STREET: Lincoln Dr.

DAY: THURSDAY

PROJECT# 19-1139-003

	NO	RTHBO	JND	SO	UTHBO	UND	EA	ASTBOU	ND	W	ESTBOU	ND	
LANES:	NL 0	NT 1	NR 0	SL 0	ST 0	SR 0	EL 0	ET 2	ER 1	WL 1	WT 2	WR 0	TOTAL
1:00 PM													
1:15 PM													
1:30 PM													
1:45 PM													
2:00 PM													
2:15 PM													
2:30 PM													
2:45 PM													
3:00 PM													
3:15 PM													
3:30 PM													
3:45 PM													
4:00 PM	8	0	2	0	0	0	0	236	5	7	321	0	579
4:15 PM	5	0	4	0	0	0	0	287	2	5	373	0	676
4:30 PM	4	0	7	0	0	0	0	284	3	3	383	0	684
4:45 PM	5	0	4	0	0	0	0	304	6	2	425	0	746
5:00 PM	4	0	11	0	0	0	0	323	5	6	400	0	749
5:15 PM	2	0	9	0	0	0	0	335	6	3	369	0	724
5:30 PM	4	0	4	0	0	0	0	329	3	7	392	0	739
5:45 PM	1	0	4	0	0	0	0	313	8	14	401	0	741
6:00 PM													
6:15 PM													
6:30 PM													
6:45 PM													
OTAL	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTA
olumes	33	0	45	0	0	0	0	2411	38	47	3064	0	5638
proach %	42.31	0.00	57.69	####	####	####	0.00	98.45	1.55	1.51	98.49	0.00	
pp/Depart	78	/	0	0	/	85	2449	/	2456	3111	/	3097	

PEAK

Volumes 15 0 28 0 0 0 0 1291 20 18 1586 0 2958 Approach % 34.88 0.00 65.12 #### #### 0.00 98.47 1.53 1.12 98.88 0.00

PEAK HR.

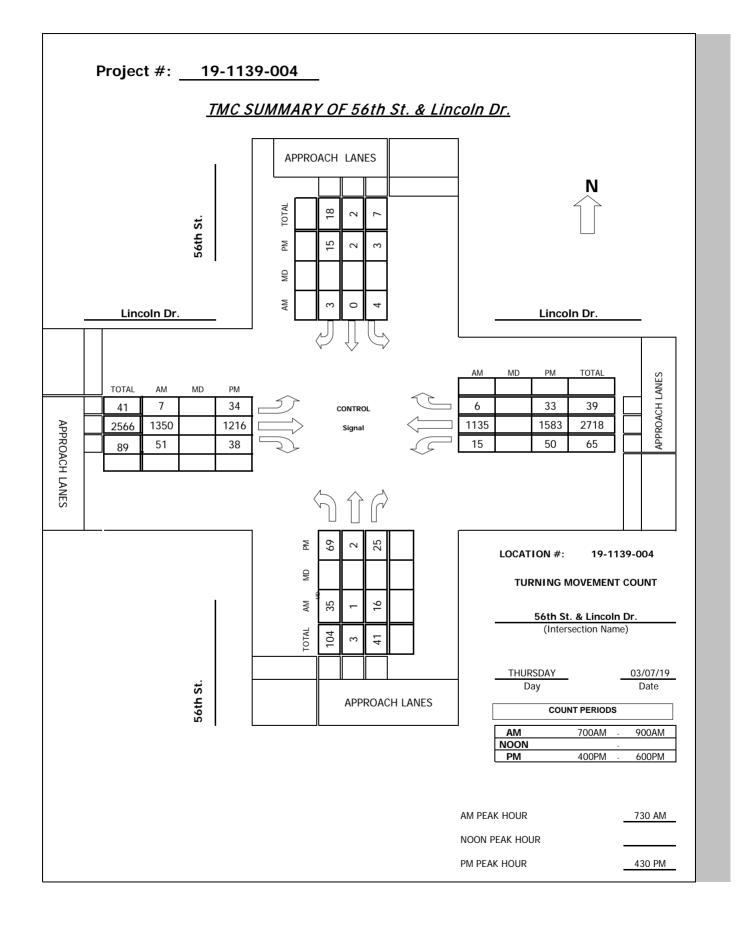
FACTOR: 0.717 0.000 0.961 0.939 0.987

CONTROL: 1-Way Stop (NB)

COMMENT 1: 0

GPS: 33.531052, -111.963798









N-S STREET: 56th St. DATE: 03/07/19 LOCATION: Paradise Valley

E-W STREET: Lincoln Dr. DAY: THURSDAY PROJECT# 19-1139-004

	NC	ORTHBO	UND	SC	OUTHBO	UND	E	ASTBOU	IND	W	/ESTBOL	JND	
LANES:	NL 0	NT 1	NR 0	SL 0	ST 1	SR 0	EL 1	ET 2	ER 0	WL 1	WT 2	WR 0	TOTAL
LANES.	O		O	U		O	'	2	O	'	2	O	
6:00 AM													
6:15 AM													
6:30 AM													
6:45 AM													
7:00 AM	8	0	3	0	0	1	3	287	16	3	209	0	530
7:15 AM	10	0	3	0	0	0	1	311	16	7	252	0	600
7:30 AM	6	0	6	1	0	1	0	303	13	5	286	2	623
7:45 AM	10	0	4	0	0	1	5	373	16	7	285	1	702
8:00 AM	8	0	5	1	0	1	1	345	12	1	265	1	640
8:15 AM	11	1	1	2	0	0	1	329	10	2	299	2	658
8:30 AM	11	0	3	1	0	1	0	322	13	7	238	0	596
8:45 AM	14	0	6	2	0	0	2	347	21	8	274	0	674
9:00 AM													
9:15 AM													
9:30 AM													
9:45 AM													
10:00 AM													
10:15 AM													
10:30 AM													
10:45 AM													
11:00 AM													
11:15 AM													
11:30 AM													
11:45 AM													

TOTAL	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
Volumes	78	1	31	7	0	5	13	2617	117	40	2108	6	5023
Approach %	70.91	0.91	28.18	58.33	0.00	41.67	0.47	95.27	4.26	1.86	97.86	0.28	
App/Depart	110	/	20	12	/	157	2747	/	2655	2154	/	2191	

AM Peak Hr Begins at: 730 AM

PEAK

Volumes 35 1 16 4 0 3 7 1350 51 15 1135 6 2623 Approach % 67.31 1.92 30.77 57.14 0.00 42.86 0.50 95.88 3.62 1.30 98.18 0.52

PEAK HR.

FACTOR: 0.929 0.875 0.893 0.954 0.934

CONTROL: Signal

COMMENT 1:

GPS: 33.531070, -111.960495

Intersection Turning Movement



N-S STREET: 56th St. DATE: 03/07/19 LOCATION: Paradise Valley

E-W STREET: Lincoln Dr. DAY: THURSDAY PROJECT# 19-1139-004

	NO	RTHBO	UND	SO	OUTHBO	UND	E	ASTBOU	ND	W	/ESTBOL	JND	_
LANES:	NL 0	NT 1	NR 0	SL 0	ST 1	SR 0	EL 1	ET 2	ER 0	WL 1	WT 2	WR 0	TOTAL
1:00 PM													
1:15 PM													
1:30 PM													
1:45 PM													
2:00 PM													
2:15 PM													
2:30 PM													
2:45 PM													
3:00 PM													
3:15 PM													
3:30 PM													
3:45 PM													
4:00 PM	11	0	8	3	0	1	4	225	9	1	338	2	602
4:15 PM	10	0	2	2	0	3	9	279	3	5	389	6	708
4:30 PM	18	0	5	1	0	1	6	280	6	10	385	8	720
4:45 PM	21	2	2	0	1	3	11	287	10	6	423	9	775
5:00 PM	15	0	6	2	0	6	8	316	14	10	403	10	790
5:15 PM	15	0	12	0	1	5	9	333	8	24	372	6	785
5:30 PM	9	0	13	2	0	1	3	325	6	13	309	5	686
5:45 PM	10	3	8	2	0	1	10	301	7	17	327	9	695
6:00 PM													
6:15 PM													
6:30 PM													
6:45 PM													
TOTAL	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
Volumes	109	5	56	12	2	21	60	2346	63	86	2946	55	5761

TOTAL	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
Volumes	109	5	56	12	2	21	60	2346	63	86	2946	55	5761
Approach %	64.12	2.94	32.94	34.29	5.71	60.00	2.43	95.02	2.55	2.79	95.43	1.78	
App/Depart	170	/	120	35	/	151	2469	/	2414	3087	/	3076	

PM Peak Hr Begins at: 430 PM

PEAK

Volumes 69 2 25 3 2 15 34 1216 38 50 1583 33 3070 Approach % 71.88 2.08 26.04 15.00 10.00 75.00 2.64 94.41 2.95 3.00 95.02 1.98

PEAK HR.

FACTOR: 0.889 0.625 0.920 0.951 0.972

CONTROL: Signal COMMENT 1: 0

GPS: 33.531070, -111.960495

APPENDIX C

EXISTING PEAK HOUR ANALYSIS



Mountain Shadows Resort Existing AM 1: Camelback Manor Dr/Desert Fairways Dr & Lincoln Dr Timings

	•	-	•	—	1	†	-	ţ	
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	
Lane Configurations	*	↑ Ъ	ች	↑ Ъ		4	7	1>	_
Traffic Volume (vph)	67	1264	5	1030	2	1	74	1	
Future Volume (vph)	67	1264	5	1030	2	1	74	1	
Turn Type	pm+pt	NA	Perm	NA	Perm	NA	Perm	NA	
Protected Phases	1	6		2		4		8	
Permitted Phases	6		2		4		8		
Detector Phase	1	6	2	2	4	4	8	8	
Switch Phase									
Minimum Initial (s)	4.0	15.0	15.0	15.0	7.0	7.0	7.0	7.0	
Minimum Split (s)	8.0	26.0	26.0	26.0	30.0	30.0	30.0	30.0	
Total Split (s)	8.0	35.0	27.0	27.0	30.0	30.0	30.0	30.0	
Total Split (%)	12.3%	53.8%	41.5%	41.5%	46.2%	46.2%	46.2%	46.2%	
Yellow Time (s)	3.0	4.5	4.5	4.5	3.0	3.0	3.0	3.0	
All-Red Time (s)	1.0	1.5	1.5	1.5	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	0.0	0.0	
Total Lost Time (s)	4.0	6.0	6.0	6.0		6.0	6.0	6.0	
Lead/Lag	Lead		Lag	Lag					
Lead-Lag Optimize?	Yes		Yes	Yes					
Recall Mode	None	C-Max	C-Max	C-Max	None	None	None	None	
Act Effct Green (s)	47.8	47.0	41.0	41.0		9.8	9.8	9.8	
Actuated g/C Ratio	0.74	0.72	0.63	0.63		0.15	0.15	0.15	
v/c Ratio	0.22	0.54	0.03	0.54		0.08	0.42	0.19	
Control Delay	5.3	7.0	8.0	10.8		15.1	30.5	9.2	
Queue Delay	0.0	0.0	0.0	0.0		0.0	0.0	0.0	
Total Delay	5.3	7.0	8.0	10.8		15.1	30.5	9.2	
LOS	Α	Α	Α	В		В	С	Α	
Approach Delay		6.9		10.8		15.1		22.5	
Approach LOS		Α		В		В		С	
Intersection Summary									
Cycle Length: 65									
Actuated Cycle Length: 65									
Offset: 0 (0%), Referenced	to phase 2	:WBTL ar	nd 6:EBT	L, Start of	Green				
Natural Cycle: 70	'								
Control Type: Actuated-Con	ordinated								
Maximum v/c Ratio: 0.54									
Intersection Signal Delay: 9	0.4			li	ntersectio	n LOS: A			
Intersection Capacity Utiliza	ation 73.3%	5		10	CU Level	of Service	e D		
Analysis Period (min) 15									

Splits and Phases: 1: Camelback Manor Dr/Desert Fairways Dr & Lincoln Dr



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Mountain Shadows Resort Existing AM

HCM 6th LOS

1: Camelback Manor Dr/Desert Fairways Dr & Lincoln Dr HCM 6th Signalized Intersection Summary

	۶	→	•	•	←	•	4	†	1	>	ļ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	↑ ↑		ሻ	↑ ↑			4		7	1>	
Traffic Volume (veh/h)	67	1264	4	5	1030	55	2	1	5	74	1	44
Future Volume (veh/h)	67	1264	4	5	1030	55	2	1	5	74	1	44
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1945	1870	1870	1870	1870
Adj Flow Rate, veh/h	74	1389	0	6	1144	28	5	2	2	88	1	28
Peak Hour Factor	0.91	0.91	0.91	0.90	0.90	0.90	0.40	0.40	0.40	0.84	0.84	0.84
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	346	2554	0	320	2169	53	151	59	35	249	5	149
Arrive On Green	0.05	0.72	0.00	0.20	0.20	0.20	0.10	0.10	0.10	0.10	0.10	0.10
Sat Flow, veh/h	1781	3647	0	389	3545	87	669	609	365	1412	55	1538
Grp Volume(v), veh/h	74	1389	0	6	573	599	9	0	0	88	0	29
Grp Sat Flow(s), veh/h/ln	1781	1777	0	389	1777	1855	1643	0	0	1412	0	1593
Q Serve(q_s), s	0.9	11.7	0.0	0.8	18.7	18.7	0.0	0.0	0.0	3.5	0.0	1.1
Cycle Q Clear(q_c), s	0.9	11.7	0.0	5.6	18.7	18.7	0.3	0.0	0.0	3.8	0.0	1.1
Prop In Lane	1.00		0.00	1.00		0.05	0.56		0.22	1.00		0.97
Lane Grp Cap(c), veh/h	346	2554	0	320	1087	1135	245	0	0	249	0	154
V/C Ratio(X)	0.21	0.54	0.00	0.02	0.53	0.53	0.04	0.00	0.00	0.35	0.00	0.19
Avail Cap(c_a), veh/h	375	2554	0	320	1087	1135	674	0	0	634	0	588
HCM Platoon Ratio	1.00	1.00	1.00	0.33	0.33	0.33	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	1.00	1.00	1.00	1.00	0.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	7.2	4.2	0.0	14.3	17.5	17.5	26.7	0.0	0.0	28.2	0.0	27.0
Incr Delay (d2), s/veh	0.1	0.8	0.0	0.1	1.8	1.8	0.1	0.0	0.0	0.9	0.0	0.6
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.2	2.3	0.0	0.1	9.1	9.5	0.1	0.0	0.0	1.3	0.0	0.4
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	7.3	5.1	0.0	14.4	19.4	19.3	26.7	0.0	0.0	29.1	0.0	27.6
LnGrp LOS	Α	Α	Α	В	В	В	С	Α	Α	С	Α	С
Approach Vol, veh/h		1463			1178			9			117	
Approach Delay, s/veh		5.2			19.3			26.7			28.7	
Approach LOS		Α			В			С			С	
Timer - Assigned Phs	1	2		4		6		8				
Phs Duration (G+Y+Rc), s	6.9	45.8		12.3		52.7		12.3				
Change Period (Y+Rc), s	4.0	6.0		6.0		6.0		6.0				
Max Green Setting (Gmax), s	4.0	21.0		24.0		29.0		24.0				
Max Q Clear Time (g_c+l1), s	2.9	20.7		2.3		13.7		5.8				
Green Ext Time (p_c), s	0.0	0.2		0.0		8.5		0.3				
Intersection Summary												
HCM 6th Ctrl Delay			12.3									

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Mountain Shadows	Resort
Existing AM	

2: Mountain Shadows West & Lincoln Dr HCM 6th TWSC

Intersection						
Int Delay, s/veh	0.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	44	7		44		7
Traffic Vol, veh/h	1329	23	0	1104	0	13
Future Vol, veh/h	1329	23	0	1104	0	13
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized		None	-	None	-	None
Storage Length	-	125	-	-	-	0
Veh in Median Storage	, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	95	95	88	88
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1445	25	0	1162	0	15

Major/Minor	Major1	Ma	ijor2	Mir	nor1	
Conflicting Flow All	0	0	-	-	-	723
Stage 1		-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy		-	-	-	-	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.32
Pot Cap-1 Maneuver	-	-	0	-	0	*540
Stage 1	-	-	0	-	0	-
Stage 2		-	0	-	0	-
Platoon blocked, %	-	-		-		1
Mov Cap-1 Maneuver		-	-	-	-	*540
Mov Cap-2 Maneuver		-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	EB	WB	NB
HCM Control Delay, s	0	0	11.9
HCM LOS			В

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBT
Capacity (veh/h)	540	-	-	-
HCM Lane V/C Ratio	0.027	-	-	-
HCM Control Delay (s)	11.9	-	-	-
HCM Lane LOS	В	-	-	-
HCM 95th %tile Q(veh)	0.1		-	-

ivotes			
Volume exceeds canacity	\$- Dolay ovenode 200e	Computation Not Defined	*· All major volume in platoon

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Mountain Shadows Resort Existing AM

3: Mountain Shadows East & Lincoln Dr ${\rm HCM}$ 6th TWSC

ntersection								
t Delay, s/veh	0.3							
Novement	EBT	EBR	WBL	WBT	NBL	NBR		
Lane Configurations	^	7	ሻ	44	W			
Fraffic Vol, veh/h	1315	27	11	1081	23	14		
uture Vol, veh/h	1315	27	11	1081	23	14		
Conflicting Peds, #/hr	0	0	0	0	0	0		
Sign Control	Free	Free	Free	Free	Stop	Stop		
RT Channelized	-	None	-	None	-	None		
Storage Length		135	80	-	0	-		
Veh in Median Storage	.# 0	-	-	0	1			
Grade. %	0			0	0			
Peak Hour Factor	90	90	94	94	77	77		
Heavy Vehicles, %	2	2	2	2	2	2		
Wymt Flow	1461	30	12	1150	30	18		
VIVIII. I IUW	1401	30	12	1130	30	10		
Anior/Minor N	Anior1		Major?		Minor1			
	Major1 0	0	Major2 1491	0	<u>Vinor1</u> 2060	731		
Conflicting Flow All		0		0				
Stage 1	-		-	-	1461	-		
Stage 2	-	-		-	599	-		
Critical Hdwy	-	-	4.14		6.84	6.94		
Critical Hdwy Stg 1	-	-	-	-	5.84	-		
Critical Hdwy Stg 2	-	-	-		5.84	-		
Follow-up Hdwy	-	-	2.22	-	3.52	3.32		
Pot Cap-1 Maneuver	-	-	*808		*87	*540		
Stage 1	-	-	-	-	*510	-		
Stage 2		-	-		*600	-		
Platoon blocked, %	-	-	1	-	1	1		
Mov Cap-1 Maneuver	-		*808	-	*86	*540		
Nov Cap-2 Maneuver	-	-	-	-	*286	-		
Stage 1	-		-	-	*502	-		
Stage 2	-		-	-	*600	-		
ŭ.								
Approach	EB		WB		NB			
HCM Control Delay, s	0		0.1		17			
HCM LOS	0		0.1		C			
200								
Minor Lane/Major Mvm	t	NBLn1	EBT	EBR	WBL	WBT		
	ı	348	EB1	EBR		WBI		
Capacity (veh/h)					* 808			
HCM Lane V/C Ratio		0.138	-	-	0.014	-		
HCM Control Delay (s)		17	-	-	9.5	-		
HCM Lane LOS		С	-	-	Α	-		
HCM 95th %tile Q(veh)		0.5	-		0	-		
lotes								
							outation Not Defined *:	All major volume in platoon

Mountain Shadows Resort Existing AM 4: 56th St/56th St & Lincoln Dr Timings

	۶	→	•	←	4	†	1	-	ļ
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT
Lane Configurations	ሻ	∱ Љ	ሻ	∱ Љ		ની	7		4
Traffic Volume (vph)	7	1275	14	1056	33	1	15	4	0
Future Volume (vph)	7	1275	14	1056	33	1	15	4	0
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	Perm	NA
Protected Phases		6		2		4			8
Permitted Phases	6		2		4		4	8	
Detector Phase	6	6	2	2	4	4	4	8	8
Switch Phase									
Minimum Initial (s)	15.0	15.0	15.0	15.0	7.0	7.0	7.0	7.0	7.0
Minimum Split (s)	31.5	31.5	31.5	31.5	13.0	13.0	13.0	29.0	29.0
Total Split (s)	36.0	36.0	36.0	36.0	29.0	29.0	29.0	29.0	29.0
Total Split (%)	55.4%	55.4%	55.4%	55.4%	44.6%	44.6%	44.6%	44.6%	44.6%
Yellow Time (s)	4.5	4.5	4.5	4.5	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.0	2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	0.0		0.0
Total Lost Time (s)	6.5	6.5	6.5	6.5		6.0	6.0		6.0
Lead/Lag									
Lead-Lag Optimize?									
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None	None	None	None
Act Effct Green (s)	52.6	52.6	52.6	52.6		7.7	7.7		7.7
Actuated g/C Ratio	0.81	0.81	0.81	0.81		0.12	0.12		0.12
v/c Ratio	0.02	0.52	0.07	0.39		0.20	0.07		0.04
Control Delay	2.1	6.0	4.6	4.0		28.4	0.5		0.3
Queue Delay	0.0	0.0	0.0	0.0		0.0	0.0		0.0
Total Delay	2.1	6.0	4.6	4.0		28.4	0.5		0.3
LOS	Α	Α	Α	Α		С	Α		Α
Approach Delay		5.9		4.0		19.8			0.3
Approach LOS		Α		Α		В			Α
Intersection Summary									
Cycle Length: 65									
Actuated Cycle Length: 65									
Offset: 0 (0%), Referenced	to phase 2	:WBTL ar	nd 6:EBT	L, Start of	f Green				
Natural Cycle: 65									
Control Type: Actuated-Co	ordinated								
Maximum v/c Ratio: 0.52									
Intersection Signal Delay: 5					ntersectio				
Intersection Capacity Utiliza	ation 63.8%)		10	CU Level	ot Service	e B		
Analysis Period (min) 15									
Splits and Phases: 4: 56	th St/56th	St & Linco	oln Dr						
▼ Ø2 (R)						★	4		



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Mountain Shadows Resort Existing AM

HCM 6th LOS

4: 56th St/56th St & Lincoln Dr HCM 6th Signalized Intersection Summary

	۶	→	•	•	←	•	4	†	1	\	ļ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	↑ ↑		ሻ	↑ ↑			ની	7		4	
Traffic Volume (veh/h)	7	1275	47	14	1056	6	33	1	15	4	0	3
Future Volume (veh/h)	7	1275	47	14	1056	6	33	1	15	4	0	3
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	8	1433	25	15	1112	2	35	1	7	5	0	0
Peak Hour Factor	0.89	0.89	0.89	0.95	0.95	0.95	0.93	0.93	0.93	0.88	0.88	0.88
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	431	2663	46	320	2712	5	202	4	99	171	0	0
Arrive On Green	0.75	0.75	0.75	0.75	0.75	0.75	0.06	0.06	0.06	0.06	0.00	0.00
Sat Flow, veh/h	506	3574	62	364	3639	7	1489	69	1585	973	0	0
Grp Volume(v), veh/h	8	712	746	15	543	571	36	0	7	5	0	0
Grp Sat Flow(s),veh/h/ln	506	1777	1859	364	1777	1869	1558	0	1585	973	0	0
Q Serve(g_s), s	0.4	11.1	11.1	1.2	7.3	7.3	0.0	0.0	0.3	0.2	0.0	0.0
Cycle Q Clear(g_c), s	7.7	11.1	11.1	12.3	7.3	7.3	1.3	0.0	0.3	1.5	0.0	0.0
Prop In Lane	1.00		0.03	1.00		0.00	0.97		1.00	1.00		0.00
Lane Grp Cap(c), veh/h	431	1324	1386	320	1324	1393	206	0	99	172	0	0
V/C Ratio(X)	0.02	0.54	0.54	0.05	0.41	0.41	0.17	0.00	0.07	0.03	0.00	0.00
Avail Cap(c_a), veh/h	431	1324	1386	320	1324	1393	622	0	561	582	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	4.4	3.5	3.5	6.1	3.0	3.0	29.2	0.0	28.7	29.9	0.0	0.0
Incr Delay (d2), s/veh	0.1	1.6	1.5	0.3	0.9	0.9	0.4	0.0	0.3	0.1	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.0	2.1	2.1	0.1	1.3	1.4	0.5	0.0	0.1	0.1	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	4.5	5.1	5.0	6.4	4.0	3.9	29.6	0.0	29.0	29.9	0.0	0.0
LnGrp LOS	Α	Α	Α	Α	Α	Α	С	Α	С	С	Α	<u>A</u>
Approach Vol, veh/h		1466			1129			43			5	
Approach Delay, s/veh		5.1			4.0			29.5			29.9	
Approach LOS		Α			Α			С			С	
Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		54.9		10.1		54.9		10.1				
Change Period (Y+Rc), s		6.5		6.0		6.5		6.0				
Max Green Setting (Gmax), s		29.5		23.0		29.5		23.0				
Max Q Clear Time (g_c+I1), s		14.3		3.3		13.1		3.5				
Green Ext Time (p_c), s		6.3		0.1		8.9		0.0				
Intersection Summary												
HCM 6th Ctrl Delay			5.0									

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Mountain Shadows Resort Existing PM

1: Camelback Manor Dr/Desert Fairways Dr & Lincoln Dr

	•	-	1	—	1	Ť	-	¥	
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	
Lane Configurations	*	↑ ↑	ች	↑ Ъ		43-	7	1>	
Traffic Volume (vph)	62	1073	5	1265	4	2	112	2	
Future Volume (vph)	62	1073	5	1265	4	2	112	2	
Turn Type	pm+pt	NA	Perm	NA	Perm	NA	Perm	NA	
Protected Phases	1	6		2		4		8	
Permitted Phases	6		2		4		8		
Detector Phase	1	6	2	2	4	4	8	8	
Switch Phase									
Minimum Initial (s)	4.0	15.0	15.0	15.0	7.0	7.0	7.0	7.0	
Minimum Split (s)	8.0	26.0	26.0	26.0	30.0	30.0	30.0	30.0	
Total Split (s)	8.0	35.0	27.0	27.0	30.0	30.0	30.0	30.0	
Total Split (%)	12.3%	53.8%	41.5%	41.5%	46.2%	46.2%	46.2%	46.2%	
Yellow Time (s)	3.0	4.5	4.5	4.5	3.0	3.0	3.0	3.0	
All-Red Time (s)	1.0	1.5	1.5	1.5	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	0.0	0.0	
Total Lost Time (s)	4.0	6.0	6.0	6.0		6.0	6.0	6.0	
Lead/Lag	Lead		Lag	Lag					
Lead-Lag Optimize?	Yes		Yes	Yes					
Recall Mode	None	C-Max	C-Max	C-Max	None	None	None	None	
Act Effct Green (s)	42.5	40.5	34.4	34.4		12.5	12.5	12.5	
Actuated g/C Ratio	0.65	0.62	0.53	0.53		0.19	0.19	0.19	
v/c Ratio	0.25	0.53	0.02	0.83		0.05	0.57	0.29	
Control Delay	7.6	8.8	21.8	28.9		14.9	31.1	6.8	
Queue Delay	0.0	0.0	0.0	0.0		0.0	0.0	0.0	
Total Delay	7.6	8.8	21.8	28.9		14.9	31.1	6.8	
LOS	Α	Α	С	С		В	С	Α	
Approach Delay		8.8		28.9		14.9		20.5	
Approach LOS		Α		С		В		С	
Intersection Summary									
Cycle Length: 65									
Actuated Cycle Length: 65									
Offset: 49 (75%), Reference	d to phase	e 2:WBTL	and 6:Ef	BTL, Start	of Green				
Natural Cycle: 80									
Control Type: Actuated-Coo	rdinated								

Maximum v/c Ratio: 0.83

Intersection Signal Delay: 20.0
Intersection Capacity Utilization 70.1%
Analysis Period (min) 15

Intersection LOS: B
ICU Level of Service C

Splits and Phases: 1: Camelback Manor Dr/Desert Fairways Dr & Lincoln Dr



11/15/2019 Synchro 10 Report CivTech Page 1 Mountain Shadows Resort Existing PM

1: Camelback Manor Dr/Desert Fairways Dr & Lincoln Dr HCM 6th Signalized Intersection Summary

	۶	-	\rightarrow	•	←	•	4	†	<i>></i>	-	ļ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	- 1	↑ Ъ		ሻ	↑ Ъ			4		ሻ	1>	
Traffic Volume (veh/h)	62	1073	3	5	1265	168	4	2	5	112	2	84
Future Volume (veh/h)	62	1073	3	5	1265	168	4	2	5	112	2	84
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1945	1870	1870	1870	1870
Adj Flow Rate, veh/h	67	1166	0	5	1360	95	6	3	3	151	3	73
Peak Hour Factor	0.92	0.92	0.92	0.93	0.93	0.93	0.69	0.69	0.69	0.74	0.74	0.74
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	275	2429	0	365	1950	136	152	75	48	313	8	202
Arrive On Green	0.04	0.68	0.00	0.39	0.39	0.39	0.13	0.13	0.13	0.13	0.13	0.13
Sat Flow, veh/h	1781	3647	0	481	3370	235	521	570	364	1410	63	1532
Grp Volume(v), veh/h	67	1166	0	5	715	740	12	0	0	151	0	76
Grp Sat Flow(s),veh/h/ln	1781	1777	0	481	1777	1828	1454	0	0	1410	0	1595
Q Serve(g_s), s	0.9	10.0	0.0	0.4	21.9	22.1	0.0	0.0	0.0	3.1	0.0	2.8
Cycle Q Clear(g_c), s	0.9	10.0	0.0	3.7	21.9	22.1	2.8	0.0	0.0	5.9	0.0	2.8
Prop In Lane	1.00		0.00	1.00		0.13	0.50		0.25	1.00		0.96
Lane Grp Cap(c), veh/h	275	2429	0	365	1028	1058	275	0	0	313	0	210
V/C Ratio(X)	0.24	0.48	0.00	0.01	0.70	0.70	0.04	0.00	0.00	0.48	0.00	0.36
Avail Cap(c_a), veh/h	308	2429	0	365	1028	1058	644	0	0	648	0	589
HCM Platoon Ratio	1.00	1.00	1.00	0.67	0.67	0.67	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	1.00	1.00	1.00	1.00	0.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	9.5	4.8	0.0	10.6	15.1	15.1	24.7	0.0	0.0	26.9	0.0	25.7
Incr Delay (d2), s/veh	0.2	0.7	0.0	0.1	3.9	3.8	0.1	0.0	0.0	1.2	0.0	1.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.3	2.3	0.0	0.0	9.8	10.1	0.2	0.0	0.0	2.3	0.0	1.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	9.7	5.5	0.0	10.6	19.0	19.0	24.7	0.0	0.0	28.0	0.0	26.8
LnGrp LOS	Α	Α	Α	В	В	В	С	Α	Α	С	Α	С
Approach Vol, veh/h		1233			1460			12			227	
Approach Delay, s/veh		5.8			19.0			24.7			27.6	
Approach LOS		Α			В			С			С	
Timer - Assigned Phs	1	2		4		6		8				
Phs Duration (G+Y+Rc), s	6.8	43.6		14.6		50.4		14.6				
Change Period (Y+Rc), s	4.0	6.0		6.0		6.0		6.0				
Max Green Setting (Gmax), s	4.0	21.0		24.0		29.0		24.0				
Max Q Clear Time (q_c+l1), s	2.9	24.1		4.8		12.0		7.9				
Green Ext Time (p_c), s	0.0	0.0		0.0		7.5		0.8				
Intersection Summary												
HCM 6th Ctrl Delay			14.1									
HCM 6th LOS			В									

11/15/2019 Synchro 10 Report CivTech Page 2 Mvmt Flow

Intersection						
Int Delay, s/veh	0.2					
Movement	FBT	FBR	WBL	WBT	NBL	NBR
Movement		EDI	WDL		INDL	NDI
Lane Configurations	- 44	7		- 44		7
Traffic Vol, veh/h	1192	34	0	1489	0	28
Future Vol, veh/h	1192	34	0	1489	0	28
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	125		-	-	0
Veh in Median Storage,	# 0	-		0	0	-
Grade, %	0	-		0	0	-
Peak Hour Factor	95	95	94	94	63	63
Heavy Vehicles, %	2	2	2	2	2	2

Major/Minor I	Major1	Ma	ijor2	Mir	nor1	
Conflicting Flow All	0	0	-	-	-	628
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.32
Pot Cap-1 Maneuver	-	-	0	-	0	*588
Stage 1	-	-	0	-	0	-
Stage 2	-	-	0	-	0	
Platoon blocked, %	-	-		-		1
Mov Cap-1 Maneuver	-	-	-	-	-	*588
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

1255 36 0 1584 0 44

EB	WB	NB
0	0	11.6
		В
	EB 0	EB WB 0

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBT
Capacity (veh/h)	588	-	-	-
HCM Lane V/C Ratio	0.076	-	-	-
HCM Control Delay (s)	11.6	-	-	-
HCM Lane LOS	В	-	-	-
HCM 95th %tile Q(veh)	0.2	-	-	-

NOICS			
~: Volume exceeds capacity	\$: Delay exceeds 300s	+: Computation Not Defined	*: All major volume in platoon

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nt Delay, s/veh	0.1						
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
ane Configurations	<u>₽₽1</u>	EDR	WDL	↑ ↑	INDL.	NDK	
Traffic Vol. veh/h	TT	19	17	TT	14	26	
Future Vol. veh/h	1201	19	17	1475	14	26	
Conflicting Peds, #/hr	0	19	0	14/5	0	20	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	riee -	None	riee -	None	Stop -	None	
Storage Length		135	80	None -	0	None -	
siorage Lengin /eh in Median Storage		130	- 80	0	1		
Grade, % Peak Hour Factor	96	96	94	94	72	72	
	96	96	94	94	2	2	
Heavy Vehicles, %							
/lvmt Flow	1251	20	18	1569	19	36	
	Major1	1	Major2	1	Minor1		
Conflicting Flow All	0	0	1271	0	2072	626	
Stage 1	-		-	-	1251	-	
Stage 2	-	-	-	-	821	-	
Critical Hdwy	-	-	4.14	-	6.84	6.94	
Critical Hdwy Stg 1	-	-	-	-	5.84	-	
Critical Hdwy Stg 2	-	-	-	-	5.84		
ollow-up Hdwy	-	-	2.22	-	3.52	3.32	
Pot Cap-1 Maneuver	-	-	*880	-	-	*588	
Stage 1	-	-	-	-	*555	-	
Stage 2	-	-	-	-	*464	-	
Platoon blocked, %	-	-	1	-	2	1	
Nov Cap-1 Maneuver	-	-	*880	-		*588	
Nov Cap-2 Maneuver	-	-	-	-	*223	-	
Stage 1			-	-	*544	-	
Stage 2	-	-	-	-	*464		
, and the second							
Approach	EB		WB		NB		
HCM Control Delay, s	0		0.1		IND		
HCM LOS	0		0.1				
IOW EUG							
Minor Lane/Major Mvn	nt	NBLn1	EBT	EBR	WBL	WBT	
Capacity (veh/h)			-	-	* 880	-	
HCM Lane V/C Ratio		-	-	-	0.021	-	
HCM Control Delay (s))		-	-	9.2		
HCM Lane LOS		-	-	-	Α	-	
HCM 95th %tile Q(veh)		-	-	0.1	-	
Votes							

Mountain Shadows Resort

Existing PM

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Mountain Shadows Resort Existing PM

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4: 56th St/56th St & Lincoln Dr Timings

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Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT			
Lane Configurations	*	↑ ↑	ች	↑ ↑		ર્ન	7		4			
Traffic Volume (vph)	32	1160	47	1414	64	2	23	3	2			
Future Volume (vph)	32	1160	47	1414	64	2	23	3	2			
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	Perm	NA			
Protected Phases		6		2		4			8			
Permitted Phases	6		2		4		4	8				
Detector Phase	6	6	2	2	4	4	4	8	8			
Switch Phase												
Minimum Initial (s)	15.0	15.0	15.0	15.0	7.0	7.0	7.0	7.0	7.0			
Minimum Split (s)	31.5	31.5	31.5	31.5	13.0	13.0	13.0	29.0	29.0			
Total Split (s)	35.0	35.0	35.0	35.0	30.0	30.0	30.0	30.0	30.0			
Total Split (%)	53.8%	53.8%	53.8%	53.8%	46.2%	46.2%	46.2%	46.2%	46.2%			
Yellow Time (s)	4.5	4.5	4.5	4.5	3.0	3.0	3.0	3.0	3.0			
All-Red Time (s)	2.0	2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0			
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	0.0		0.0			
Total Lost Time (s)	6.5	6.5	6.5	6.5		6.0	6.0		6.0			
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None	None	None	None			
Act Effct Green (s)	47.2	47.2	47.2	47.2		9.2	9.2		9.2			
Actuated g/C Ratio	0.73	0.73	0.73	0.73		0.14	0.14		0.14			
v/c Ratio	0.19	0.51	0.20	0.59		0.40	0.09		0.13			
Control Delay	5.4	3.4	7.5	7.4		31.0	2.9		18.9			
Queue Delay	0.0	0.0	0.0	0.0		0.0	0.0		0.0			
Total Delay	5.4	3.4	7.5	7.4		31.0	2.9		18.9			
LOS	Α	Α	Α	Α		С	Α		В			
Approach Delay		3.5		7.4		23.7			18.9			
Approach LOS		Α		Α		С			В			
Intersection Summary												
Cycle Length: 65												
Actuated Cycle Length: 65												
Offset: 29 (45%), Reference	d to phase	2:WBTL	and 6:Ef	BTL, Start	of Green							
Natural Cycle: 65	•											
Control Type: Actuated-Coor	rdinated											
Maximum v/c Ratio: 0.59												
Intersection Signal Delay: 6.3				li	ntersectio	n LOS: A						
Intersection Capacity Utilization 60.8% ICU Level of Service B												
Analysis Period (min) 15												
Splits and Phases: 4: 56th	St/56th	St & Linco	oln Dr									
4-	1 3030011	JI OX LITIU	JIII DI			-d.						
▼ Ø2 (R)						1 Ø4						

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Mountain Shadows Resort Existing PM

HCM 6th LOS

4: 56th St/56th St & Lincoln Dr HCM 6th Signalized Intersection Summary

	۶	→	•	•	←	•	4	†	1	\	ļ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	† }		ň	† 1>			ની	7		44	
Traffic Volume (veh/h)	32	1160	35	47	1414	31	64	2	23	3	2	14
Future Volume (veh/h)	32	1160	35	47	1414	31	64	2	23	3	2	14
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	35	1261	27	49	1488	22	72	2	15	5	3	14
Peak Hour Factor	0.92	0.92	0.92	0.95	0.95	0.95	0.89	0.89	0.89	0.63	0.63	0.63
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	288	2542	54	417	2561	38	240	5	148	85	38	96
Arrive On Green	1.00	1.00	1.00	0.71	0.71	0.71	0.09	0.09	0.09	0.09	0.09	0.09
Sat Flow, veh/h	347	3557	76	429	3585	53	1406	56	1585	179	411	1033
Grp Volume(v), veh/h	35	630	658	49	737	773	74	0	15	22	0	0
Grp Sat Flow(s), veh/h/ln	347	1777	1857	429	1777	1861	1462	0	1585	1623	0	0
Q Serve(g_s), s	2.2	0.0	0.0	2.4	13.2	13.2	2.1	0.0	0.6	0.0	0.0	0.0
Cycle Q Clear(g_c), s	15.4	0.0	0.0	2.4	13.2	13.2	3.0	0.0	0.6	0.8	0.0	0.0
Prop In Lane	1.00		0.04	1.00		0.03	0.97		1.00	0.23		0.64
Lane Grp Cap(c), veh/h	288	1270	1327	417	1270	1330	245	0	148	219	0	0
V/C Ratio(X)	0.12	0.50	0.50	0.12	0.58	0.58	0.30	0.00	0.10	0.10	0.00	0.00
Avail Cap(c_a), veh/h	288	1270	1327	417	1270	1330	633	0	585	652	0	0
HCM Platoon Ratio	2.00	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	2.2	0.0	0.0	3.0	4.5	4.5	28.0	0.0	27.0	27.1	0.0	0.0
Incr Delay (d2), s/veh	0.9	1.4	1.3	0.6	1.9	1.9	0.7	0.0	0.3	0.2	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.1	0.5	0.5	0.2	3.0	3.1	1.1	0.0	0.2	0.3	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	3.0	1.4	1.3	3.6	6.5	6.4	28.7	0.0	27.3	27.3	0.0	0.0
LnGrp LOS	Α	Α	Α	Α	Α	Α	С	Α	С	С	Α	Α
Approach Vol, veh/h		1323			1559			89			22	
Approach Delay, s/veh		1.4			6.3			28.5			27.3	
Approach LOS		Α			Α			С			С	
Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		52.9		12.1		52.9		12.1				
Change Period (Y+Rc), s		6.5		6.0		6.5		6.0				
Max Green Setting (Gmax), s		28.5		24.0		28.5		24.0				
Max Q Clear Time (g_c+l1), s		15.2		5.0		17.4		2.8				
Green Ext Time (p_c), s		8.4		0.3		6.3		0.1				
Intersection Summary												
HCM 6th Ctrl Delay			5.0									

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APPENDIX D

TRIP GENERATION



CIVTECH INC.

Proposed March 2019

This form facilitates trip generation estimation using data within the Institute of Transportation Engineer's (ITE) *Trip Generation Manual*, 10th Edition and methodology described within ITE *Generation Handbook*, 3rd Edition. These references will be referred to as *Manual and Handbook*, respectively. The *Manual* contains data collected by various transportation professiona wide range of different land uses, with each land use category represented by a land use code (LUC). Average rates and equations have been established that correlate the relationship by an independent variable that describes the development size and generated trips for each categorized LUC in various settings and time periods. The *Handbook* indicates an established methodology for how to use data contained within the Manual when to use the fitted curve instead of the average rate and when to adjustments to the volume of trips are appropriate and by do so. The methodology steps are represented visually in boxes in Figure 3.1. This worksheet applies calculations for each box if applicable.

Box 1 - Define Study Site Land Use Type & Site Characteristics, Box 2 - Define Site Context and Box 3 - Define Analysis Objectives Types of Trips & Time Period

The analyst is to pick an appropriate LUC(s) based on the subject's zoning/land use(s)/future land use(s). The size of the land use(s) is described in reference to an independent variable(s specific to (each) the land use (example: 1,000 square feet of building area is relatively common).

Context assessment is to "simply determine whether the study sites is in a multimodal setting" and "could have persons accessing the site by walking, bicycling, or riding transit." This assessment is used in Box 4. The *Manual* separates data into 4 setting categories - **Rural, General Urban/Suburban**, **Dense Multi-Urban Use** and **Center City Core**. This worksheet us the following abbreviations, respectively: **R**, **G**, **D**, and **C**. The *Manual* does not have data for all settings of all land use codes. The "General Urban/Suburban" setting is used by default. This tool will focus on vehicular trips for a 24-hour period on a typical weekday as well as its AM peak hour and PM peak hour. Other time period(s) may be of interest.

Land Use Types and Size

Proposed Use	Amount Units	ITE LUC	ITE Land Use Name
Quality Restaurant	5.000 1,000 square feet	931	Quality Restaurant

Equation Type: Equation Used [Equated Rate] (Type Abbreviations: Weighted Average Rate ("WA"), Fitted Curve Type: Equation Used [Equated Rate]

Proposed Use	ADT	AM Peak Hour	PM Peak Hour	(not used)
Quality Restaurant	WA: T=X*83.84 [83.84]	WA: T=X*0.73 [0.73]	WA: T=X*7.8 [7.80]	

Box 5/Box 9 - Estimate Baseline Trips/Estimate Vehicular Trips (Apply Equations and in/out Distributions)

Baseline Vehicular Trips

-		Α	DT		AM Peak Hour				PM Peak Hour				(not used)
Proposed Use	% In	% In In Out Total			% In	% In In Out Total			% In In Out			Total	
Quality Restaurant	50%	210	210	420	80%	3	1	4	67%	26	13	39	
Totals		210 210 420				3	1	4		26	13	39	



APPENDIX E

TRIP DISTRIBUTION

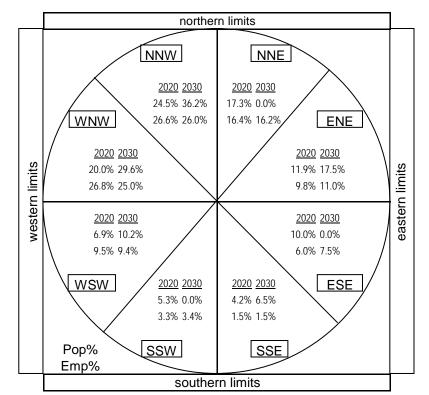


Trip Distribution - Summaries

		202	0			2	2030	
Quadrant	Population	Percent	Employment	Percent	Population	Percent	Employment	Percent
North Northwest	225,802	24.5%	128,618	26.6%	225,802	36.2%	138,038	26.0%
North Northeast	159,900	17.3%	79,130	16.4%	-	0.0%	86,232	16.2%
North	385,702	41.8%	207,748	43.0%	225,802	36.2%	224,270	42.2%
East Northeast	109,464	11.9%	47,207	9.8%	109,464	17.5%	58,581	11.0%
East Southeast	92,590	10.0%	29,090	6.0%	-	0.0%	40,006	7.5%
East	202,053	21.9%	76,297	15.8%	109,464	17.5%	98,587	18.5%
South Southeast	38,611	4.2%	7,389	1.5%	40,655	6.5%	7,782	1.5%
South Southwest	48,469	5.3%	16,139	3.3%	-	0.0%	18,347	3.4%
South	87,080	9.5%	23,528	4.8%	40,655	6.5%	26,129	4.9%
West Southwest	63,344	6.9%	45,971	9.5%	63,424	10.2%	50,054	9.4%
West Northwest	184,638	20.0%	129,429	26.8%	184,638	29.6%	132,868	25.0%
West	247,982	26.9%	175,399	36.3%	248,062	39.8%	182,922	34.4%
Totals	922,817	100.1%	482,972	99.9%	623,983	100.0%	531,907	100.0%

Radii

Population: 10-mile Radius



Trip Distribution - Pop from North

10-mile Radius													
RAZ	MPA	2020	2030	% of	2020	2030	RAZ	MPA	2020	2030	% of	2020	2030
	1411 7 1	Population	Population	TAZ	Adjusted	Adjusted		1411 7 1	Population	Population	TAZ	Adjusted	Adjusted
NNW						NNE							
280		52,543	52,543	50%	26,272	26,272	291	ME	56,729		5%	2,836	-
290		90,250	90,250	70%	63,175	63,175	298	ME	56,065		80%	44,852	-
297	TE	52,336	52,336	5%	2,617	2,617	299	ME	41,300		80%	33,040	-
298		56,065	56,065	20%	11,213	11,213	311	GI	79,589		40%	31,836	-
309		52,159	52,159	90%	46,943	46,943	312	GI	36,760		75%	27,570	-
310		54,854	54,854	20%	10,971	10,971	318	GI	49,414		40%	19,766	-
311	GI	79,589	79,589	60%	47,753	47,753		-	-			-	-
317	CH	34,879	34,879	20%	6,976	6,976		-	-			-	-
318	GI	49,414	49,414	20%	9,883	9,883		-	-			-	-
	-	-	-		-	-		-	-			-	-
	-	-	-		-	-		-	-			-	-
	-	-	-		-	-		-	-	-		-	-
	-	-	-		-	-		-	-	-		-	-
	-	-	-		-	-		-	-	-		-	-
	-	-	-		-	-		-	-	-		-	-
	-	-	-		-	-		-	-	-		-	-
	-	-	-		-	-		-	-	-		-	-
	-	-	-		-	-		-	-	-		-	-
	-	-	-		-	-		-	-	-		-	-
	-	-	-		-	-		-	-	-		-	-
	-	-	-		-	-		-	-	-		-	-
	-	-	-		-	-		-	-	-		-	-
	-	-	-		-	-		-	-	-		-	-
	-	-	-		-	-		-	-	-		-	-
	-	-	-		-	-		-	-	-		-	-

From North 225,802 225,802 159,900 - 385,702 225,802

Trip Distribution - Pop from East

202,053

109,464

1	10-mile Radius													
F	RAZ	MPA	2020	2030	% of	2020	2030	RAZ	MPA	2020	2030	% of	2020	2030
			Population	Population	TAZ	Adjusted	Adjusted			Population	Population	TAZ	Adjusted	Adjusted
- 1	312	GI	36,760	36,760	25%	9,190	9,190	ESE 318	GI	49,414		10%	4,941	
	318	GI	49,414	49,414	20%			319	GI	80,978		40%		-
	319	GI	80,978	80,978	60%	9,883 48,587	9,883 48,587	329	GI	39,078		30%		-
	320	ME	2,129	2,129	00%	2,129	46,367 2,129	339	QC	55,529		75%	41,647	-
	321	ME	24,334	24,334		24,334	2,129	129	PC	18,869		10%	1,887	-
	322	ME	38,353	38,353	40%	15,341	15,341	129	-	10,009		10 /6	1,007	-
	322	IVIL	30,333	50,555	40 /6	15,541	13,341		_				_	_
		_	_	_		_	_		_	_			_	_
		_	_	_		_	_		_	_			_	_
		_	_	_		_	_		_	_			_	_
		_	_	_		_	_		_	_			_	_
		_	_	_		_	_		_	_	_		_	_
		_	_	_		_	_		-	_	_		_	_
		_	_	_		_	_		-	_	_		_	_
		_	_	_		_	-		-	-	_		-	-
		_	-	-		-	-		-	-	-		-	-
		-	-	-		-	-		-	-	-		-	-
		-	-	-		-	-		-	-	-		-	-
		-	-	-		-	-		-	-	-		-	-
		-	-	-		-	-		-	-	-		-	-
		-	-	-		-	-		-	-	-		-	-
		-	-	-		-	-		-	-	-		-	-
		-	-	-		-	-		-	-	-		-	-
		-	-	-		-	-		-	-	-		-	-
		-	-	-		-	-		-	-	-		-	-
	F	rom Ea	ıst			109,464	109,464						92,590	-

Trip Distribution - Pop from South

10-mi	le Radi	ius											
RAZ	MPA	2020	2030	% of	2020	2030	RAZ	MPA	2020	2030	% of	2020	2030
SSE		Population	Population	TAZ	Adjusted	Adjusted	SSW		Population	Population	TAZ	Adjusted	Adjusted
328	CH	45,639	45,639	20%	9,128	9,128	325	СН	43,457		10%	4,346	_
329		39,078	39,078	70%	27,355	27,355	327	СН	23,575		30%	7,073	-
122	GC C	75	147	50%	38	74	328	CH	45,639		80%	36,511	-
123	GC	2,045	4,008	10%	205	401	120		600		75%	450	-
129	PC	18,869	36,984	10%	1,887	3,698	121	GC	-			-	-
	-	-	-		-	-	122	GC	75		10%	8	-
	-	-	-		-	-	123	GC	2,045		4%	82	-
	-	-	-		-	-		-	-			-	-
	-	-	-		-	-		-	-			-	-
	-	-	-		-	-		-	-			-	-
	-	-	-		-	-		-	-			-	-
	-	-	-		-	-		-	-	-		-	-
	-	-	-		-	-		-	-	-		-	-
	-	-	-		-	-		-	-	-		-	-
	_	-	_		_	_		_	_	_		_	-
	_	_	_		_	_		_	_	_		_	_
	_	_	_		_	_		_	_	_		_	_
	-	_	_		_	_		-	_	_		_	-
	-	-	-		_	-		-	-	-		_	-
	-	-	-		-	-		-	-	-		-	-
	-	-	-		-	-		-	-	-		-	-
	-	-	-		-	-		-	-	-		-	-
	-	-	-		-	-		-	-	-		-	-
	-	-	-		-	-		-	-	-		-	-
_		d.			00.044	40.055						40, 400	
F	rom So	uth			38,611	40,655						48,469	40.055
												87,080	40,655

APPENDIX F

BACKGROUND TRAFFIC



Location of counts: Scottsdale Road between Indian Bend and Lincoln

Source(s): https://www.scottsdaleaz.gov/transportation/studies-reports/traffic-volume

				Expansion
			Avg Growth	Factor to
	Year	Volume	Rate to 2012	2012
Beginning	2012	43,500		
End	2014	45,000	1.7%	0.967

Growth Rate Used 1.7% Per-Year Multiplier 1.017

	Expansion	
Year	Factor(s)	
2019	1.000	
2020	1.017	<- Expansion factor to opening
2021	1.034	
2022	1.052	
2023	1.070	
2024	1.088	
2025	1.106	
2026	1.125	
2027	1.144	
2028	1.164	
2029	1.184	
2030	1.204	
2031	1.224	
2032	1.245	
2033	1.266	
2034	1.288	
2035	1.310	
2036	1.332	
2037	1.354	
2038	1.378	
2039	1.401	



APPENDIX G

2020 PEAK HOUR ANALYSIS



Mountain Shadows Resort Background AM 1: Camelback Manor Dr/Desert Fairways Dr & Lincoln Dr Timings

	•	-	•	—	1	†	-	ţ	
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	
Lane Configurations	*	↑ ₽	ሻ	↑ ₽		4	*	1>	,
Traffic Volume (vph)	68	1285	5	1048	2	1	75	1	
Future Volume (vph)	68	1285	5	1048	2	1	75	1	
Turn Type	pm+pt	NA	Perm	NA	Perm	NA	Perm	NA	
Protected Phases	1	6		2		4		8	
Permitted Phases	6		2		4		8		
Detector Phase	1	6	2	2	4	4	8	8	
Switch Phase									
Minimum Initial (s)	4.0	15.0	15.0	15.0	7.0	7.0	7.0	7.0	
Minimum Split (s)	8.0	26.0	26.0	26.0	30.0	30.0	30.0	30.0	
Total Split (s)	8.0	35.0	27.0	27.0	30.0	30.0	30.0	30.0	
Total Split (%)	12.3%	53.8%	41.5%	41.5%	46.2%	46.2%	46.2%	46.2%	
Yellow Time (s)	3.0	4.5	4.5	4.5	3.0	3.0	3.0	3.0	
All-Red Time (s)	1.0	1.5	1.5	1.5	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	0.0	0.0	
Total Lost Time (s)	4.0	6.0	6.0	6.0		6.0	6.0	6.0	
Lead/Lag	Lead		Lag	Lag					
Lead-Lag Optimize?	Yes		Yes	Yes					
Recall Mode	None	C-Max	C-Max	C-Max	None	None	None	None	
Act Effct Green (s)	47.8	47.0	41.0	41.0		9.8	9.8	9.8	
Actuated g/C Ratio	0.74	0.72	0.63	0.63		0.15	0.15	0.15	
v/c Ratio	0.22	0.55	0.03	0.55		0.08	0.43	0.19	
Control Delay	5.4	7.1	8.0	11.0		15.1	30.5	9.2	
Queue Delay	0.0	0.0	0.0	0.0		0.0	0.0	0.0	
Total Delay	5.4	7.1	8.0	11.0		15.1	30.5	9.2	
LOS	Α	Α	Α	В		В	С	Α	
Approach Delay		7.0		11.0		15.1		22.4	
Approach LOS		Α		В		В		С	
Intersection Summary									
Cycle Length: 65									
Actuated Cycle Length: 65									
Offset: 0 (0%), Referenced	to phase 2	:WBTL ar	nd 6:EBT	L, Start of	Green				
Natural Cycle: 70	'								
Control Type: Actuated-Con	ordinated								
Maximum v/c Ratio: 0.55									
Intersection Signal Delay: 9	0.5			li	ntersectio	n LOS: A			
Intersection Capacity Utiliza	ation 74.0%	5		10	CU Level	of Service	e D		
Analysis Period (min) 15									

Splits and Phases: 1: Camelback Manor Dr/Desert Fairways Dr & Lincoln Dr



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Mountain Shadows Resort Background AM

HCM 6th LOS

1: Camelback Manor Dr/Desert Fairways Dr & Lincoln Dr HCM 6th Signalized Intersection Summary

	۶	→	•	•	←	•	4	†	1	>	ļ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	↑ ↑		ሻ	↑ ↑			4		ሻ	1>	
Traffic Volume (veh/h)	68	1285	4	5	1048	56	2	1	5	75	1	45
Future Volume (veh/h)	68	1285	4	5	1048	56	2	1	5	75	1	45
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1945	1870	1870	1870	1870
Adj Flow Rate, veh/h	75	1412	0	6	1164	29	5	2	2	89	1	30
Peak Hour Factor	0.91	0.91	0.91	0.90	0.90	0.90	0.40	0.40	0.40	0.84	0.84	0.84
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	341	2552	0	313	2165	54	151	59	35	250	5	150
Arrive On Green	0.05	0.72	0.00	0.20	0.20	0.20	0.10	0.10	0.10	0.10	0.10	0.10
Sat Flow, veh/h	1781	3647	0	381	3543	88	669	607	365	1412	51	1542
Grp Volume(v), veh/h	75	1412	0	6	584	609	9	0	0	89	0	31
Grp Sat Flow(s),veh/h/ln	1781	1777	0	381	1777	1854	1641	0	0	1412	0	1593
Q Serve(g_s), s	0.9	12.1	0.0	0.8	19.1	19.1	0.0	0.0	0.0	3.6	0.0	1.2
Cycle Q Clear(g_c), s	0.9	12.1	0.0	6.0	19.1	19.1	0.3	0.0	0.0	3.9	0.0	1.2
Prop In Lane	1.00		0.00	1.00		0.05	0.56		0.22	1.00		0.97
Lane Grp Cap(c), veh/h	341	2552	0	313	1086	1133	246	0	0	250	0	155
V/C Ratio(X)	0.22	0.55	0.00	0.02	0.54	0.54	0.04	0.00	0.00	0.36	0.00	0.20
Avail Cap(c_a), veh/h	369	2552	0	313	1086	1133	673	0	0	634	0	588
HCM Platoon Ratio	1.00	1.00	1.00	0.33	0.33	0.33	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	1.00	1.00	1.00	1.00	0.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	7.4	4.3	0.0	14.6	17.7	17.7	26.6	0.0	0.0	28.2	0.0	27.0
Incr Delay (d2), s/veh	0.1	0.9	0.0	0.1	1.9	1.8	0.1	0.0	0.0	0.9	0.0	0.6
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.2	2.4	0.0	0.1	9.3	9.7	0.1	0.0	0.0	1.4	0.0	0.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	7.5	5.2	0.0	14.7	19.6	19.6	26.7	0.0	0.0	29.1	0.0	27.6
LnGrp LOS	Α	Α	Α	В	В	В	С	Α	Α	С	Α	C
Approach Vol, veh/h		1487			1199			9			120	
Approach Delay, s/veh		5.3			19.6			26.7			28.7	
Approach LOS		Α			В			С			С	
Timer - Assigned Phs	1	2		4		6		8				
Phs Duration (G+Y+Rc), s	7.0	45.7		12.3		52.7		12.3				
Change Period (Y+Rc), s	4.0	6.0		6.0		6.0		6.0				
Max Green Setting (Gmax), s	4.0	21.0		24.0		29.0		24.0				
Max Q Clear Time (g_c+l1), s	2.9	21.1		2.3		14.1		5.9				
Green Ext Time (p_c), s	0.0	0.0		0.0		8.5		0.3				
Intersection Summary												
HCM 6th Ctrl Delay			12.4									

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Int Delay, s/veh	0.2							
			WDI	WDT	NDI	NDD		
Movement	EBT	EBR	WBL	WBT	NBL	NBR	ĺ	
Lane Configurations	44	7	_	414	C	7		
Traffic Vol, veh/h	1352		7		0	13		
Future Vol, veh/h	1352	23	7	1123	0	13		
Conflicting Peds, #/hr	0	-	0	0	0	0		
Sign Control	Free		Free	Free	Stop	Stop		
RT Channelized	-	140110	-	None	-	None		
Storage Length	-		-	-	-	0		
Veh in Median Storage,			-	0	0	-		
Grade, %	0		-	0	0	-		
Peak Hour Factor	92	92	95	95	88	88		
Heavy Vehicles, %	2	2	2	2	2	2		
Mvmt Flow	1470	25	7	1182	0	15		
Maine/Minne	4-11		M-:0		M:1		í	
	Major1		Major2		Vinor1	205		
Conflicting Flow All	0	0	1495	0		735		
Stage 1	-	-	-		-	-		
Stage 2	-	-		-	-	-		
Critical Hdwy	-	-	4.14	-	-	6.94		
Critical Hdwy Stg 1	-	-	-	-	-	-		
Critical Hdwy Stg 2	-	-	-	-	-	-		
Follow-up Hdwy	-	-	2.22	-	-	3.32		
Pot Cap-1 Maneuver	-	-	*808	-	0	*540		
Stage 1	-	-	-	-	0	-		
Stage 2	-	-	-	-	0	-		
Platoon blocked, %	-	-	1	-		1		
Mov Cap-1 Maneuver		-	*808	-	-	*540		
Mov Cap-2 Maneuver			-		-	-		
Stage 1		-	-	-	-	-		
Stage 2								
Stage 2								
Approach	EB		WB		NB			
HCM Control Delay, s	0		0.2		11.9			
HCM LOS					В			
Minor Lane/Major Mvm	t	NBLn1	EBT	EBR	WBL	WBT		
Capacity (veh/h)		540	LDI	LDIX	* 808	WD1		
HCM Carter Delay (2)		0.027			0.009	- 0.1		
HCM Control Delay (s)		11.9	-		9.5	0.1		
HCM Lane LOS		В	-	-	Α	Α		
		0.1	-	-	0	-		
HCM 95th %tile Q(veh)		0.1						
HCM 95th %tile Q(veh) Notes		0.1						

ntersection							
nt Delay, s/veh	0.3						
Novement	EBT	EBR	WBL	WBT	NBL	NBR	
ane Configurations	44	7	7	44	W	HUIT	
raffic Vol, veh/h	1337	27	11	1099	23	14	
uture Vol. veh/h	1337	27	11	1099	23	14	
conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-			None	-	None	
Storage Length		135	80	-	0	-	
/eh in Median Storage	. # 0	-		0	1		
Grade, %	0			0	0		
Peak Hour Factor	90	90	94	94	77	77	
Heavy Vehicles, %	2	2	2	2	2	2	
Nymt Flow	1486	30	12	1169	30	18	
		- 50		,	- 00	.5	
Major/Minor N	Major1		Major2		Minor1		
onflicting Flow All	0		1516	0	2095	743	
Stage 1	0	-	1310	-	1486	743	
Stage 2	- 1				609		
Critical Hdwy			4.14		6.84	6.94	
Critical Hdwy Stg 1			4.14		5.84	0.74	
Critical Hdwy Stg 2					5.84		
Follow-up Hdwy			2.22		3.52	3.32	
Pot Cap-1 Maneuver			792		*79	*540	
Stage 1			172		*510	340	
Stage 2					*600		
Platoon blocked. %			1		1	1	
Mov Cap-1 Maneuver			792		*78	*540	
Mov Cap-1 Maneuver	- 1		172		*283	340	
Stage 1					*502		
Stage 2					*600		
Stage 2					300		
pproach	EB		WB		NB		
HCM Control Delay, s	0		0.1		17.1		
HCM LOS	0		0.1		17.1 C		
ICIVI LUS					C		
liner Lane/Major May		NDI p4	EDT	EDD	WDI	WDT	
Minor Lane/Major Mvm	IL	NBLn1	EBT	EBR	WBL	WBT	
Capacity (veh/h)		345	-	-	792	-	
ICM Lane V/C Ratio		0.139	-	-	0.015	-	
HCM Control Delay (s)		17.1	-	-	9.6	-	
HCM Lane LOS		C	-	-	A	-	
HCM 95th %tile Q(veh))	0.5	-	-	0	-	
lotes							

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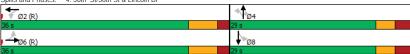
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Mountain Shadows Resort Background AM

4: 56th St/56th St & Lincoln Dr

	•	→	•	•	4	†	/	-	ļ
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT
Lane Configurations	*	† 1>	ች	∱ Љ		ર્ન	7		4
Traffic Volume (vph)	7	1297	14	1074	34	1	15	4	0
Future Volume (vph)	7	1297	14	1074	34	1	15	4	0
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	Perm	NA
Protected Phases		6		2		4			8
Permitted Phases	6		2		4		4	8	
Detector Phase	6	6	2	2	4	4	4	8	8
Switch Phase									
Minimum Initial (s)	15.0	15.0	15.0	15.0	7.0	7.0	7.0	7.0	7.0
Minimum Split (s)	31.5	31.5	31.5	31.5	13.0	13.0	13.0	29.0	29.0
Total Split (s)	36.0	36.0	36.0	36.0	29.0	29.0	29.0	29.0	29.0
Total Split (%)	55.4%	55.4%	55.4%	55.4%	44.6%	44.6%	44.6%	44.6%	44.6%
Yellow Time (s)	4.5	4.5	4.5	4.5	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.0	2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	0.0		0.0
Total Lost Time (s)	6.5	6.5	6.5	6.5		6.0	6.0		6.0
Lead/Lag									
Lead-Lag Optimize?									
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None	None	None	None
Act Effct Green (s)	52.6	52.6	52.6	52.6		7.7	7.7		7.7
Actuated g/C Ratio	0.81	0.81	0.81	0.81		0.12	0.12		0.12
v/c Ratio	0.02	0.53	0.07	0.40		0.21	0.07		0.04
Control Delay	2.3	6.1	4.7	4.0		28.5	0.5		0.3
Queue Delay	0.0	0.0	0.0	0.0		0.0	0.0		0.0
Total Delay	2.3	6.1	4.7	4.0		28.5	0.5		0.3
LOS	Α	Α	Α	Α		С	Α		Α
Approach Delay		6.1		4.1		20.2			0.3
Approach LOS		Α		Α		С			Α
Intersection Summary									
Cycle Length: 65									
Actuated Cycle Length: 65									
Offset: 0 (0%), Referenced	to phase 2	:WBTL ai	nd 6:EBT	L, Start of	f Green				
Natural Cycle: 65	'								
Control Type: Actuated-Con	ordinated								
Maximum v/c Ratio: 0.53									
Intersection Signal Delay: 5	5.5			ıl	ntersectio	n LOS: A			
Intersection Capacity Utiliza	ation 64.5%	5		10	CU Level	of Service	e C		
Analysis Period (min) 15									
Cultural Disease 4 5 %	41- CHE/!!	Ct 0 1 :-	-l- D-						
Splits and Phases: 4: 56	th St/56th	St & Linc	זע חוכ						



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Mountain Shadows Resort Background AM

HCM 6th LOS

4: 56th St/56th St & Lincoln Dr HCM 6th Signalized Intersection Summary

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	↑ ↑			ħβ			ની	7		4	
Traffic Volume (veh/h)	7	1297	48	14	1074	6	34	1	15	4	0	3
Future Volume (veh/h)	7	1297	48	14	1074	6	34	1	15	4	0	3
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	8	1457	26	15	1131	2	37	1	7	5	0	0
Peak Hour Factor	0.89	0.89	0.89	0.95	0.95	0.95	0.93	0.93	0.93	0.88	0.88	0.88
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	423	2656	47	312	2707	5	205	4	101	172	0	0
Arrive On Green	0.74	0.74	0.74	0.74	0.74	0.74	0.06	0.06	0.06	0.06	0.00	0.00
Sat Flow, veh/h	497	3572	64	356	3640	6	1495	65	1585	960	0	0
Grp Volume(v), veh/h	8	724	759	15	552	581	38	0	7	5	0	0
Grp Sat Flow(s),veh/h/ln	497	1777	1859	356	1777	1869	1560	0	1585	960	0	0
Q Serve(g_s), s	0.4	11.5	11.5	1.2	7.5	7.5	0.0	0.0	0.3	0.2	0.0	0.0
Cycle Q Clear(g_c), s	7.9	11.5	11.5	12.7	7.5	7.5	1.3	0.0	0.3	1.5	0.0	0.0
Prop In Lane	1.00		0.03	1.00		0.00	0.97		1.00	1.00		0.00
Lane Grp Cap(c), veh/h	423	1321	1382	312	1321	1390	209	0	101	172	0	0
V/C Ratio(X)	0.02	0.55	0.55	0.05	0.42	0.42	0.18	0.00	0.07	0.03	0.00	0.00
Avail Cap(c_a), veh/h	423	1321	1382	312	1321	1390	623	0	561	580	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	4.6	3.6	3.6	6.4	3.1	3.1	29.1	0.0	28.6	29.8	0.0	0.0
Incr Delay (d2), s/veh	0.1	1.6	1.6	0.3	1.0	0.9	0.4	0.0	0.3	0.1	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.0	2.2	2.2	0.1	1.4	1.5	0.6	0.0	0.1	0.1	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	4.7	5.2	5.2	6.7	4.1	4.0	29.5	0.0	28.9	29.9	0.0	0.0
LnGrp LOS	Α	Α	Α	Α	Α	Α	С	Α	С	С	Α	A
Approach Vol, veh/h		1491			1148			45			5	
Approach Delay, s/veh		5.2			4.1			29.4			29.9	
Approach LOS		Α			Α			С			С	
Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		54.8		10.2		54.8		10.2				
Change Period (Y+Rc), s		6.5		6.0		6.5		6.0				
Max Green Setting (Gmax), s		29.5		23.0		29.5		23.0				
Max Q Clear Time (g_c+I1), s		14.7		3.3		13.5		3.5				
Green Ext Time (p_c), s		6.4		0.1		8.9		0.0				
Intersection Summary												
HCM 6th Ctrl Delay			5.2									

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Mountain Shadows Resort Background PM

1: Camelback Manor Dr/Desert Fairways Dr & Lincoln Dr

	•	-	•	-	1	Ť	-	¥	
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	
Lane Configurations	ሻ	↑ ↑	ሻ	↑ ↑		4	7	f)	Т
Traffic Volume (vph)	63	1091	5	1287	4	2	114	2	
Future Volume (vph)	63	1091	5	1287	4	2	114	2	
Turn Type	pm+pt	NA	Perm	NA	Perm	NA	Perm	NA	
Protected Phases	1	6		2		4		8	
Permitted Phases	6		2		4		8		
Detector Phase	1	6	2	2	4	4	8	8	
Switch Phase									
Minimum Initial (s)	4.0	15.0	15.0	15.0	7.0	7.0	7.0	7.0	
Minimum Split (s)	8.0	26.0	26.0	26.0	30.0	30.0	30.0	30.0	
Total Split (s)	8.0	35.0	27.0	27.0	30.0	30.0	30.0	30.0	
Total Split (%)	12.3%	53.8%	41.5%	41.5%	46.2%	46.2%	46.2%	46.2%	
Yellow Time (s)	3.0	4.5	4.5	4.5	3.0	3.0	3.0	3.0	
All-Red Time (s)	1.0	1.5	1.5	1.5	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	0.0	0.0	
Total Lost Time (s)	4.0	6.0	6.0	6.0		6.0	6.0	6.0	
Lead/Lag	Lead		Lag	Lag					
Lead-Lag Optimize?	Yes		Yes	Yes					
Recall Mode	None	C-Max	C-Max	C-Max	None	None	None	None	
Act Effct Green (s)	42.3	40.3	34.3	34.3		12.7	12.7	12.7	
Actuated g/C Ratio	0.65	0.62	0.53	0.53		0.20	0.20	0.20	
v/c Ratio	0.26	0.54	0.02	0.85		0.05	0.57	0.29	
Control Delay	7.7	9.0	21.6	29.8		14.8	31.1	6.7	
Queue Delay	0.0	0.0	0.0	0.0		0.0	0.0	0.0	
Total Delay	7.7	9.0	21.6	29.8		14.8	31.1	6.7	
LOS	Α	Α	С	С		В	С	Α	
Approach Delay		9.0		29.7		14.8		20.5	
Approach LOS		Α		С		В		С	
Intersection Summary									
Cycle Length: 65									
Actuated Cycle Length: 65									
Offset: 49 (75%), Reference	ed to phase	e 2:WBTL	and 6:Ef	BTL, Start	of Green				
Natural Cycle: 80									

Natural Cycle: 80 Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.85

Intersection Signal Delay: 20.5
Intersection Capacity Utilization 70.8%
Analysis Period (min) 15

Intersection LOS: C ICU Level of Service C

Splits and Phases: 1: Camelback Manor Dr/Desert Fairways Dr & Lincoln Dr



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HCM 6th LOS

1: Camelback Manor Dr/Desert Fairways Dr & Lincoln Dr HCM 6th Signalized Intersection Summary

	۶	→	*	1	—	4	1	†	1	\	↓	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	† }		ň	† }			44		7	- 1}	
Traffic Volume (veh/h)	63	1091	3	5	1287	171	4	2	5	114	2	85
Future Volume (veh/h)	63	1091	3	5	1287	171	4	2	5	114	2	85
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1945	1870	1870	1870	1870
Adj Flow Rate, veh/h	68	1186	0	5	1384	98	6	3	3	154	3	74
Peak Hour Factor	0.92	0.92	0.92	0.93	0.93	0.93	0.69	0.69	0.69	0.74	0.74	0.74
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	269	2421	0	357	1940	137	153	76	49	316	8	206
Arrive On Green	0.04	0.68	0.00	0.39	0.39	0.39	0.13	0.13	0.13	0.13	0.13	0.13
Sat Flow, veh/h	1781	3647	0	472	3367	238	522	566	363	1410	62	1532
Grp Volume(v), veh/h	68	1186	0	5	728	754	12	0	0	154	0	77
Grp Sat Flow(s),veh/h/ln	1781	1777	0	472	1777	1828	1451	0	0	1410	0	1595
Q Serve(g_s), s	0.9	10.4	0.0	0.5	22.5	22.7	0.0	0.0	0.0	3.2	0.0	2.9
Cycle Q Clear(g_c), s	0.9	10.4	0.0	4.0	22.5	22.7	2.9	0.0	0.0	6.1	0.0	2.9
Prop In Lane	1.00		0.00	1.00		0.13	0.50		0.25	1.00		0.96
Lane Grp Cap(c), veh/h	269	2421	0	357	1024	1053	278	0	0	316	0	214
V/C Ratio(X)	0.25	0.49	0.00	0.01	0.71	0.72	0.04	0.00	0.00	0.49	0.00	0.36
Avail Cap(c_a), veh/h	301	2421	0	357	1024	1053	643	0	0	648	0	589
HCM Platoon Ratio	1.00	1.00	1.00	0.67	0.67	0.67	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	1.00	1.00	1.00	1.00	0.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	9.9	5.0	0.0	10.8	15.4	15.4	24.5	0.0	0.0	26.8	0.0	25.6
Incr Delay (d2), s/veh	0.2	0.7	0.0	0.1	4.2	4.2	0.1	0.0	0.0	1.2	0.0	1.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.3	2.4	0.0	0.0	10.1	10.5	0.2	0.0	0.0	2.3	0.0	1.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	10.1	5.7	0.0	10.9	19.6	19.6	24.6	0.0	0.0	28.0	0.0	26.6
LnGrp LOS	В	Α	Α	В	В	В	С	Α	A	С	Α	С
Approach Vol, veh/h		1254			1487			12			231	
Approach Delay, s/veh		5.9			19.6			24.6			27.5	
Approach LOS		Α			В			С			С	
Timer - Assigned Phs	1	2		4		6		8				
Phs Duration (G+Y+Rc), s	6.8	43.5		14.7		50.3		14.7				
Change Period (Y+Rc), s	4.0	6.0		6.0		6.0		6.0				
Max Green Setting (Gmax), s	4.0	21.0		24.0		29.0		24.0				
Max Q Clear Time (g_c+I1), s	2.9	24.7		4.9		12.4		8.1				
Green Ext Time (p_c), s	0.0	0.0		0.0		7.5		8.0				
Intersection Summary												
HCM 6th Ctrl Delay			14.5									

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Mountain Shadows Resort Background PM 3: East Access & Lincoln Dr HCM 6th TWSC

Intersection								
Intersection Int Delay, s/veh	0.7							
		EDD	WDI	MDT	NIDI	NDD		
Movement Lane Configurations	EBT	EBR	WBL	WBT	NBL	NBR		
Traffic Vol, veh/h	↑ ↑	7 35	15	4 ↑↑ 1514	0	7 28		
Future Vol. veh/h	1212	35	15	1514	0	28		
Conflicting Peds, #/hr	0	0	0	1514	0	0		
Sign Control	Free	Free	Free	Free	Stop	Stop		
RT Channelized	riee -	None	riee -	None	Stop -	None		
Storage Length		125		NOTIC		0		
Veh in Median Storage		123		0	0	-		
Grade, %	0			0	0			
Peak Hour Factor	95	95	94	94	63	63		
Heavy Vehicles, %	2	2	2	2	2	2		
Mymt Flow	1276	37	16	1611	0	44		
	.2.0	- 01	.0		- 0			
				_				
	Major1		Major2		/linor1	400		
Conflicting Flow All	0	0	1313	0	-	638		
Stage 1	-		-	-	-	-		
Stage 2	-		414		-	- / 04		
Critical Hdwy	-	-	4.14	-	-	6.94		
Critical Hdwy Stg 1	-	-	-		-	-		
Critical Hdwy Stg 2	-		2.22	-	-	3.32		
Follow-up Hdwy Pot Cap-1 Maneuver	-		*880	-	0	*588		
			880	- 1	0	588		
Stage 1 Stage 2		-	-	- 1	0	-		
Platoon blocked, %	- 1		1		0	1		
Mov Cap-1 Maneuver			*880	-		*588		
Mov Cap-1 Maneuver	- 1		880			200		
Stage 1			- 1					
Stage 2								
Staye 2								
Approach	EB		WB		NB			
HCM Control Delay, s	0		0.9		11.6			
HCM LOS					В			
Minor Lane/Major Mvm	ıt	NBLn1	EBT	EBR	WBL	WBT		
Capacity (veh/h)		588		-	* 880			
HCM Lane V/C Ratio		0.076			0.018			
HCM Control Delay (s)		11.6			9.2	0.8		
HCM Lane LOS		В			A	A		
HCM 95th %tile Q(veh)		0.2			0.1	-		
Notes	- 1					_		_
 Volume exceeds cap 	oacity	\$: De	elay exc	ceeds 30	JUS	+: Com	outation Not Defined	*: /

nt Delay, s/veh	0.1							
Movement	EBT	EBR	WBL	WBT	NBL	NBR		
ane Configurations	^	7	7	- 44	W			
raffic Vol, veh/h	1221	19	17	1500	14	26		
uture Vol, veh/h	1221	19	17	1500	14	26		
Conflicting Peds, #/hr	0	0	0	0	0	0		
Sign Control	Free	Free	Free	Free	Stop	Stop		
RT Channelized		None	-	None	-	None		
Storage Length	-	135	80	-	0	-		
/eh in Median Storage	, # 0	-	-	0	1	-		
Grade, %	0	-	-	0	0	-		
Peak Hour Factor	96	96	94	94	72	72		
Heavy Vehicles, %	2	2	2	2	2	2		
Nymt Flow	1272	20	18	1596	19	36		
lajor/Minor I	Major1		Major2	1	Minor1			
onflicting Flow All	0	0	1292	0	2106	636		
Stage 1	-	-	-	-	1272	-		
Stage 2					834			
Critical Hdwy			4.14		6.84	6.94		
Critical Hdwy Stg 1			-		5.84	-		
Critical Hdwy Stg 2					5.84			
follow-up Hdwy			2.22		3.52	3.32		
Pot Cap-1 Maneuver			*880		-	*588		
Stage 1			-		*555	-		
Stage 2					*419			
Platoon blocked, %			1		2	1		
Mov Cap-1 Maneuver			*880		-	*588		
Mov Cap-2 Maneuver			-		*210	-		
Stage 1					*544			
Stage 2			-		*419			
pproach	EB		WB		NB			
HCM Control Delay, s	0		0.1					
HCM LOS			3.1					
Minor Lane/Major Mvm	nt I	NBLn1	EBT	EBR	WBL	WBT		
Capacity (veh/h)	it I	WOLIT	LUI	LDI	* 880	WDI		
ICM Lane V/C Ratio					0.021			
ICM Control Delay (s)			-		9.2			
ICM Lane LOS					9.2 A			
HCM 95th %tile Q(veh)	١		-	-	0.1			
, ,)				U. I			
otes								
: Volume exceeds cap	pacity	\$: De	elay exc	ceeds 3	00s	+: Com	putation Not Defined	*: All major volume in platoon

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4: 56th St/56th St & Lincoln Dr Timings

	•	→	•	←	4	†	/	-	ļ	
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	
Lane Configurations	1	↑ Ъ	ች	↑ 1>		ર્ન	7		4	
Fraffic Volume (vph)	33	1180	48	1438	65	2	23	3	2	
uture Volume (vph)	33	1180	48	1438	65	2	23	3	2	
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	Perm	NA	
Protected Phases		6		2		4			8	
Permitted Phases	6		2		4		4	8		
Detector Phase	6	6	2	2	4	4	4	8	8	
Switch Phase										
Minimum Initial (s)	15.0	15.0	15.0	15.0	7.0	7.0	7.0	7.0	7.0	
Minimum Split (s)	31.5	31.5	31.5	31.5	13.0	13.0	13.0	29.0	29.0	
Total Split (s)	35.0	35.0	35.0	35.0	30.0	30.0	30.0	30.0	30.0	
Total Split (%)	53.8%	53.8%	53.8%	53.8%	46.2%	46.2%	46.2%	46.2%	46.2%	
Yellow Time (s)	4.5	4.5	4.5	4.5	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	0.0		0.0	
Total Lost Time (s)	6.5	6.5	6.5	6.5		6.0	6.0		6.0	
Lead/Lag										
Lead-Lag Optimize?										
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None	None	None	None	
Act Effct Green (s)	47.1	47.1	47.1	47.1		9.3	9.3		9.3	
Actuated g/C Ratio	0.72	0.72	0.72	0.72		0.14	0.14		0.14	
v/c Ratio	0.21	0.52	0.21	0.60		0.40	0.09		0.13	
Control Delay	5.8	3.5	7.9	7.6		31.0	2.9		19.3	
Queue Delay	0.0	0.0	0.0	0.0		0.0	0.0		0.0	
Total Delay	5.8	3.5	7.9	7.6		31.0	2.9		19.3	
LOS	Α	A 3.5	Α	A 7.6		C	Α		B 19.3	
Approach Delay						23.8				
Approach LOS		Α		Α		С			В	
ntersection Summary										
Cycle Length: 65										
Actuated Cycle Length: 65										
Offset: 29 (45%), Reference	d to phase	2:WBTL	and 6:El	3TL, Start	of Green					
latural Cycle: 70										
Control Type: Actuated-Coo	rdinated									
Maximum v/c Ratio: 0.60										
ntersection Signal Delay: 6.					ntersectio		_			
ntersection Capacity Utiliza	tion 61.6%			10	CU Level	of Service	e B			
Analysis Period (min) 15										
Splits and Phases: 4: 56th	St/56th	St & Linco	oln Dr							
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Mountain Shadows Resort Background PM

HCM 6th LOS

4: 56th St/56th St & Lincoln Dr HCM 6th Signalized Intersection Summary

	۶	→	•	•	←	•	4	†	1	-	ļ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	↑ ↑		ሻ	↑ ↑			ની	7		4	
Traffic Volume (veh/h)	33	1180	36	48	1438	32	65	2	23	3	2	14
Future Volume (veh/h)	33	1180	36	48	1438	32	65	2	23	3	2	14
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	36	1283	28	51	1514	23	73	2	15	5	3	14
Peak Hour Factor	0.92	0.92	0.92	0.95	0.95	0.95	0.89	0.89	0.89	0.63	0.63	0.63
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	281	2540	55	410	2559	39	240	5	148	85	38	96
Arrive On Green	1.00	1.00	1.00	0.71	0.71	0.71	0.09	0.09	0.09	0.09	0.09	0.09
Sat Flow, veh/h	338	3556	78	419	3583	54	1401	55	1585	178	411	1030
Grp Volume(v), veh/h	36	641	670	51	750	787	75	0	15	22	0	0
Grp Sat Flow(s),veh/h/ln	338	1777	1856	419	1777	1861	1457	0	1585	1618	0	0
Q Serve(g_s), s	2.4	0.0	0.0	2.6	13.6	13.6	2.3	0.0	0.6	0.0	0.0	0.0
Cycle Q Clear(g_c), s	16.0	0.0	0.0	2.6	13.6	13.6	3.1	0.0	0.6	3.0	0.0	0.0
Prop In Lane	1.00		0.04	1.00		0.03	0.97		1.00	0.23		0.64
Lane Grp Cap(c), veh/h	281	1269	1326	410	1269	1329	245	0	148	219	0	0
V/C Ratio(X)	0.13	0.50	0.51	0.12	0.59	0.59	0.31	0.00	0.10	0.10	0.00	0.00
Avail Cap(c_a), veh/h	281	1269	1326	410	1269	1329	633	0	585	652	0	0
HCM Platoon Ratio	2.00	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	2.3	0.0	0.0	3.0	4.6	4.6	28.0	0.0	27.0	27.1	0.0	0.0
Incr Delay (d2), s/veh	0.9	1.4	1.4	0.6	2.0	1.9	0.7	0.0	0.3	0.2	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.1	0.5	0.5	0.2	3.1	3.2	1.1	0.0	0.2	0.3	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	3.3	1.4	1.4	3.6	6.6	6.5	28.7	0.0	27.3	27.3	0.0	0.0
LnGrp LOS	А	Α	Α	Α	A	A	С	A	С	С	A	A
Approach Vol, veh/h		1347			1588			90			22	
Approach Delay, s/veh		1.5			6.5			28.5			27.3	
Approach LOS		Α			Α			С			С	
Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		52.9		12.1		52.9		12.1				
Change Period (Y+Rc), s		6.5		6.0		6.5		6.0				
Max Green Setting (Gmax), s		28.5		24.0		28.5		24.0				
Max Q Clear Time (g_c+I1), s		15.6		5.1		18.0		5.0				
Green Ext Time (p_c), s		8.4		0.3		6.2		0.1				
Intersection Summary		<u> </u>		<u> </u>	<u> </u>		<u> </u>		<u> </u>	<u> </u>		
HCM 6th Ctrl Delay			5.1									

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Mountain Shadows Resort Total AM 1: Camelback Manor Dr/Desert Fairways Dr & Lincoln Dr Timings

	•	-	1	•	1	†	-	↓	
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	
Lane Configurations	ች	↑ Ъ	ች	↑ 1>		4	7	f)	_
Traffic Volume (vph)	68	1286	5	1048	2	1	75	1	
Future Volume (vph)	68	1286	5	1048	2	1	75	1	
Turn Type	pm+pt	NA	Perm	NA	Perm	NA	Perm	NA	
Protected Phases	1	6		2		4		8	
Permitted Phases	6		2		4		8		
Detector Phase	1	6	2	2	4	4	8	8	
Switch Phase									
Minimum Initial (s)	4.0	15.0	15.0	15.0	7.0	7.0	7.0	7.0	
Minimum Split (s)	8.0	26.0	26.0	26.0	30.0	30.0	30.0	30.0	
Total Split (s)	8.0	35.0	27.0	27.0	30.0	30.0	30.0	30.0	
Total Split (%)	12.3%	53.8%	41.5%	41.5%	46.2%	46.2%	46.2%	46.2%	
Yellow Time (s)	3.0	4.5	4.5	4.5	3.0	3.0	3.0	3.0	
All-Red Time (s)	1.0	1.5	1.5	1.5	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	0.0	0.0	
Total Lost Time (s)	4.0	6.0	6.0	6.0		6.0	6.0	6.0	
Lead/Lag	Lead		Lag	Lag					
Lead-Lag Optimize?	Yes		Yes	Yes					
Recall Mode	None	C-Max	C-Max	C-Max	None	None	None	None	
Act Effct Green (s)	47.8	47.0	41.0	41.0		9.8	9.8	9.8	
Actuated g/C Ratio	0.74	0.72	0.63	0.63		0.15	0.15	0.15	
v/c Ratio	0.22	0.55	0.03	0.55		0.08	0.43	0.19	
Control Delay	5.4	7.1	8.0	11.0		15.1	30.5	9.2	
Queue Delay	0.0	0.0	0.0	0.0		0.0	0.0	0.0	
Total Delay	5.4	7.1	8.0	11.0		15.1	30.5	9.2	
LOS	Α	Α	Α	В		В	С	Α	
Approach Delay		7.0		11.0		15.1		22.4	
Approach LOS		А		В		В		С	
Intersection Summary									
Cycle Length: 65									
Actuated Cycle Length: 65									
Offset: 0 (0%), Referenced t	o phase 2	:WBTL ar	nd 6:EBT	L, Start of	Green				
Natural Cycle: 70									
Control Type: Actuated-Coo	rdinated								
Maximum v/c Ratio: 0.55									
Intersection Signal Delay: 9.					ntersectio				
Intersection Capacity Utilizat	tion 74.0%	5		10	CU Level	of Service	e D		
Analysis Period (min) 15									

Splits and Phases: 1: Camelback Manor Dr/Desert Fairways Dr & Lincoln Dr



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Mountain Shadows Resort Total AM 1: Camelback Manor Dr/Desert Fairways Dr & Lincoln Dr HCM 6th Signalized Intersection Summary

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	ħβ		Ť	ħβ			4		7	1≽	
Traffic Volume (veh/h)	68	1286	4	5	1048	56	2	1	5	75	1	45
Future Volume (veh/h)	68	1286	4	5	1048	56	2	1	5	75	1	45
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1945	1870	1870	1870	1870
Adj Flow Rate, veh/h	75	1413	0	6	1164	29	5	2	2	89	1	30
Peak Hour Factor	0.91	0.91	0.91	0.90	0.90	0.90	0.40	0.40	0.40	0.84	0.84	0.84
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	341	2552	0	313	2165	54	151	59	35	250	5	150
Arrive On Green	0.05	0.72	0.00	0.20	0.20	0.20	0.10	0.10	0.10	0.10	0.10	0.10
Sat Flow, veh/h	1781	3647	0	380	3543	88	669	607	365	1412	51	1542
Grp Volume(v), veh/h	75	1413	0	6	584	609	9	0	0	89	0	31
Grp Sat Flow(s),veh/h/ln	1781	1777	0	380	1777	1854	1641	0	0	1412	0	1593
Q Serve(g_s), s	0.9	12.1	0.0	0.8	19.1	19.1	0.0	0.0	0.0	3.6	0.0	1.2
Cycle Q Clear(g_c), s	0.9	12.1	0.0	6.0	19.1	19.1	0.3	0.0	0.0	3.9	0.0	1.2
Prop In Lane	1.00		0.00	1.00		0.05	0.56		0.22	1.00		0.97
Lane Grp Cap(c), veh/h	341	2552	0	313	1086	1133	246	0	0	250	0	155
V/C Ratio(X)	0.22	0.55	0.00	0.02	0.54	0.54	0.04	0.00	0.00	0.36	0.00	0.20
Avail Cap(c_a), veh/h	369	2552	0	313	1086	1133	673	0	0	634	0	588
HCM Platoon Ratio	1.00	1.00	1.00	0.33	0.33	0.33	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	1.00	1.00	1.00	1.00	0.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	7.4	4.3	0.0	14.6	17.7	17.7	26.6	0.0	0.0	28.2	0.0	27.0
Incr Delay (d2), s/veh	0.1	0.9	0.0	0.1	1.9	1.8	0.1	0.0	0.0	0.9	0.0	0.6
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.2	2.4	0.0	0.1	9.3	9.7	0.1	0.0	0.0	1.4	0.0	0.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	7.5	5.2	0.0	14.7	19.6	19.6	26.7	0.0	0.0	29.1	0.0	27.6
LnGrp LOS	A	Α	A	В	В	В	С	A	A	С	A	С
Approach Vol, veh/h		1488			1199			9			120	
Approach Delay, s/veh		5.3			19.6			26.7			28.7	
Approach LOS		Α			В			С			С	
Timer - Assigned Phs	1	2		4		6		8				
Phs Duration (G+Y+Rc), s	7.0	45.7		12.3		52.7		12.3				
Change Period (Y+Rc), s	4.0	6.0		6.0		6.0		6.0				
Max Green Setting (Gmax), s	4.0	21.0		24.0		29.0		24.0				
Max Q Clear Time (g_c+l1), s	2.9	21.1		2.3		14.1		5.9				
Green Ext Time (p_c), s	0.0	0.0		0.0		8.5		0.3				
Intersection Summary												
HCM 6th Ctrl Delay			12.4									
HCM 6th LOS			В									

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Intersection						
Int Delay, s/veh	0.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	44	7	7	^		7
Traffic Vol, veh/h	1353	24	7	1124	0	13
Future Vol, veh/h	1353	24	7	1124	0	13
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	125	75	-	-	0
Veh in Median Storage	, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	95	95	88	88
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	1471	26	7	1183	0	15

Major/Minor	Major1	N	Najor2	Mir	nor1	
Conflicting Flow All	0	0	1497	0	-	736
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	4.14	-	-	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	2.22	-	-	3.32
Pot Cap-1 Maneuver	-	-	*808	-	0	*540
Stage 1	-	-	-	-	0	-
Stage 2	-	-	-	-	0	-
Platoon blocked, %	-	-	1	-		1
Mov Cap-1 Maneuve	r -	-	*808	-	-	*540
Mov Cap-2 Maneuve	r -	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	EB	WB	NB
HCM Control Delay, s	0	0.1	11.9
HCM LOS			В

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	540	-	-	* 808	-
HCM Lane V/C Ratio	0.027	-	-	0.009	-
HCM Control Delay (s)	11.9	-	-	9.5	-
HCM Lane LOS	В	-	-	Α	-
HCM 95th %tile Q(veh)	0.1	-	-	0	-

Notes			
~ Volume exceeds capacity	\$: Delay exceeds 300s	+: Computation Not Defined	*· All major volume in platoon

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Interception						
Intersection Int Delay, s/veh	0.3					
iiii beiay, s/veii	0.3					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^	7	7	44	14	
	1337	28	11	1099	23	14
Future Vol, veh/h	1337	28	11	1099	23	14
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	135	80	-	0	-
Veh in Median Storage,	# 0	-	-	0	1	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	94	94	77	77
Heavy Vehicles, %	2	2	2	2	2	2
	1486	31	12	1169	30	18
WWINCT IOW	1400	31	12	1107	50	10
	lajor1		Major2		Minor1	
Conflicting Flow All	0	0	1517	0	2095	743
Stage 1	-	-	-	-	1486	-
Stage 2	-	-	-	-	609	-
Critical Hdwy	-	-	4.14	-	6.84	6.94
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	-	-	-	-	5.84	-
Follow-up Hdwy	-	-	2.22	-	3.52	3.32
Pot Cap-1 Maneuver	-	-	791	-	*79	*540
Stage 1	-	-	-	-	*510	-
Stage 2	-	-	-	-	*600	-
Platoon blocked. %		-	1	-	1	1
Mov Cap-1 Maneuver		-	791	-	*78	*540
Mov Cap-2 Maneuver					*283	
Stage 1					*502	
Stage 2					*600	
Stage 2					000	
Approach	EB		WB		NB	
LICH Control Dolon -	0		0.1		17.1	
HCM Control Delay, s					C	
					C	
HCM LOS		UDI =1	EDT	EDD	_	WDT
HCM LOS Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
HCM LOS Minor Lane/Major Mvmt Capacity (veh/h)	1	345	-	-	WBL 791	-
HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	1	345 0.139	- :	-	WBL 791 0.015	
HCM LOS Minor Lane/Major Mvmt Capacity (veh/h)	1	345	-	-	WBL 791	-

Mountain Shadows Resort

HCM 95th %tile Q(veh)

Total AM

Notes
-: Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon

Mountain Shadows Resort Total AM 4: 56th St/56th St & Lincoln Dr Timings

	•	→	•	•	4	†	-	-	ļ
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT
Lane Configurations	*	↑ ₽	ች	↑ Ъ		ની	7		43-
Traffic Volume (vph)	7	1297	14	1075	34	1	15	4	0
Future Volume (vph)	7	1297	14	1075	34	1	15	4	0
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	Perm	NA
Protected Phases		6		2		4			8
Permitted Phases	6		2		4		4	8	
Detector Phase	6	6	2	2	4	4	4	8	8
Switch Phase									
Minimum Initial (s)	15.0	15.0	15.0	15.0	7.0	7.0	7.0	7.0	7.0
Minimum Split (s)	31.5	31.5	31.5	31.5	13.0	13.0	13.0	29.0	29.0
Total Split (s)	36.0	36.0	36.0	36.0	29.0	29.0	29.0	29.0	29.0
Total Split (%)	55.4%	55.4%	55.4%	55.4%	44.6%	44.6%	44.6%	44.6%	44.6%
Yellow Time (s)	4.5	4.5	4.5	4.5	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.0	2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	0.0		0.0
Total Lost Time (s)	6.5	6.5	6.5	6.5		6.0	6.0		6.0
Lead/Lag									
Lead-Lag Optimize?									
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None	None	None	None
Act Effct Green (s)	52.6	52.6	52.6	52.6		7.7	7.7		7.7
Actuated g/C Ratio	0.81	0.81	0.81	0.81		0.12	0.12		0.12
v/c Ratio	0.02	0.53	0.07	0.40		0.21	0.07		0.04
Control Delay	2.3	6.1	4.7	4.0		28.5	0.5		0.3
Queue Delay	0.0	0.0	0.0	0.0		0.0	0.0		0.0
Total Delay	2.3	6.1	4.7	4.0		28.5	0.5		0.3
LOS	А	Α	Α	Α		С	Α		Α
Approach Delay		6.1		4.1		20.2			0.3
Approach LOS		Α		Α		С			Α
Intersection Summary									
Cycle Length: 65									
Actuated Cycle Length: 65	5								
Offset: 0 (0%), Reference		:WBTL ar	nd 6:EBT	L. Start of	Green				
Natural Cycle: 65				,					
Control Type: Actuated-Co	oordinated								
Maximum v/c Ratio: 0.53									
Intersection Signal Delay:	5.5			li	ntersection	n LOS: A			
Intersection Capacity Utiliz		,		10	CU Level	of Service	e C		
Analysis Period (min) 15									
Splits and Phases: 4: 5	6th St/56th	St & Linco	oln Dr						

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Mountain Shadows Resort Total AM

HCM 6th LOS

4: 56th St/56th St & Lincoln Dr HCM 6th Signalized Intersection Summary

	ၨ	→	•	1	←	4	4	†	1	\	↓	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	↑ ↑		ሻ	↑ ↑			ની	7		4	
Traffic Volume (veh/h)	7	1297	48	14	1075	6	34	1	15	4	0	3
Future Volume (veh/h)	7	1297	48	14	1075	6	34	1	15	4	0	3
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	8	1457	26	15	1132	2	37	1	7	5	0	0
Peak Hour Factor	0.89	0.89	0.89	0.95	0.95	0.95	0.93	0.93	0.93	0.88	0.88	0.88
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	422	2656	47	312	2707	5	205	4	101	172	0	0
Arrive On Green	0.74	0.74	0.74	0.74	0.74	0.74	0.06	0.06	0.06	0.06	0.00	0.00
Sat Flow, veh/h	496	3572	64	356	3640	6	1495	65	1585	960	0	0
Grp Volume(v), veh/h	8	724	759	15	553	581	38	0	7	5	0	0
Grp Sat Flow(s),veh/h/ln	496	1777	1859	356	1777	1869	1560	0	1585	960	0	0
Q Serve(g_s), s	0.4	11.5	11.5	1.2	7.5	7.5	0.0	0.0	0.3	0.2	0.0	0.0
Cycle Q Clear(g_c), s	7.9	11.5	11.5	12.7	7.5	7.5	1.3	0.0	0.3	1.5	0.0	0.0
Prop In Lane	1.00		0.03	1.00		0.00	0.97		1.00	1.00		0.00
Lane Grp Cap(c), veh/h	422	1321	1382	312	1321	1390	209	0	101	172	0	0
V/C Ratio(X)	0.02	0.55	0.55	0.05	0.42	0.42	0.18	0.00	0.07	0.03	0.00	0.00
Avail Cap(c_a), veh/h	422	1321	1382	312	1321	1390	623	0	561	580	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	4.6	3.6	3.6	6.4	3.1	3.1	29.1	0.0	28.6	29.8	0.0	0.0
Incr Delay (d2), s/veh	0.1	1.6	1.6	0.3	1.0	0.9	0.4	0.0	0.3	0.1	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.0	2.2	2.2	0.1	1.4	1.5	0.6	0.0	0.1	0.1	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	4.7	5.2	5.2	6.7	4.1	4.0	29.5	0.0	28.9	29.9	0.0	0.0
LnGrp LOS	А	A	Α	Α	Α	A	С	Α	С	С	A	A
Approach Vol, veh/h		1491			1149			45			5	
Approach Delay, s/veh		5.2			4.1			29.4			29.9	
Approach LOS		Α			Α			С			С	
Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		54.8		10.2		54.8		10.2				
Change Period (Y+Rc), s		6.5		6.0		6.5		6.0				
Max Green Setting (Gmax), s		29.5		23.0		29.5		23.0				
Max Q Clear Time (g_c+I1), s		14.7		3.3		13.5		3.5				
Green Ext Time (p_c), s		6.4		0.1		8.9		0.0				
Intersection Summary												
HCM 6th Ctrl Delay			5.2									

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Mountain Shadows Resort Total PM

1: Camelback Manor Dr/Desert Fairways Dr & Lincoln Dr

	•	-	1	•	1	†	-	Į.	
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	
Lane Configurations	ሻ	† 1>	ሻ	↑ ↑		4	7	rî,	Т
Traffic Volume (vph)	63	1104	5	1293	4	2	115	2	
Future Volume (vph)	63	1104	5	1293	4	2	115	2	
Turn Type	pm+pt	NA	Perm	NA	Perm	NA	Perm	NA	
Protected Phases	1	6		2		4		8	
Permitted Phases	6		2		4		8		
Detector Phase	1	6	2	2	4	4	8	8	
Switch Phase									
Minimum Initial (s)	4.0	15.0	15.0	15.0	7.0	7.0	7.0	7.0	
Minimum Split (s)	8.0	26.0	26.0	26.0	30.0	30.0	30.0	30.0	
Total Split (s)	8.0	35.0	27.0	27.0	30.0	30.0	30.0	30.0	
Total Split (%)	12.3%	53.8%	41.5%	41.5%	46.2%	46.2%	46.2%	46.2%	
Yellow Time (s)	3.0	4.5	4.5	4.5	3.0	3.0	3.0	3.0	
All-Red Time (s)	1.0	1.5	1.5	1.5	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	0.0	0.0	
Total Lost Time (s)	4.0	6.0	6.0	6.0		6.0	6.0	6.0	
Lead/Lag	Lead		Lag	Lag					
Lead-Lag Optimize?	Yes		Yes	Yes					
Recall Mode	None	C-Max	C-Max	C-Max	None	None	None	None	
Act Effct Green (s)	42.3	40.3	34.2	34.2		12.7	12.7	12.7	
Actuated g/C Ratio	0.65	0.62	0.53	0.53		0.20	0.20	0.20	
v/c Ratio	0.26	0.55	0.02	0.86		0.05	0.57	0.29	
Control Delay	7.8	9.2	22.2	30.2		13.9	31.0	6.7	
Queue Delay	0.0	0.0	0.0	0.0		0.0	0.0	0.0	
Total Delay	7.8	9.2	22.2	30.2		13.9	31.0	6.7	
LOS	А	Α	С	С		В	С	Α	
Approach Delay		9.1		30.1		13.9		20.5	
Approach LOS		Α		С		В		С	
Intersection Summary									
Cycle Length: 65									
Actuated Cycle Length: 65									
Offset: 49 (75%), Reference	ed to phase	e 2:WBTL	and 6:Ef	BTL, Start	of Green				
Natural Cycle: 80									

Natural Cycle: 80 Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.86

Intersection Signal Delay: 20.7
Intersection Capacity Utilization 71.2%
Analysis Period (min) 15

Intersection LOS: C ICU Level of Service C

Splits and Phases: 1: Camelback Manor Dr/Desert Fairways Dr & Lincoln Dr



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1: Camelback Manor Dr/Desert Fairways Dr & Lincoln Dr HCM 6th Signalized Intersection Summary

	۶	→	•	√	←	4	<u> </u>	†	~	\	 	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	↑ ↑		ħ	† î>			4		7	ĵ.,	
Traffic Volume (veh/h)	63	1104	3	5	1293	171	4	2	6	115	2	85
Future Volume (veh/h)	63	1104	3	5	1293	171	4	2	6	115	2	85
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1945	1870	1870	1870	1870
Adj Flow Rate, veh/h	68	1200	0	5	1390	98	6	3	5	155	3	74
Peak Hour Factor	0.92	0.92	0.92	0.93	0.93	0.93	0.69	0.69	0.69	0.74	0.74	0.74
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	267	2418	0	352	1938	136	140	75	75	317	8	207
Arrive On Green	0.04	0.68	0.00	0.39	0.39	0.39	0.14	0.14	0.14	0.14	0.14	0.14
Sat Flow, veh/h	1781	3647	0	466	3368	237	453	553	559	1407	62	1532
Grp Volume(v), veh/h	68	1200	0	5	731	757	14	0	0	155	0	77
Grp Sat Flow(s),veh/h/ln	1781	1777	0	466	1777	1828	1565	0	0	1407	0	1595
Q Serve(g_s), s	0.9	10.6	0.0	0.5	22.7	22.9	0.0	0.0	0.0	3.2	0.0	2.9
Cycle Q Clear(g_c), s	0.9	10.6	0.0	4.2	22.7	22.9	2.9	0.0	0.0	6.1	0.0	2.9
Prop In Lane	1.00		0.00	1.00		0.13	0.43		0.36	1.00		0.96
Lane Grp Cap(c), veh/h	267	2418	0	352	1022	1051	290	0	0	317	0	215
V/C Ratio(X)	0.25	0.50	0.00	0.01	0.72	0.72	0.05	0.00	0.00	0.49	0.00	0.36
Avail Cap(c_a), veh/h	299	2418	0	352	1022	1051	657	0	0	647	0	589
HCM Platoon Ratio	1.00	1.00	1.00	0.67	0.67	0.67	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	1.00	1.00	1.00	1.00	0.00	0.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	10.1	5.0	0.0	11.0	15.5	15.5	24.5	0.0	0.0	26.8	0.0	25.5
Incr Delay (d2), s/veh	0.2	0.7	0.0	0.1	4.3	4.3	0.1	0.0	0.0	1.2	0.0	1.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.3	2.4	0.0	0.0	10.2	10.6	0.2	0.0	0.0	2.3	0.0	1.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	10.2	5.7	0.0	11.1	19.7	19.8	24.6	0.0	0.0	27.9	0.0	26.6
LnGrp LOS	В	A	A	В	В	В	С	A	A	С	A	С
Approach Vol, veh/h		1268			1493			14			232	
Approach Delay, s/veh		6.0			19.7			24.6			27.5	
Approach LOS		Α			В			С			С	
Timer - Assigned Phs	1	2		4		6		8				
Phs Duration (G+Y+Rc), s	6.8	43.4		14.8		50.2		14.8				
Change Period (Y+Rc), s	4.0	6.0		6.0		6.0		6.0				
Max Green Setting (Gmax), s	4.0	21.0		24.0		29.0		24.0				
Max Q Clear Time (g_c+I1), s	2.9	24.9		4.9		12.6		8.1				
Green Ext Time (p_c), s	0.0	0.0		0.0		7.6		8.0				
Intersection Summary	_											
HCM 6th Ctrl Delay			14.6									
HCM 6th LOS			В									

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2:	West Access	& Lincoln Dr
		HCM 6th TWSC

Intersection	0.0						
Int Delay, s/veh	0.2						
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	44	7	7	44		7	
Traffic Vol, veh/h	1218	43	16	1521	0	29	
Future Vol, veh/h	1218	43	16	1521	0	29	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized		None	-	None	-	None	
Storage Length			75			0	
Veh in Median Storage,	# 0			0	0	-	
Grade, %	0			0	0		
Peak Hour Factor	95	95	94	94	63	63	
Heavy Vehicles, %	2		2	2	2	2	
Mymt Flow	1282	45	17	1618	0	46	
WWIII FIOW	1282	45	17	1018	U	40	
Major/Minor N	Najor1	1	Major2	- 1	Vinor1		
Conflicting Flow All	0	0	1327	0	-	641	
Stage 1	-	-	-	-		-	
Stage 2	-	-	-	-	-	-	
Critical Hdwy	-	-	4.14	-	-	6.94	
Critical Hdwy Stg 1		-	-	-	-	-	
Critical Hdwy Stg 2		-	-	-	-	-	
Follow-up Hdwy		-	2.22			3.32	
Pot Cap-1 Maneuver			*880		0	*588	
Stage 1			-		0	300	
Stage 2					0		
Platoon blocked, %			1		U	1	
	-	-		-			
Mov Cap-1 Maneuver	-	-	*880	-	-	*588	
Mov Cap-2 Maneuver	-	-	-	-	-	-	
Stage 1	-	-	-	-	-	-	
Stage 2	-	-	-	-	-	-	
Approach	EB		WB		NB		
HCM Control Delay, s	0		0.1		11.6		
HCM LOS	U		0.1		В		
HCIVI LU3					D		
Minor Lane/Major Mvmt		NBLn1	EBT	EBR	WBL	WBT	
Capacity (veh/h)		588	-	-	* 880	-	
		0.078	-	-	0.019	-	
HCM Lane V/C Ratio		11.6	-	-	9.2	-	
					Α	-	
HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS		В	-	-			
HCM Control Delay (s)					0.1	-	
HCM Control Delay (s) HCM Lane LOS		В		•			

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Lane Configurations ↑↑ ↑ ↑↑ <th>NBR 27 27 0 Stop None 22 38</th>	NBR 27 27 0 Stop None 22 38
Lane Configurations AA IF IAA IF Traffic Vol, veh/h 1222 25 21 1504 18 2 Euture Vol, veh/h 1222 25 21 1504 18 2 Conflicting Peds, #/hr 0 0 0 0 0 0 Sign Control Free Free Free Free Free Free Free Free Free Stop Stop Stop 0 - 0	27 27 0 Stop None - - 72 2 38
Lane Configurations	27 27 0 Stop None - - 72 2 38
Traffic Vol, veh/h 1222 25 21 1504 18 2 Future Vol, veh/h 1222 25 21 1504 18 2 Conflicting Peds,#hr 0 0 0 0 0 0 Sign Control Free Free Free Free Free Free Stop Sto None 22 2 2 2	27 0 Stop None - - 72 2 38
Future Vol, veh/h Conflicting Peds, #/hr Sign Control Free Free Free Free Free Free Free Fre	27 0 Stop None - - 72 2 38
Conflicting Peds, #hr 0	0 Stop None - - - 72 2 38
Sign Control Free RTCAN Free RTCANNOR Free RTCANNO	Stop None - - 72 2 38
RT Channelized	None - - 72 2 38
Storage Length	72 2 38
Weh in Median Storage, # 0 - 0 1 Grade, % 0 - 0 0 Peak Hour Factor 96 96 94 94 72 7 Heavy Vehicles, % 2 3 3	72 2 38
Grade, % 0 - 0 0 0 Peak Hour Factor 96 96 94 94 72 7 Heavy Vehicles, % 2 2 2 2 2 Mvmt Flow 1273 26 22 1600 25 3 Major/Minor Major1 Major2 Minor1 Conflicting Flow All 0 0 1299 0 2117 63 Stage 1 1273 Stage 2 844 Critical Hdwy Stg 1 - 584 Critical Hdwy Stg 2 - 584 Follow-up Hdwy - 222 3.52 3.5 Follow-up Hdwy - 222 3.52 3.5 Stage 2 584 Follow-up Hdwy - 222 3.52 3.5 Stage 1 - 585 Stage 2 - 419 Platoon blocked, % - 1 2 Mov Cap-1 Maneuver - 880 - 55 Stage 2 - 419 Platoon blocked, % - 1 2 Mov Cap-2 Maneuver - 880 - 58 Mov Cap-2 Maneuver - 880 - 58 Mov Cap-2 Maneuver - 880 - 58 Mov Cap-2 Maneuver - 1880 - 58	72 2 38 637
Peak Hour Factor 96 96 94 94 72 7 Heavy Vehicles, % 2 Minor1 Minor1 Conflicting Flow All 0 0 1299 0 2117 63 53 54 64 684 684 685 64 684 685 686 684 686 686 684 686 686 684 686	72 2 38 637
Heavy Vehicles, % 2 2 2 2 2 2 Mvmt Flow 1273 26 22 1600 25 3 3	2 38 637
Mvml Flow 1273 26 22 1600 25 3 Major/Minor Major1 Major2 Minor1 Conflicting Flow All 0 0 1299 0 2117 63 Stage 1 - - - 1273 54 - - 844 Critical Hdwy - - 4.14 - 6.84 6. Critical Hdwy Stg 1 - - 5.84 Critical Hdwy Stg 2 - - 5.84 Follow-up Hdwy - 2.22 3.52 3.3 3.52 3.3 2 7.55 Stage 1 - - 880 - - 7.555 Stage 2 - - 4.19 Platoon blocked, % - - 1 2 Mov Cap-1 Maneuver - 880 - - 758 - 754 Stage 2 - - - 769 Stage 2 - - - 1 - - 754 Stage 2 - - </td <td>637</td>	637
Major/Minor Major1 Major2 Minor1 Conflicting Flow All 0 0 1299 0 2117 63 Stage 1 - - - 1273 63 Stage 2 - - - 844 6.94 6.94 6.94 6.94 6.94 6.94 6.94 6.94 6.94 6.92 6.94 6.94 6.92 6.94 6.92 6.94 6.92 6.92 6.94 6.92 6.92 6.92 6.92 7.92 7.84 6.92 7.92 7.84 6.92 7.92 7.84 6.92 7.92 7.84 6.92 7.92 7.84 6.92 7.92 7.84 6.92 7.84 6.92 7.84 6.92 7.92 7.84 7.84 7.92 7.92 7.84 7.92 7.92 7.92 7.92 7.92 7.92 7.92 7.92 7.92 7.92 7.92 7.92 7.92 7.92 7.92 7.92 7.92<	637
Conflicting Flow All	
Conflicting Flow All	
Stage 1	
Stage 2	
Critical Hdwy - 4.14 - 6.84 6.9 Critical Hdwy Stg 1 - - - 5.84 Critical Hdwy Stg 2 - - - 5.84 Follow-up Hdwy - 2.22 3.52 3.3 3.3 Pot Cap-1 Maneuver - *880 - *55 584 58 58 58 58 3.2 *3.2 3.2 3.3 3.3 3.2 4.19 9.2 4.19 9.2 9.2 9.2 9.2 9.2 9.2 9.2 9.2 9.2 9.2 9.2 9.2 9.2 9.2 9.2 </td <td></td>	
Critical Hdwy Stg 1 - - 5.84 Critical Hdwy Stg 2 - - 5.84 Critical Hdwy Stg 2 - - 5.84 Follow-up Hdwy - 2.22 3.52 3.3 Pot Cap-1 Maneuver - *880 - *55 Stage 1 - - *555 Stage 2 - - *419 Platoon blocked, % - 1 - 2 Mov Cap-1 Maneuver - *880 - *56 Mov Cap-2 Maneuver - - *209 Stage 1 - - *541 Stage 2 - - *419 Approach EB WB NB HCM Control Delay, s 0 0.1	-
Critical Hdwy Stg 2 - - 5.84 Follow-up Hdwy - 2.22 3.52 3.52 Pot Cap-1 Maneuver - *880 - *55 Stage 1 - - *555 Stage 2 - - *419 Platoon blocked, % - 1 2 Mov Cap-1 Maneuver - *880 - *58 Mov Cap-2 Maneuver - - *209 Stage 1 - - *541 Stage 2 - - *419 Approach EB WB NB HCM Control Delay, s 0 0.1	6.94
Follow-up Hdwy - 2.22 - 3.52 3.5 Pol Cap-1 Maneuver - '880 - '565 Stage 1 '555 Stage 2 '419 Platoon blocked, % - 1 - 2 Mov Cap-1 Maneuver - '880 - '565 Mov Cap-2 Maneuver - '880 - '564 Stage 1 - '541 Stage 2 - '419 Approach EB WB NB HCM Control Delay, s 0 0.1	-
Pot Cap-1 Maneuver - *880 - *58 Stage 1 - - *555 Stage 2 - - *419 Platoon blocked, % - 1 2 Mov Cap-1 Maneuver - *880 - *58 Mov Cap-2 Maneuver - - *209 Stage 1 - - *541 Stage 2 - - *419 Approach EB WB NB HCM Control Delay, s 0 0.1	-
Stage 1 - - *555 Stage 2 - - *419 Platoon blocked, % - 1 - 2 Mov Cap-1 Maneuver - *880 - *58 Mov Cap-2 Maneuver - - *209 Stage 1 - - *541 Stage 2 - *419 Approach EB WB NB HCM Control Delay, s 0 0.1	3.32
Stage 2 - - *419 Platoon blocked, % - 1 - 2 Mov Cap-1 Maneuver - *880 - *58 Mov Cap-2 Maneuver - - *209 Stage 1 - - *541 Stage 2 - - *419 Approach EB WB NB HCM Control Delay, S 0 0.1	*588
Platoon blocked, % - - 1 - 2 Mov Cap-1 Maneuver - *880 - *58 Mov Cap-2 Maneuver - - *209 Stage 1 - - *541 Stage 2 - - *419 Approach EB WB NB HCM Control Delay, s 0 0.1	-
Mov Cap-1 Maneuver - *880 - *58 Mov Cap-2 Maneuver - - *209 Stage 1 - - *541 Stage 2 - - *419 Approach EB WB NB HCM Control Delay, s 0 0.1	-
Mov Cap-2 Maneuver - * 209 Stage 1 - - * 541 Stage 2 - - * 419 Approach EB WB NB HCM Control Delay, s 0 0.1	1
Mov Cap-2 Maneuver - - *209 Stage 1 - - *541 Stage 2 - - *419 Approach EB WB NB HCM Control Delay, s 0 0.1	*588
Stage 1 - - * 541 Stage 2 - - * 419 Approach EB WB NB HCM Control Delay, s 0 0.1	
Stage 2 - - - *419 Approach EB WB NB HCM Control Delay, s 0 0.1	-
Approach EB WB NB HCM Control Delay, s 0 0.1	
HCM Control Delay, s 0 0.1	
HCM Control Delay, s 0 0.1	
HCM LOS	
TICIVI EUS -	
Minor Lane/Major Mvmt NBLn1 EBT EBR WBL WB	WBT
Capacity (veh/h) * 880	-
HCM Lane V/C Ratio 0.025	
HCM Control Delay (s) 9.2	
HCM Lane LOS A	-
HCM 95th %tile Q(veh) 0.1	-
Notes	-
-: Volume exceeds capacity \$: Delay exceeds 300s +: Co	

Mountain Shadows Resort

Total PM

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Mountain Shadows Resort Total PM 4: 56th St/56th St & Lincoln Dr Timings

	•	-	•	•	1	†	/	-	ļ
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT
Lane Configurations		† 1>	ች	↑ Ъ		ર્ન	7		4
Traffic Volume (vph)	33	1182	52	1443	68	2	26	3	2
Future Volume (vph)	33	1182	52	1443	68	2	26	3	2
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	Perm	NA
Protected Phases		6		2		4			8
Permitted Phases	6		2		4		4	8	
Detector Phase	6	6	2	2	4	4	4	8	8
Switch Phase									
Minimum Initial (s)	15.0	15.0	15.0	15.0	7.0	7.0	7.0	7.0	7.0
Minimum Split (s)	31.5	31.5	31.5	31.5	13.0	13.0	13.0	29.0	29.0
Total Split (s)	35.0	35.0	35.0	35.0	30.0	30.0	30.0	30.0	30.0
Total Split (%)	53.8%	53.8%	53.8%	53.8%	46.2%	46.2%	46.2%	46.2%	46.2%
Yellow Time (s)	4.5	4.5	4.5	4.5	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.0	2.0	2.0	2.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0		0.0	0.0		0.0
Total Lost Time (s)	6.5	6.5	6.5	6.5		6.0	6.0		6.0
Lead/Lag									
Lead-Lag Optimize?									
Recall Mode	C-Max	C-Max	C-Max	C-Max	None	None	None	None	None
Act Effct Green (s)	47.0	47.0	47.0	47.0		9.4	9.4		9.4
Actuated g/C Ratio	0.72	0.72	0.72	0.72		0.14	0.14		0.14
v/c Ratio	0.21	0.52	0.23	0.61		0.41	0.10		0.13
Control Delay	6.0	3.5	8.4	7.7		31.1	3.4		19.1
Queue Delay	0.0	0.0	0.0	0.0		0.0	0.0		0.0
Total Delay	6.0	3.5	8.4	7.7		31.1	3.4		19.1
.OS	Α	Α	Α	Α		С	Α		В
Approach Delay		3.6		7.7		23.6			19.1
Approach LOS		Α		Α		С			В
Intersection Summary									
Cycle Lenath: 65									
Actuated Cycle Length: 65									
Offset: 29 (45%), Reference	ed to phase	e 2:WBTL	and 6:El	BTL, Start	of Green	ı			
Vatural Cycle: 70	'								
Control Type: Actuated-Co	ordinated								
Maximum v/c Ratio: 0.61									
Intersection Signal Delay: 6	5.6			li li	ntersectio	n LOS: A			
Intersection Capacity Utiliza	ation 64.2%	5		10	CU Level	of Service	e C		
Analysis Period (min) 15									
	th St/56th	St & Linc	oln Dr						
4-						A			

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Mountain Shadows Resort Total PM

HCM 6th LOS

4: 56th St/56th St & Lincoln Dr HCM 6th Signalized Intersection Summary

	۶	→	•	•	←	•	4	†	1	\	ļ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	↑ ↑		ሻ	↑ ↑			ની	7		4	
Traffic Volume (veh/h)	33	1182	36	52	1443	32	68	2	26	3	2	14
Future Volume (veh/h)	33	1182	36	52	1443	32	68	2	26	3	2	14
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	36	1285	28	55	1519	23	76	2	18	5	3	14
Peak Hour Factor	0.92	0.92	0.92	0.95	0.95	0.95	0.89	0.89	0.89	0.63	0.63	0.63
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	279	2535	55	409	2554	39	245	5	150	85	39	98
Arrive On Green	1.00	1.00	1.00	0.71	0.71	0.71	0.09	0.09	0.09	0.09	0.09	0.09
Sat Flow, veh/h	336	3556	77	419	3583	54	1432	50	1585	177	413	1033
Grp Volume(v), veh/h	36	642	671	55	753	789	78	0	18	22	0	0
Grp Sat Flow(s),veh/h/ln	336	1777	1856	419	1777	1861	1481	0	1585	1624	0	0
Q Serve(g_s), s	2.4	0.0	0.0	2.8	13.7	13.8	1.9	0.0	0.7	0.0	0.0	0.0
Cycle Q Clear(g_c), s	16.2	0.0	0.0	2.8	13.7	13.8	3.1	0.0	0.7	0.8	0.0	0.0
Prop In Lane	1.00		0.04	1.00		0.03	0.97		1.00	0.23		0.64
Lane Grp Cap(c), veh/h	279	1267	1323	409	1267	1326	250	0	150	222	0	0
V/C Ratio(X)	0.13	0.51	0.51	0.13	0.59	0.60	0.31	0.00	0.12	0.10	0.00	0.00
Avail Cap(c_a), veh/h	279	1267	1323	409	1267	1326	635	0	585	652	0	0
HCM Platoon Ratio	2.00	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	2.4	0.0	0.0	3.1	4.7	4.7	27.9	0.0	26.9	27.0	0.0	0.0
Incr Delay (d2), s/veh	1.0	1.5	1.4	0.7	2.1	2.0	0.7	0.0	0.4	0.2	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.1	0.5	0.5	0.2	3.1	3.2	1.2	0.0	0.3	0.3	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	3.4	1.5	1.4	3.8	6.7	6.6	28.6	0.0	27.3	27.2	0.0	0.0
LnGrp LOS	Α	Α	Α	Α	Α	Α	С	Α	С	С	Α	Α
Approach Vol, veh/h		1349			1597			96			22	
Approach Delay, s/veh		1.5			6.6			28.4			27.2	
Approach LOS		Α			Α			С			С	
Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		52.8		12.2		52.8		12.2				
Change Period (Y+Rc), s		6.5		6.0		6.5		6.0				
Max Green Setting (Gmax), s		28.5		24.0		28.5		24.0				
Max Q Clear Time (g_c+l1), s		15.8		5.1		18.2		2.8				
Green Ext Time (p_c), s		8.4		0.4		6.1		0.1				
Intersection Summary												
HCM 6th Ctrl Delay			5.2									

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APPENDIX H

SCOTTSDALE DS&PM SECTION 5-3.201



TRANSPORTATION CHAPTER 5

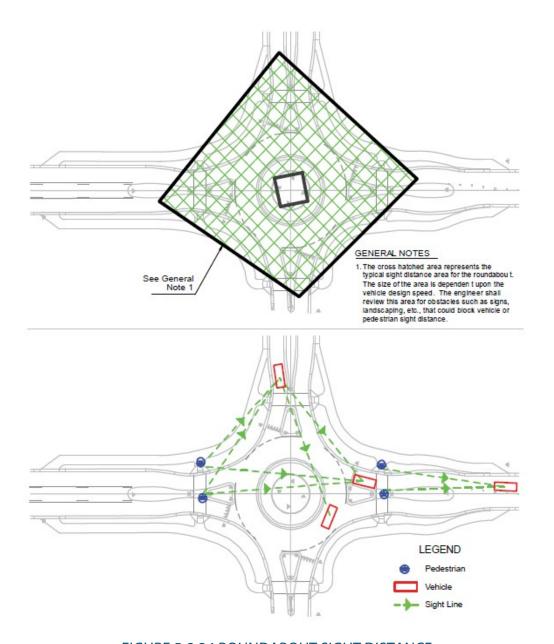


FIGURE 5-3.34 ROUNDABOUT SIGHT DISTANCE

STREET ACCESS AND DRIVEWAYS

Driveway types are determined by land use type and street classification. The standards for these driveway types are illustrated in Figure 5-3.38 through Figure 5-3.43. Refer to Figure 5-3.39 for driveway grade standards.

DRIVEWAY SPACING

Standard and minimum driveway spacing will generally conform to the following standards. This minimum spacing applies to proposed site driveway separation as well

5-3.2000

5-3.201

as separation from existing or planned driveways and streets on adjacent parcels. The spacing is measured to the driveway or street centerline.

STREET TYPE	STANDARD DRIVEWAY SPACING	MINIMUM DRIVEWAY SPACING
Local Residential / Local Collector	50 feet	50 feet
Local Industrial / Local Commercial	165 feet	125 feet
Minor Collector	165 feet	125 feet
Major Collector	250 feet	150 feet
Minor Arterial	330 feet	250 feet
Major Arterial	500 feet	300 feet

FIGURE 5-3.35 DRIVEWAY SPACING

Standard driveway spacing criteria shall apply for all new driveways where there are no conflicts with existing driveway and street intersections, site frontage is adequate, and there are no conflicts with natural features or drainage structures. The minimum driveway spacing may be allowed when approved by Transportation staff where those conflicts noted above exist or other site plan associated issues do not allow the standard driveway spacing to be implemented. In locations where the standard driveway spacing cannot be achieved, a deceleration lane may be required to mitigate the impact of the closer driveway spacing.

For sites that have frontage on two streets, primary access should be onto the minor street frontage. A maximum of two driveway openings is permitted to a site or parcel from the abutting street(s). The Transportation Department may permit additional driveway entrances when projected travel demands indicate it is in the interests of good traffic operation, and when adequate street frontage exists to maintain the above guidelines.

Where new development adjoins other similarly zoned property or compatible land uses, a cross access easement may be required to permit vehicular movement between the parcels or to reduce the number of access points required onto the adjacent public street. Combining driveways reduces the number of conflict points for pedestrians, bicyclists, and other vehicles. This may be required regardless of the development status of the adjoining property, unless the cross access is determined to be unfeasible by city staff.

New driveways on collector and arterial streets in areas that do not have raised medians shall align with existing or planned driveways and street intersections to avoid creating interlocking left turns and other conflicts. Offsets in the driveway centerlines may be allowed up to 6 feet. If the driveways cannot be aligned, the driveways should be offset a minimum distance of 125 feet along streets without a center turn lane, and a minimum 250 feet along streets with a center turn lane. When site driveway locations are modified, any existing driveways that are not going to be utilized for access must be removed and replaced with curb, gutter, and sidewalk to match the adjacent improvements.

DRIVEWAY LOCATIONS

A new access driveway will not be allowed (measured to the driveway centerline):

- A. Within 30 feet of any commercial property line, except when it is a joint-use driveway serving two abutting commercial properties and access agreements have been exchanged between, and recorded by, the two abutting property owners;
- B. When the total width of all driveways serving a property exceeds 50 percent of the curb line frontage;
- C. Within 50 feet of the rights-of-way line of an intersecting non-arterial street;
- D. Within 100 feet of the rights-of-way line of an intersecting arterial street;
- E. Within 100 feet of an approved median opening location on an arterial street;
- F. Less than the minimum spacing as established under Section 5-3.201;

VEHICULAR NON-ACCESS EASEMENT

For proper control of driveway access, a vehicular non-access easement (V.N.E.) is to be granted to the city, except at approved access points, along all collector and arterial streets when abutting property develops.

RESIDENTIAL DRIVEWAYS

A. Single-family Residential Development

Driveways serving single-family residential units should be S-1 type driveways as shown in Figure 5-3.40. Only one driveway per lot street frontage is allowed except where the street frontage is of sufficient length to maintain a separation of 50 feet between driveways. The minimum driveway length is 18 feet, measured from the face of the garage opening to the back of sidewalk or the back of curb if no sidewalk is provided. Refer to Section 2-2.308 for additional discussion on driveways. Refer to Standard Detail Drawings (2200 Series) for access ramp design requirements.

B. Multi-family Residential Development

Driveways serving multi-family residential units should be CL and CH type driveways, as shown in Figure 5-3.41 through Figure 5-3.44. Type CL-1 and CL-2 are low-volume driveways to be used on local streets. Type CH-1, -2 and -3 are high volume driveways to be used on collector and arterial streets. CL type driveways may be required along urban character collector and arterial streets with higher pedestrian traffic. The minimum driveway length is 50 feet, measured from the entrance to the off-street parking area to the back of sidewalk, or to the back of curb if no sidewalk is provided. Refer to Standard Detail Drawings (2200 Series) for access ramp design requirements.

C. Limitations on Residential Access

Residential properties that have frontage on a local street, an arterial, or collector street are limited to local street access.

In some instances, residential parcels fronting only on arterial or collector streets may be given access if alternate public access is not available. When such access is allowed, the driveway must be circular, or it must have a turn-around area to ensure there is no need for backing onto the street.

5-3.203

5-3.202

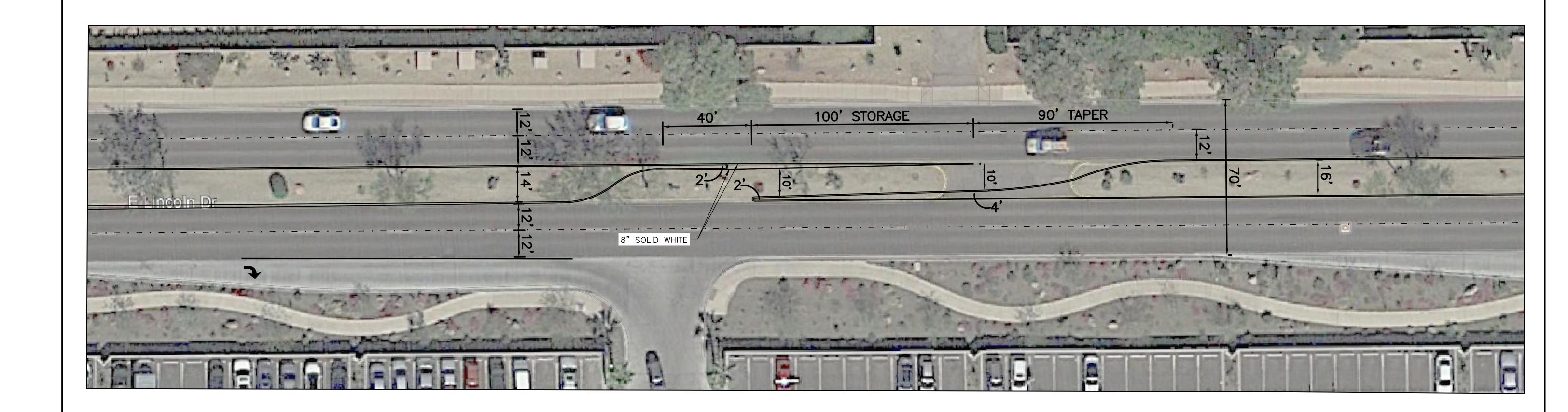
5-3.204

APPENDIX I

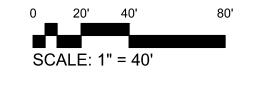
LEFT TURN LANE DESIGN CONCEPT











SHEET TITLE	Plan Shee	et Title		
PROJECT TITLE N	MOUNTAIN LEFT T		W	SHT.
DESIGNED: XXXXX	DRAWN: XXXXX		XXXXX	FIG-1

APPENDIX J

QUEUE STORAGE ANALYSIS



Queue Length Analysis

Signalized Intersection 2020

Average Vehicle Length (ft): 25 Cycles: 2

Intersection Cycle Length (sec): 65

Equation Used: storage length = 2 x (vehicles/hour)/(cycles/hour) x average vehicle length

Intersection	Approach	AM Peak	Midday	PM Peak	Max vehs per	Max trucks	Storage
Intersection	Approach	(veh/hr)	Peak	(veh/hr)	2 cycles	per 2 cycles	Length
	NB Left	2	0	4	1	0	25'
	SB Left	75	0	115	5	0	125'
	EB Left	68	0	63	3	0	75'
Desert Fairways Dr & Lincoln Dr	WB Left	5	0	5	1	0	25'
Desert I all ways DI & Lincoll DI	NB Right	5	0	6	1	0	25'
	SB Right	45	0	85	4	0	100'
	EB Right	4	0	3	1	0	25'
	WB Right	56	0	171	7	0	175'
	NB Left	34	0	68	3	0	75'
	SB Left	4	0	3	1	0	25'
	EB Left	7	0	33	2	0	50'
56th St & Lincoln Dr	WB Left	14	0	52	2	0	50'
John St & Elifcolli Di	NB Right	15	0	26	1	0	25'
	SB Right	3	0	14	1	0	25'
	EB Right	48	0	36	2	0	50'
	WB Right	6	0	32	2	0	50'



Unsignalized Intersection 2020

Average Vehicle Length (ft): 25

Equation Used: storage length = $2 \times (vehicles/hour)/(60 \text{ minutes/hour}) \times average vehicle length}$

Intersection	Annroach	AM Peak	Midday	PM Peak	Veh per 2	Trucks per	Storage
intersection	Approach	(veh/hr)	Peak	(veh/hr)	minutes	2 minutes	Length
	NB Left	0	0	0	0	0	0'
	SB Left	0	0	0	0	0	0'
	EB Left	0	0	0	0	0	0'
Mountain Shadows West &	WB Left	7	0	16	1	0	25'
Lincoln Dr	NB Right	13	0	29	1	0	25'
	SB Right	0	0	0	0	0	0'
	EB Right	24	0	43	2	0	50'
	WB Right	0	0	0	0	0	0'
	NB Left	23	0	18	1	0	25'
	SB Left	0	0	0	0	0	0'
	EB Left	0	0	0	0	0	0'
Mountain Shadows East &	WB Left	11	0	21	1	0	25'
Lincoln Dr	NB Right	14	0	27	1	0	25'
	SB Right	0	0	0	0	0	0'
	EB Right	28	0	25	1	0	25'
	WB Right	0	0	0	0	0	0'



	•	→	•	←	†	\	ļ
Lane Group	EBL	EBT	WBL	WBT	NBT	SBL	SBT
Lane Group Flow (vph)	75	1417	6	1226	21	89	55
v/c Ratio	0.22	0.55	0.03	0.55	0.08	0.43	0.19
Control Delay	5.4	7.1	8.0	11.0	15.1	30.5	9.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	5.4	7.1	8.0	11.0	15.1	30.5	9.2
Queue Length 50th (ft)	7	134	1	197	3	33	0
Queue Length 95th (ft)	22	233	m4	319	6	61	23
Internal Link Dist (ft)		606		1662	828		853
Turn Bay Length (ft)	150		50			75	
Base Capacity (vph)	334	2557	216	2217	641	511	620
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.22	0.55	0.03	0.55	0.03	0.17	0.09
Intersection Summary							

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m Volume for 95th percentile queue is metered by upstream signal.

	۶	→	•	←	†	/	ļ
Lane Group	EBL	EBT	WBL	WBT	NBT	NBR	SBT
Lane Group Flow (vph)	8	1489	15	1138	38	16	8
v/c Ratio	0.02	0.52	0.07	0.40	0.21	0.07	0.04
Control Delay	2.3	6.0	4.6	4.0	28.5	0.5	0.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	2.3	6.0	4.6	4.0	28.5	0.5	0.3
Queue Length 50th (ft)	1	224	2	83	14	0	0
Queue Length 95th (ft)	m0	331	8	135	38	0	0
Internal Link Dist (ft)		411		603	435		338
Turn Bay Length (ft)	50		80			285	
Base Capacity (vph)	358	2849	225	2859	531	598	530
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.02	0.52	0.07	0.40	0.07	0.03	0.02
Intersection Summary							

m Volume for 95th percentile queue is metered by upstream signal.

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	۶	→	•	←	†	-	ļ
Lane Group	EBL	EBT	WBL	WBT	NBT	SBL	SBT
Lane Group Flow (vph)	68	1203	5	1574	18	155	118
v/c Ratio	0.26	0.55	0.02	0.86	0.05	0.57	0.29
Control Delay	7.8	9.2	22.2	30.1	13.9	31.0	6.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	7.8	9.2	22.2	30.1	13.9	31.0	6.7
Queue Length 50th (ft)	8	124	2	333	3	56	1
Queue Length 95th (ft)	26	226	m3	#580	11	76	21
Internal Link Dist (ft)		606		1662	828		853
Turn Bay Length (ft)	150		50			75	
Base Capacity (vph)	266	2193	229	1839	637	513	659
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.26	0.55	0.02	0.86	0.03	0.30	0.18

Intersection Summary

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 ^{# 95}th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

	۶	→	•	←	†	/	ţ
Lane Group	EBL	EBT	WBL	WBT	NBT	NBR	SBT
Lane Group Flow (vph)	36	1291	55	1615	78	29	30
v/c Ratio	0.24	0.51	0.22	0.63	0.41	0.10	0.13
Control Delay	6.7	3.5	8.1	8.1	31.1	3.4	19.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	6.7	3.5	8.1	8.1	31.1	3.4	19.7
Queue Length 50th (ft)	3	66	7	168	29	0	8
Queue Length 95th (ft)	m7	82	28	290	61	9	18
Internal Link Dist (ft)		411		603	435		338
Turn Bay Length (ft)	50		80			285	
Base Capacity (vph)	151	2548	250	2552	488	621	581
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.24	0.51	0.22	0.63	0.16	0.05	0.05
Intersection Summary							

intersection Summary

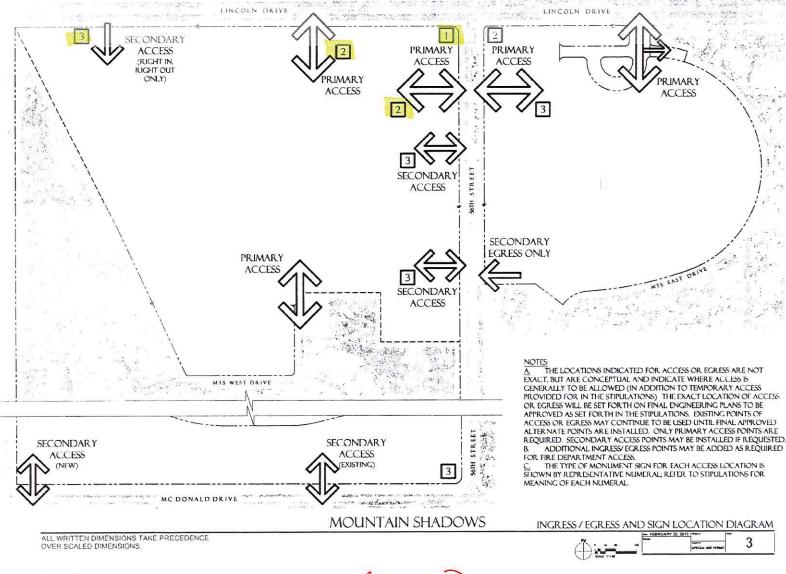
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m Volume for 95th percentile queue is metered by upstream signal.

APPENDIX K

SUP ACCESS DIAGRAMS





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